GENERAL NOTES:

DESIGN CRITERIA

1. THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE 2000 INTERNATION BUILDING CODE.

2. DESIGN LIVE LOADS:

ROOF	20 PSF
LEVEL 2 AND CORRIDORS	100 PSF
STAIRWAYS	100 PSF
ROOF MECHANICAL	PER PLAN

3. DESIGN DEAD LOADS:

PARTITIONS	20 PSF
CEILING / MECHANICAL	5 PSF
ROOFING / INSULATION	7 PSF
STRUCTURE	SELF WEIGHT

4. SNOW LOAD — 5 PSF

5. WIND LOAD DESIGN CRITERIA:

EQUIVALENT BASIC WIND SPEED	70 MPH
3 SCREWS GUST WIND SPEED	85 MPH
EXPOSURE	C
IMPORTANCE FACTOR	1.0

6. SIESMIC DESIGN DATA

OCCUPANCY CATAGORY	I
SEISMIC USE GROUP	I
IMPORTANCE FACTOR	1.0

7. SPECTRAL RESPONSE COEFFICIENTS
   Sds = 0.010
   Sd1 = 0.032

8. SITE CLASS — A

9. SIESMIC DESIGN CATAGORY — A

10. BASIC SEISMIC-FORCE-RESISTANCE SYSTEM - SHEAR WALL

DRAWINGS

1. BACKGROUND DRAWINGS SHOWN ON THE STRUCTURAL PLANS ARE FOR REFERENCE ONLY. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND OTHER DRAWINGS FOR ACTUAL REQUIREMENTS OF NON-STRUCTURAL ELEMENTS.

2. STRUCTURAL DRAWINGS SHOW ELEVATIONS AND RELATIONSHIPS OF STRUCTURAL ELEMENTS IN THEIR FINAL POSITION. CONSTRUCTION MEANS AND METHODS SHALL ALLOW FOR CAMBERS, LOSS OF CAMBER DURING CONSTRUCTION LOADING, DEFLECTIONS, AND NORMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING CONSTRUCTION.

LATERAL LOAD RESISTING SYSTEM

1. THE LATERAL LOAD RESISTING SYSTEM OF THE BUILDING CONSIST OF THE FOUNDATIONS, SLAB-ON-GRADE, FLOOR SLABS, ROOF DECK DIAPHRAGMS, AND WALLS. THE BUILDING FRAME IS NOT STABLE UNTIL ALL LATERAL LOAD RESISTING ELEMENTS ARE CONSTRUCTION IN THE FINAL CONDITION.

EXISTING CONDITIONS

1. FIELD VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO START OF SHOP DRAWINGS OR CONSTRUCTION. WHERE INDICATED ON DRAWINGS, SUBMIT RECORD OF FIELD CONDITIONS TO ARCHITECT.

2. EXISTING CONDITIONS REQUIRING MODIFICATIONS TO DOCUMENTS FOR PROPOSED CONSTRUCTION SHALL BE IMMEDIATELY SUBMITTED TO ARCHITECT.

EXISTING CONDITIONS

1. MAIN WIND FORCE RESISTING SYSTEM SUBJECT TO 2000 IBC QUALITY ASSURANCE FOR WIND REQUIREMENTS ARE AS FOLLOWS:

   A. ROOF DECK DIAPHRAGMS.
   B. FLOOR DECK DIAPHRAGMS.
   C. SHEAR WALLS
   D. STRUTS CONNECTED TO BRACED FRAMES.
   E. MASONRY WALLS.

2. REFER TO SPECIFICATIONS FOR TSTING AND SPECIAL INSPECTION REQUIREMENTS.

SUBMITTALS

1. REVIEW OF SUBMITTAL INFORMATION SHALL BE FOR GENERAL REQUIREMENTS OF PROJECT; AND SHALL NOT INCLUDE CHECKING OF DETAILED DIMENSIONS OR DETAILED QUANTITIES, NOR REVIEW OF THE CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE WORK SITE, OR MEANS AND METHODS OF DOING WORK.

2. CONTRACTOR SHALL CHECK ALL SHOP DRAWING SUBMITTALS FOR COMPLIANCE WITH CONTRACT DOCUMENTS.

3. CONTRACTOR SHALL INDICATE CHECKING AND APPROVAL OF SHOP DRAWINGS BY AFFIXING HIS SHOP DRAWING STAMP WITH THE DATE OF APPROVAL AND NAME OF PERSON APPROVING SHOP DRAWINGS.

4. CONSTRUCTION SCHEDULE SHALL ALLOW 2 WEEKS FOR SHOP DRAWING REVIEW AND RETURN BY THE STRUCTURAL ENGINEER. REVIEW BY THE DESIGN TEAM SHALL BE FOR GENERAL COMPLIANCE WITH THE COMPLIANCE WITH THE CONTRACT DOCUMENTS.

5. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL QUANTITIES, DIMENSIONS, CONSTRUCTION MEANS, METHODS, AND JOB SITE SAFETY.

6. ACTIONS TAKEN ON THE SHOP DRAWINGS SHALL NOT RELIEVE THE CONTRACTOR FROM RESPONSIBILITY OF COMPLIANCE WITH CONTRACT DOCUMENTS.

FOUNDATIONS

1. THE SUBSURFACE INFORMATION AND FOUNDATION DESIGN ARE BASED ON REPORT NO. 0701-1049, PREPARED BY FUGRO SOUTH, INC., DATED APRIL 26, 2001.

2. METHODS OF CONSTRUCTION OF FOUNDATION AND OTHER PORTIONS OF PROJECT SHALL COMPLY WITH THE REPORT.

DRILLED PIERS

DRILLED PIERS SHALL COMPLY WITH THE FOLLOWING:

1. PIER DESIGN IS BASED ON THE FOLLOWING:

A.	ALLOWABLE END BEARING:	34 KSF
B.	ALLOWABLE SKIN FRICTION:	5.5 KSF
C.	ALLOWABLE NEGATIVE SKIN FRICTION:	1.8 KSF
D.	BEARING STRATA IS GRAY LIMESTONE	

2. LONGITUDINAL PIER REINFORCING SHALL BE DELIVERED TO SITE IN STANDARD 60' LENGTHS. PIER CASES SHALL BE CUT TO PROPER LENGTHS AFTER PIER DEPTH IS FIELD VERIFIED.

3. SPLICES IN VERTICAL REINFORCING SHALL BE 64 BAR DIAMETERS UNLESS NOTED OTHERWISE.

4. PENETRATION SHALL BE DRILLED FULL DEPTH IN A CONTINUOUS OPERATION. CONCRETE SHALL BE PLACED IN PIER WITHIN 8 HOURS OF COMPLETED DRILLING

5. ELEVATION OF TOP OF PIERS, WHERE NOT GIVEN, IS AT THE BOTTOM OF THE DEEPEST STRUCTURAL ELEMENT SUPPORTED BY THE PIER.

6. PIER SHAFTS ABOVE GRADE SHALL BE FORMED TO SCHEDULED SHAFT DIAMETER.

7. PIERS SHALL NOT BE DRILLED UNTIL ALL SUBGRADE TREATMENTS ARE COMPLETED.

8. PIER SHAFT DIAMETERS SHALL NOT EXCEED THE SCHEDULED DIAMETERS WITHOUT APPROVAL OF THE ENGINEER.

9. PROVIDE ADDITIONAL REINFORCING AND ACCESSORIES AS REQUIRED TO HANDLE AND PLACE CAGE.

FOOTINGS

1. ALLOWABLE BEARING PRESSURE IS 4,000 POUNDS PER SQUARE FOOT.

2. SOILS BELOW FOOTINGS AND EXTENDING 1/2 FOOTING WIDTH ON EACH SIDE OF FOOTING SHALL BE EXCAVATED TO TOP OF ROCK. SELECT FILL SHALL BE REPLACED IN MAXIMUM 8" (LOOSE MEASUREMENT) LIFTS AND COMPACTED TO A MINIMUM 95% ASTM D 698 STANDARD PROCTOR DENSITY AT A MOISTURE CONTENT OF -3% TO +3% OF OPTIMUM.

3. BEARING SURFACES SHALL BE CLEAN AND FREE OF LOOSE OR WET MATERIALS.

4. SIDES OF FOOTING SHALL BE FORMED TO DIMENSIONS SHOWN.

SLAB-ON-GRADE

1. FLOOR SLAB SHALL BE 5" THICK OVER VAPOR BARRIER AND SELECT FILL TO TOP OF ROCK.

2. FLOOR SLAB ON GRADE HAS BEEN DESIGNED AS A STRUCTURAL DIAPHRAGM.

3. REINFORCE SLAB WITH #3 AT 14" EACH WAY, PLACED 2" FROM TOP OF SLAB. PROVIDE ADEQUATE MEANS OF CHAIRING AND ACCESSORY STEEL AS REQUIRED TO SECURE POSITION.

4. PROVIDE CONTRACTION AND/OR ISOLATION JOINTS AT COLUMN CENTERLINES AND AT MAXIMUM SPACING OF 5 FEET. THE MAXIMUM DISTANCE BETWEEN CONSTRUCTION AND/OR ISOLATION JOINTS SHALL BE 80'-0". SEE ARCH'L & LANDSCAPE PLANS FOR PATTERN.

5. SOIL BELOW SLABS SHALL COMPLY WITH THE FOLLOWING:

   A. EXCAVATE TO TOP OF ROCK. AT POOL EXCAVATION MAINTAIN A MINIMUM OF 4" FROM BOTTOM OF POOL TO TOP OF ROCK.
   B. SCARIFY AND RECOMPACT 6" OF EXPOSED SOIL. RECOMPACTED SOIL SHALL BE COMPACTED AT-3% TO +3% OF OPTIMUM MOISTURE CONTENT TO 95% OF ASTM D 698 STANDARD PROCTOR DENSITY. LOOSE LIFT THICKNESS SHALL 6 TO 8".
   C. PLACE LOW PI FILL AT CONTROLLED MOISTURE AND COMPACTION.

6. LOW PI FILL SHALL COMPLY WITH THE FOLLOWING:

   A. MAXIMUM PLASTICITY INDEX OF 6 TO 15.
   B. LIQUID LIMIT LESS THAN 40.
   C. 20 TO 45 PERCENT OF PARTICILES PASSING NO. 200 SIEVE.

7. LOW PI FILL SHALL BE CONTAINED WITHIN LIMITS OF PERIMETER GRADE BEAMS.

8. ALL MOISTURE CONTENTS AND COMPACTIONS SHALL BE VERIFIED BY AN INDEPENDENT TESTING LABORATORY.

9. SEE SPECIFICATIONS FOR BACKFILL REQUIREMENTS AT UTILITY TRENCHES.

CONSTRUCTION IN EXPANSIVE SOILS

1. PROVIDE VOID BOXES UNDER ALL PIER SUPPORTED STRUCTURAL ELEMENTS AT OR BELOW GRADE. ONLY SLABS-ON-GRADE SHALL BEAR ON SOIL.

2. PROVIDE PROPER PROTECTION OF VOID BOXES FROM MOISTURE. VOID BOXES THAT BECOME MOIST OR WEAK SHALL BE REPLACED PRIOR TO PLACING CONCRETE.

3. PROVIDE VOID BOXES DESIGNED TO SUPPORT THE HEIGHT OF REINFORCING STEEL AND WET CONCRETE. PLACE CONCRETE TO PREVENT OVERLOAD AND COLLAPSE OF VOID BOXES.

4. PROVIDE END CLOSURES AND JOINT SEALS TO PREVENT CONCRETE FROM ENTERING VOID BOXES.

5. PROVIDE VOID BOXES SPECIFICALLY PREMANUFACTURED WITH SEALED EDGES TO SHAPES REQUIRED FOR CIRCULAR OR IRREGULAR INTERFACES OF VOID BOXES AND CONCRETE ELEMENTS. FIELD FABRICATION OF VOID BOXES SHALL NOT BE ALLOWED.

6. CONSTRUCTION GRADING SHALL PROVIDE DRAINAGE OF WATER AWAY FROM THE BUILDING FOUNDATION.

7. WATER SHALL NOT POND NEAR BUILDING STRUCTURE. OPEN EXCAVATIONS MUST BE IMMEDIATELY PUMPED DRY OF ALL WATER.

8. THE BUILDING SUBGRADE SHALL BE MAINTAINED AT THE SPECIFIED MOISTURE CONTENT DURING CONSTRUCTION.

9. BACKFILL ON EXTERIOR SIDE OF GRADE BEAMS SHALL BE ON-SITE CLAYS PLACED IN MAXIMUM 8 INCH LIFTS AND COMPACTED TO A MINIMUM OF 85% STANDARD PROCTOR DENSITY PER ASTM D 698 AT OPTIMUM MOISTURE CONTENT.

10. SIDES OF ALL BELOW GRADE CONCRETE CONSTRUCTION SHALL BE FORMED TO THE LINES AND SHAPES SHOWN IN THE SECTIONS AND DETAILS. CONCRETE SHALL NOT BE PLACED AGAINST SOIL.

11. SIDES OF PIER SHAFTS SHALL REMAIN VERTICAL TO THE TOP OF GRADE OR THE TOP OF PIER. MUSHROOMING OF CONCRETE SHALL NOT BE ALLOWED.

CONCRETE REINFORCING

1. ALL CONCRETE REINFORCEMENT SHALL BE OF DOMESTIC MANUFACTURE AND SHALL CONFORM TO ASTM A615, GRADE 60, U.N.O. FIELD BENT DOWELS SHALL CONFORM TO ASTM A-615, GRADE 40.

2. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.

3. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE PER ACI 315.

4. REINFORCING SHOWN IN SECTIONS AND DETAILS IS SCHEMATIC AND INDICATES THAT REINFORCING EXISTS. PROVIDE REINFORCING AS REQUIRED BY SCHEDULE, NOTE, GENERAL NOTES, AND SPECIFICATIONS.

5. LONGITUDINAL BEAM REINFORCING SHOWN IN SECTION OR DETAIL TO BE CONTINUOUS SHALL HAVE TOP BARS HOOKED AT DISCONTINUOUS ENDS WITH STANDARD 90 DEGREE HOOKS.

6. WHEN PERMITTED, WELDING OF REINFORCEMENT SHALL BE PER AWS D1.4.

7. REINFORCING BARS MAY BE SPLICED ONLY AS SHOWN ON THE DRAWINGS EXCEPT THAT REINFORCING DESIGNATED AS "CONTINUOUS" MAY BE LAP SPLICED 36 BAR DIAMETERS. LAP SPLICES OF CONTINUOUS REINFORCEMENT IN UNSCHEDULED BEAMS SHALL BE MADE OVER THE SUPPORT FOR BOTTOM BARS AND AT MID-SPAN FOR TOP BARS.

8. CONCRETE PROTECTION FOR REINFORCEMENT, INCLUDING PRIMARY, STIRRUPS, TIES, ETC. SHALL BE AS NOTED BELOW, OR PER ACI 318 FOR CONDITIONS NOT NOTED:

	FOR CARBONATE, LIGHTWEIGHT AND SAND LIGHTWEIGHT AGGREGATES	FOR SILICEOUS AGGREGATES
CONCRETE PLACED AGAINST SOIL	3 INCHES	3 INCHES
FOOTINGS OR PIERS	3 INCHES	3 INCHES
GRADE BEAMS (FORMED)	3 INCHES BOTTOM	3 INCHES BOTTOM
	2 INCHES SIDE	2 INCHES SIDE
	2 INCHES TOP	2 INCHES TOP
BEAMS, COLUMNS	1-1/2 INCHES	1 INCHES
SLABS, ELEVATED, U.N.O. ON PLAN	1 INCH	1-1/4 INCHES
SLABS ON GRADE	2 INCHES TOP	2 INCHES TOP

DEFORMED BAR ANCHORS

1. DEFORMED BAR ANCHORS (DBA) SHALL BE NELSON, FLUX FILLED DEFORMED BAR ANCHORS, TYPE D2L, WELDED TO STEEL AS SHOWN IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER.

CONCRETE

1. CONCRETE PROPORTIONING, MIXING, TRANSPORTING, PLACING, AND CURING SHALL BE PER ACI 301.

2. CONCRETE SURFACES SHALL CONFORM TO TOLERANCE LIMITS PER ACI 117, U.N.O.

3. CONCRETE SHALL HAVE SAND, COARSE AGGREGATES PER ASTM C-33 OR C-330 AS SCHEDULED, TYPE I OR III PORTLAND CEMENT PER ASTM C-150, AND SHALL COMPLY WITH THE FOLLOWING:

CLASS	28 DAY STRENGTH	COARSE AGGR.	SLUMP	AGG SIZE	USAGE
A	3000	C33	5"-7"	1.5"	FOOTINGS
B	4000	C33	3"-5"	1"	SLAB-ON-GRADE,
C	4000	C33	3"-5"	1"	COLUMNS, FORMED SLABS

4. PROVIDE THE FOLLOWING ADDITIONAL CHARACTERISTICS FOR MIXES IN EXPOSED PLAZA DECKS AND POOL STRUCTURES.

   CONCRETE EXPOSED TO FREEZE/THAW IN MOIST CONDITIONS SHALL HAVE A MINIMUM f'c OF 4500 PSI PER ACI 318, TABLE 4.2.2

   A. HIGH RANGE WATER REDUCER
   B. MAXIMUM WATER/CEMENT RATIO OF 0.45
   C. AIR CONTENT OF 5% TO 7%
   D. MAXIMUM AGGREGATE SIZE OF 3/4"
   E. ADMIXTURES WITH NO CHLORIDES

5. JOINTS NOT SHOWN SHALL BE MADE AND LOCATED TO LEAST IMPAIR STRENGTH AND APPEARANCE OF STRUCTURE, AS APPROVED. NO HORIZONTAL JOINTS SHALL BE PERMITTED IN CONCRETE EXCEPT WHERE THEY NORMALLY OCCUR OR WHERE NOTED. VERTICAL JOINTS SHALL OCCUR NEAR CENTERS OF SPANS AT LOCATIONS APPROVED.

6. JOINTS BETWEEN PIERS AND PIER CAPS, FOOTINGS AND WALLS OR COLUMNS BE PREPARED BY ROUGHENING THE SURFACE OF THE CONCRETE IN AN APPROVED MANNER SO THAT THE AGGREGATE SHALL BE EXPOSED UNIFORMLY LEAVING NO LATINATE, LOOSENED PARTICLES OR DAMAGED CONCRETE.

7. U.N.O., OR AS INDICATED ON ARCHITECTURAL DRAWINGS, PROVIDE CHAMFERS AT EXPOSED EDGES OF CONCRETE.

8. ALL SLEEVES, INSERTS, ANCHORS, AND EMBEDDED ITEMS REQUIRED FOR ADJOINING WORK SHALL BE VERIFIED BY CONTRACTOR AND SHALL BE IN PLACE PRIOR TO CONCRETING.

EXPANSION BOLTS

1. EXPANSION BOLTS SHALL BE ONE OF THE FOLLOWING:

   A. KWIK BOLT BY HILTI FASTENING SYSTEMS
   B. WEJ-IT ANCHOR BOLT BY WEJ-IT CORPORATION

2. EXPANSION BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER AND THE FOLLOWING:

   A. DRILL HOLES PERPENDICULAR TO FACE OF CONCRETE.
   B. HOLES SHALL BE SAME NOMINAL DIAMETER AS BOLT. DO NOT ENLARGE HOLE DIAMETER DURING DRILLING PROCESS.

3. PRIOR TO DRILLING, LOCATE EXISTING REINFORCING STEEL IN CONCRETE. POSITION BOLTS TO NOT CONFLICT WITH REINFORCING. WHERE BOLTS MUST BE RELOCATED MORE THAN 1 1/2 INCHES FROM THEORETICAL LOCATION, NOTIFY THE ARCHITECT. ADJUST SIZES OF CONNECTION MATERIALS TO ALLOW FOR REPOSITIONED BOLTS.

4. HOLES IN CONNECTION PLATES MAY BE OVERSIZED PROVIDED WELDED PLATE WASHERS WITH PROPER HOLE DIAMETERS ARE PROVIDED. WELDS SHALL DEVELOP FULL CAPACITY OF BOLT.

STRUCTURAL MASONRY

1. MATERIALS SHALL COMPLY WITH THE FOLLOWING:

   A. HOLLOW MASONRY UNITS - ASTM C90, LIGHTWEIGHT, TYPE NI, MINIMUM COMPRESSIVE STRENGTH OF 2500 ON NET AREA.
   B. MORTAR - ASTM C 270, TYPE S, WITH MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI.
   C. COARSE GROUT - ASTM C 476, WITH MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI, MAXIMUM AGGREGATE SIZE OF 1/2 INCH.
   D. Z TIES - 3/16 INCH DIAMETER, ASTM A 82, GALVANIZED.

2. MASONRY DESIGN BASED ON PRISM STRENGTH OF FM = 2500 PSI.

3. WALL VERTICAL REINFORCING SHALL BE AS FOLLOWS:

   A. SEE TYPICAL DETAILS SHEET S4.01
   B. 2-#5 VERTICAL FULL HEIGHT EACH SIDE OF OPENINGS.
   C. 2-#5 VERTICAL FULL HEIGHT AT CORNERS AND ENDS OF WALLS.
   D. VERTICAL BARS SHALL BE CENTERED IN CELLS.
   E. BAR LAP SPLICES SHALL BE 60 BAR DIAMETERS, MINIMUM.
   F. SPLICES SHALL BE STAGGERED WITH NO MORE THAN HALF OF BARS LAPPED WITHIN LENGTH OF LAPS.

4. WALL HORIZONTAL REINFORCING SHALL BE AS FOLLOWS:

   A. PLACED CONTINUOUSLY IN BOND BEAMS AND GROUTED SOLIDLY IN PLACE.
   B. LAP SPLICES SHALL BE 60 BAR DIAMETERS, MINIMUM.
   C. LAP SPLICES SHALL HAVE BARS TIED TOGETHER.

5. JOINT REINFORCING SHALL BE AS FOLLOWS:

   A. EXTRA HEAVY DUTY FOR 12" WALLS "DUR-O-WALL", TRUSS TYPE, AT 16 INCHES ON CENTER, UNLESS NOTED OTHERWISE.
   B. PROVIDE TWO HORIZONTAL WIRES IN BLOCK MYTHES.
   C. LAP SPLICES SHALL BE 14 INCHES MINIMUM WITH HORIZONTAL WIRES LAPPED SIDE BY SIDE.

6. TIES FOR MASONRY PILASTERS AND BEAMS SHALL BE AS FOLLOWS:

   A. Z TIES SHALL HAVE 2 INCH LONG 90 DEGREE HOOKS EACH END.
   B. PLACE Z TIES AT 16 INCHES ON CENTER IN VERTICAL BRICK SURFACES.
   C. PLACE Z TIES AT 8 INCHES ON CENTER IN HORIZONTAL BRICK SURFACES USED FOR BEAM SOFFITS. HOOK TIES AROUND 1/4 INCH DIAMETER GALVANIZED RODS PLACED IN HOLES IN BRICK. Z TIES SHALL BE 6 INCHES LONG, MINIMUM.

7. PROVIDE 1" CLEAR COVER BETWEEN TIES OR LONGITUDINAL REINFORCING AND INSIDE FACE OF MASONRY AT GROUTED BEAMS, COLUMNS, AND PILASTERS.

8. PLACE INSULATION, AS SPECIFIED, IN THE CAVITY IN 16" STRIPS BETWEEN JOINT REINFORCEMENT. INSULATION SHALL NOT EXTEND INTO GROUTED PILASTER OR BOND BEAM SECTIONS.

9. SPACE NON-DIMENSIONED WALL PILASTER EQUALLY BETWEEN THOSE DIMENSIONED, WITHOUT EXCEEDING 12'-0" ON CENTER.

10. ADJUSTABLE JOINT REINFORCING SHALL NOT BE USED IN CAVITY WALLS.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL ROLLED SHAPES SHALL CONFORM TO THE FOLLOWING:

W AND WT SHAPES:	ASTM A572, GRADE 50, WITH SPECIAL REQUIREMENTS PER AISC TECHNICAL BULLETIN #3 DATED MARCH 1997, UNLESS NOTED OTHERWISE. ASTM A36, WHERE NOTED
CHANNELS, ANGLES, M AND S SHAPES:	ASTM A36, EXCEPT WHERE NOTED.

2. PLATES SHALL BE ASTM A36 EXCEPT PLATES USED IN MOMENT CONNECTIONS AND AS STIFFENERS SHALL BE THE SAME MATERIAL AND GRADE AS THE MEMBERS CONNECTED TO THE PLATES.

3. ALL STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A-501 OR A-53.

4. ALL STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A-500, GRADE B.

5. ANCHOR RODS (ANCHOR BOLTS) SHALL BE GRADE 36, UNLESS INDICATED OTHERWISE.

6. SHEAR CONNECTORS SHALL BE HEADED STUDS, SIZE AS INDICATED, COMPLYING WITH ASTM A108.

7. FABRICATION OF STRUCTURAL STEEL SHALL BE PER AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.

8. REFER TO ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS METALS NOT SHOWN ON STRUCTURAL DRAWINGS.

9. SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT DETAILED IS PROHIBITED WITHOUT PRIOR APPROVAL.

10. ALL STEEL BEAMS SHALL BE ERECTED WITH CAMBER AS INDICATED ON DRAWINGS. ALL BEAMS SHALL BE ERECTED WITH NATURAL CAMBER UP.

11. SHORING OF GRAVITY LOADS FOR COMPOSITE STEEL BEAMS IS NOT PERMITTED UNLESS INDICATED ON THE PLANS OR WITH PRIOR APPROVAL.

12. STEEL SHELF ANGLES FOR MASONRY SHALL COMPLY WITH THE FOLLOWING:

   A. CONTINUOUS WITH 1/4 INCH JOINTS AT MAXIMUM SPACING OF 20 FEET.
   B. JOINTS SHALL BE PLACED AT MIDPOINT BETWEEN SUPPORTS.
   C. DISCONTINUOUS AT CORNERS WITH ONE HORIZONTAL LEG EXTENDED. PROVIDE 1/4 INCH GAP AT ADJACENT ANGLE.
   D. HOT DIPPED GALVANIZED.

METAL ROOF DECK

1. STEEL ROOF DECK SHALL BE VULCRAFT OR EQUAL, 3 INCH, 18 GAUGE, TYPE 3NIA (ACOUSTIC), PHOSPHATIZED/PAINTED. SHEET STEEL SHALL BE PER ASTM A611.

2. ERECT METAL ROOF DECK PER MANUFACTURER'S RECOMMENDATIONS. PLACE DECK IN THREE SPAN LENGTHS.

3. DECK CONNECTIONS SHALL COMPLY WITH THE FOLLOWING, UNLESS OTHERWISE INDICATED ON PLAN:

   A. AT EACH SUPPORT - PUDDLE WELDS AT 18 INCHES ON CENTER.
   B. AT SIDE LAPS - SCREWS MIDWAY BETWEEN SUPPORTS.
   C. EDGE PARALLEL TO DECK - PUDDLE WELDS AT 12 INCHES ON CENTER.
   D. SCREWS - #12 - 14X3/4" HWH #2 BIT TEKS
   E. PUDDLE WELDS - FULL FUSION 5/8" DIAMETER. DECK METAL FULLY INTACT AFTER WELDING.

4. ROOF DECK SHALL NOT SUPPORT SUSPENDED CEILINGS, FIXTURES, DUCTS, OR OTHER SYSTEMS.

5. JOISTS AND JOIST GIRDERS SHALL RECEIVE SHOP PRIMER, RED OXIDE.

2000170-00 02-20-02

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FEBRUARY 20, 2002

REVISIONS

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2-20-02

JOB NUMBER
0012

JOB NAME
ADDISON ATHELTIC CLUB EXPANSION & LEISURE POOL

SHEET DESCRIPTION
GENERAL NOTES

SHEET NUMBER
S1.01