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DETAIL & NOTES

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# COUNTY OF DALLAS, TEXAS DEPARTMENT OF PUBLIC WORKS

MCIP ROADWAY IMPROVEMENTS TOWN OF ADDISON, TEXAS

TOA NO. 2002-102

MCIP PROJECT NO. 10305

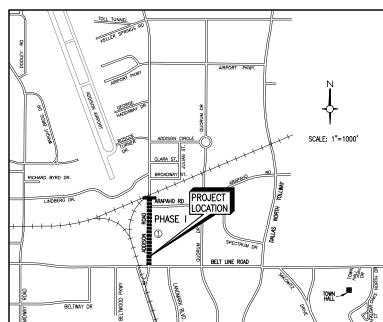
ADDISON RD - FROM BELT LINE RD TO ARAPAHO RD



NO EQUATION
NO EXCEPTION
NO RAILROAD

LENGTH DF PROJECT 1500 FT.

CONSTRUCTION TYPE

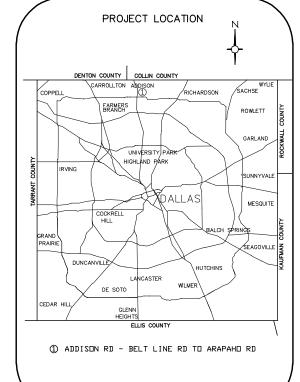




THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.

DATE: 10/31/06

#### DESIGN SPEED: 40 MPH



#### COUNTY OFFICIALS

COUNTY JUDGE JAMES FOSTER

COMMISSIONER DISTRICT NO. 1

MAURINE DICKEY

COMMISSIONER DISTRICT NO. 2
MIKE CANTRELL

COMMISSIONER DISTRICT NO. 3

JOHN WILEY PRICE

COMMISSIONER DISTRICT NO. 4 KENNETH A. MAYFIELD This record drawing is a compilation of the sealed engineering drawing for this project; modified by addenda, change orders and information furnished by the contractor. The information shown on the record drawings that was provided by the contractor or others not associated with the design engineer cannot be verified for accuracy or completeness. This original sealed drawings are on file at the offices of Birkhoff, Hendricks & Carter, L.L.P.

BY <u>J.W.B.</u> DATE <u>05/04/2010</u>

# 

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#### SUMMARY OF ABBREVIATIONS AND DEFINITIONS

	GENERAL TOPOGRAPHY & HO	rizontal/ vertic	AL CONTROL	TREES & BUSHES				
ABBREVIATION	DEFINITION	ABBREVIATION	DEFINITION	ABBREVIATION	DEFINITION			
B.C.	BACK OF CURB	P.I.	POINT OF INTERSECTION	B.D.	BOIS D'ARC TREE			
B.–B. €	BACK TO BACK	"{	PROPERTY LINE	B.T.	BLACK THORN TREE			
	CENTERLINE	P.P.	POWER POLE	C.B.	CHINABERRY TREE			
C.M.A.P.	CORRUGATED METAL ARCH PIPE	P.R.C.	POINT OF REVERSE CURVATURE	CDR.	CEDAR TREE			
C.M.P.	CORRUGATED METAL PIPE	P.T.	POINT OF TANGENCY	C.E.	CEDAR ELM TREE			
C.I.	CAST IRON	P.V.C.	PLASTIC PIPE	C.M.	CRAPE MYRTLE TREE			
C.O.	CLEANOUT	P.V.I.	POINT OF VERTICAL INTERSECTION	C.P.	CHINESE PISTACHIO TREE			
CONN.	CONNECTION	R.	RADIUS	C.W.	COTTONWOOD TREE			
CULV.	CULVERT	R.C.	REINFORCED CONCRETE	CYP.	CYPRESS TREE			
D.I.P.	DUCTILE IRON PIPE	R.C.P.	REINFORCED CONCRETE PIPE	H.B.	HACKBERRY TREE			
EL.	ELEVATION	R.C.C.P.	REINF. CONCRETE CYLINDER PIPE	HICK.	HICKORY TREE			
ELEC.	BURIED ELECTRICAL POWER LINE	R.O.W.	RIGHT OF WAY	H.T.	HAWTHORN TREE			
ENC.	ENCASEMENT	SAN. SEW. OR S.S.	SANITARY SEWER	JAP.	JAPONICA TREE			
E.P.	EDGE OF PAVEMENT	S.H. §	LAWN SPRINKLER HEAD	LIG.	LIGUSTRUM BUSH			
ESMT.	EASEMENT	Į į	SURVEY LINE	L.O.	LIVE OAK TREE			
F.−F.	FACE TO FACE	S.S.	SANITARY SEWER	LOC.	LOCUST TREE			
F.H.	FIRE HYDRANT	S.S.M.H.	SANITARY SEWER MANHOLE	MAG.	MAGNOLIA TREE			
F.M.	SANITARY SEWER FORCE MAIN	ST. SEW.	STORM SEWER	MAP.	MAPLE TREE			
F.L.	FLOW LINE	S.V.B.	LAWN SPRINKLER VALVE BOX	MSQ.	MESQUITE TREE			
G.L.M.	GAS LINE MARKER	T.B.	TELEPHONE CABLE BOX	MIM.	MIMOSA TREE			
G.M.	GAS METER	T.C.	TOP OF CURB	MUL.	MULBERRY TREE			
G.V.	GAS VALVE	TEL. T.M.H.	BURIED TELEPHONE CABLE	NAN.	NANDINA BUSH			
GUT.	GUTTER	T.M.H.	TELEPHONE CABLE MANHOLE	PEC.	PECAN TREE			
HDWL.	HEADWALL	T.S.B.	TRAFFIC SIGNAL BOX	PER.	PERSIMMON TREE			
I.P.F. OR I.R.F.	IRON PIN (ROD) FOUND	TO F.	TO FACE (OF CURB)	PHOT.	red tipped photinia bush			
L.L.	LANDSCAPING LÍGHT	TRANS.	ELECTRICAL TRANSFÖRMER	PIST.	CHINESE PISTACHIO TREE			
L.P.	LIGHT POLE	T.V.	BURIED TELEVISION CABLE	R.B.	REDBUD TREE			
L.S.	SANITARY SEWAGE LIFT STATION	T.V.B.	CABLE TELEVISION BOX	R.O.	RED OAK TREE			
L.S.T.	LANDSCAPING TIMBERS	U.G.C.M.	UNDERGROUND CABLE MARKER	S.G.	SWEET GUM TREE			
M.B.	MAILBOX	U.P.M.	UNDERGROUND PIPELINE MARKER	SYC.	SYCAMORE TREE			
M.B.G.F.	METAL BEAM GUARD FENCE	V.C.	VERTICAL CURVE	WIL.	WILLOW TREE			
M.H.	MANHOLE	W. OR W.L.	WATER LINE					
P.C.	POINT OF CURVATURE	W.M.	WATER METER					
		W.V.	WATER VALVE					

#### GENERAL NOTES:

- 1. ALL WORK AND MATERIALS SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS, AS OUTLINED IN N.C.T.C.O.G. AND TOWN OF ADDISON SPECIFICATIONS
- 2. PROPOSED MAIN LANES ALL PROPOSED 10—INCH CONCRETE REINFORCEMENT SHALL BE #4 BARS DEFORMED ON 18" CENTER EACH WAY. REINFORCED CONCRETE PAVEMENT SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 4000 P.S.I. MINIMUM 6.5 SACK MIX, MAXIMUM W/C 5.5 N.C.T.C.O.G. AGGREGATE 2—3 [ITEM 2.1.1.(C)(4)] COURSE. MAXIMUM SLUMP SLIP FORM 2 INCHES. SLUMP HAND POURED 4 INCHES.
- 3. ALL CONNECTIONS OF PROPOSED CONCRETE PAVEMENT TO EXISTING PAVEMENT SHALL INCLUDE A LONGITUDINAL BUTT JOINT WITH #5 OR #6 BAR DOWELS AS THE LOAD TRANSFER DEVICE SEE DETAIL THIS SET OF PLANS.
- 4. REASONABLE EFFORT HAS BEEN MADE TO SHOW THE LOCATION OF ALL KNOWN UNDERGROUND UTILITIES AND SERVICE LINES. HOWEVER, THE OWNER ASSUMES NO RESPONSIBILITY FOR FAILURE TO SHOW ANY OR ALL EXISTING SUBSURFACE UTILITIES OR UTILITY LINES, OR TO SHOW THEM IN THEIR EXACT LOCATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING UTILITIES, SERVICE LINES OR THE LIKE, WHICH ARE CROSSED OR EXPOSED BY THE CONSTRUCTION OPERATION.
- 5. THE CONTRACTOR SHALL FIELD VERIFY DEPTH AND HORIZONTAL ALIGNMENT OF ALL EXISTING UTILITIES (GAS LINES, WATER LINES, COMMUNICATION LINES AND SERVICE LINES) PRIOR TO CONSTRUCTION. CONFLICTS WITH PROPOSED IMPROVEMENTS SHALL BE PROVIDED IN WRITING TO THE TOWN OF ADDISON.
- 6. THE CONTRACTOR SHALL PROTECT ALL UNDERGROUND SPRINKLER SYSTEMS. ALL DAMAGE CAUSED BY CONSTRUCTION SHALL BE REPAIRED WITHIN 4B HOURS BY AN IRRIGATOR LICENSED IN THE STATE OF TEXAS. ALL COSTS SHALL BE BORNE BY THE CONTRACTOR. SEE LANDSCAPE PLANS FOR PROPOSED IRRIGATION.
- 7. UTILITY RELOCATION'S AND ADJUSTMENTS WATER METER AND WATER METER BOXES TO BE RELOCATED OR ADJUSTED SHALL BE REPLACED WITH NEW METER BOXES AND SET AT THE FINISHED GRADE ELEVATION AND WATER SERVICES RECONNECTED. ALL WATER METERS AND METER BOXES IN PROPOSED PAVEMENT AREA SHALL BE RELOCATED IN ADJACENT PARKWAY OR A NON PAVEMENT AREA, PREFERABLY BETWEEN PROPOSED BACK OF CURB AND PROPOSED SIDEWALK. EXISTING METER BOXES REMOVED SHALL BE THE PROPERTY OF THE TOWN. EXISTING WATER VALVE BOXES WITH COVERS SHALL BE REMOVED AND RESET AT THE FINISHED GRADE ELEVATION. ALL EXISTING VALVE STACKS WITH IN THE LIMITS OF THIS PROJECT SHALL BE PROVIDED WITH DEBRIS CAPS. EXISTING STORM SEWER OR SANTARY SEWER MANNHOLE FRAME AND COVER TO BE ADJUSTED TO MATCH FINISHED GRADE SHALL USE PRE CAST CONCRETE GRADE RINGS AND NON SHRINK GROUT. ANY OF THESE ITEMS THAT ARE DAMAGED BY CONSTRUCTION ACTIVITY SHALL BE REPLACED WITH ITEMS OF EQUAL OR SUPERIOR QUALITY AT THE SOLE EXPENSE OF THE CONTRACTOR. ALL WATER SERVICE LINES SHALL BE REPLACED BETWEEN WATER MAIN AND WATER METER. ALL SANITARY SEWER SERVICE LINES SHALL BE REPLACED BETWEEN WATER MAIN AND CLEANOUT WITH NEW 4 INCH SDR 35 PVC PIPE OR MATCH PIPE SIZE IF LARGER THAN 4 WICHES.
- 8. THERE SHALL BE AT LEAST 6-INCHES CLEAR DISTANCE FROM THE TOP OF THE ADJUSTED METER BOXES AND THE THE WATER METER. CONTRACTOR SHALL NOTIFY THE TOWN AND ALL OWNERS WITH METERS TO BE ADJUSTED 48 HOURS IN ADVANCE OF ANY PLANNED CONSTRUCTION. SERVICE SHALL NOT BE DISRUPTED FOR MORE THAN 4 HOURS WITHOUT THE PERMISSION FROM THE CITY ENGINEER. CONTRACTOR SHALL TAKE MEASURES NECESSARY TO RESTORE WATER SERVICE WITHIN THE 4-HOURS.
- 9. THE CONTRACTOR SHALL GIVE THE TOWN, BUSINESSES AND RESIDENTS AT LEAST 72 HOURS PRIOR NOTICE TO BEGINNING OF CONSTRUCTION AND AT LEAST 48 HOURS FOR ANY ANTICIPATED WATER OR SEWER SERVICE DISRUPTION.
- 10. CONCRETE MIX DESIGN SHALL BE SUBMITTED FOR REVIEW A MINIMUM OF 14 DAYS PRIOR TO THE POUR.
- 11. NO SEPARATE PAY ITEM FOR SUB GRADE PREPARATION UNDER DRIVE OR PARKING AREAS AND ALL COST SHALL BE INCLUDED IN THE APPROPRIATE ITEMS OF THE BID SCHEDULE.
- 12. ALL PAVEMENT REMOVAL AND REPLACEMENT SHALL BE FULL DEPTH SAW CUT ALONG NEAT LINES AS SHOWN IN THE PLANS. PROPOSED CONCRETE PAVEMENT SHALL BE CONSTRUCTED WITH LONGITUDINAL CONSTRUCTION JOINTS AT ALL CONNECTIONS TO EXISTING CONCRETE PAVEMENT. DAMAGED OR SPALDED EDGES WILL BE RECUT FULL LENGTH.
- 13. THE CONTRACTOR SHALL REMOVE AND PROPERLY DISPOSE OF ALL EXISTING CONCRETE AND H.M.A.C. PAVEMENT AS REQUIRED FOR CONSTRUCTION OF DRIVES AND PARKING AREAS. THERE SHALL BE NO SEPARATE PAYMENT FOR THIS WORK, ALL COSTS SHALL BE INCLUDED IN THE APPROPRIATE ITEMS IN THE BID SCHEDULE.
- 14. THE CONTRACTOR SHALL RESTORE ALL AREAS DISTURBED BY THE CONSTRUCTION TO THE ORIGINAL CONDITION OR BETTER. RESTORED AREAS INCLUDE BUT NOT LIMITED TO TRENCH BACK FILL, SIDE SLOPES, FENCES, IRRIGATION SYSTEMS, DRIVEWAYS, PRIVATE YARDS, SIGNS AND ROADWAYS.
- 15. THE CONTRACTOR SHALL PROTECT ALL TREES, STRUCTURES, SIGNS, MAILBOXES AND OTHER ITEMS THAT ARE NEAR AND/OR ADJACENT TO AREA OF CONSTRUCTION. PRIVATE SIGNS AND MAILBOXES TO BE REMOVED SHALL BE REPLACED OR RELOCATED WITH DIRECTION AND COORDINATION OF THE OWNER AND THE TOWN AND BE WITHIN THE GUIDE LINES OF THE TOWN ORDINANCES. ALL OTHER SIGNS REMOVED DURING CONSTRUCTION SHALL BE TEMPORARY MOUNTED AND PLACED AT PROPER LOCATIONS TO DIRECT TRAFFIC FLOW IF APPLIES.
- 16. THE CONTRACTOR SHALL MAINTAIN TWO WAY TRAFFIC DURING ROADWAY CONSTRUCTION AND MAINTAIN ACCESS TO ALL BUSINESSES BY USING EXISTING, PROPOSED OR TEMPORARY H.M.A.C. PAVEMENT. THE CONTRACTOR SHALL POST SIGNS AT DRIVEWAY'S ALONG THE CONSTRUCTION SITE TO DIRECT TRAFFIC INTO BUSINESSES. THE SIGNS SHALL BE 2 FT. BY 3 FT. WITH BLUE BACKGROUND AND WHITE LETTERS. LETTERS SHALL HAVE THE NAME OF THE BUSINESS AT THE DRIVEWAY TO BE ACCESSED.
- 17. CONTRACTOR SHALL PROVIDE PROPER BARRICADES, SIGNS, FLAG MEN AND OTHER TRAFFIC CONTROL DEVICES FOR THIS AREA.
- 18. CONTRACTOR SHALL STAKE SIDEWALK LAYOUT IN THE FIELD FOR TOWN REVIEW. CONTRACTOR SHALL MODIFY ALIGNMENT OF SIDEWALK AS DIRECTED BY THE TOWN. DETERMINATION WILL BE BASED ON AVOIDING DISRUPTION OF TREE ROOT STRUCTURE OF EXISTING LANDSCAPING.
- 19. CONTRACTOR SHALL ARRANGE AND PAY FOR SWBT, AT&T, ATMOS GAS COMPANY TO ADJUST EXISTING MANHOLES TO FINISHED GRADE OF ROADWAY.

These plans and related specifications were prepared for construction of this specific project only. Reuse of these documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.

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BY J.W.B. DATE 05/04/2010

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JLV Bully

DATE: /0/3//06



# TOWN OF ADDISON, TEXAS

# SHEET INDEX/GENERAL NOTES PHASE I

BIRKHOFF, HENDRICKS & CONWAY L. L. P.

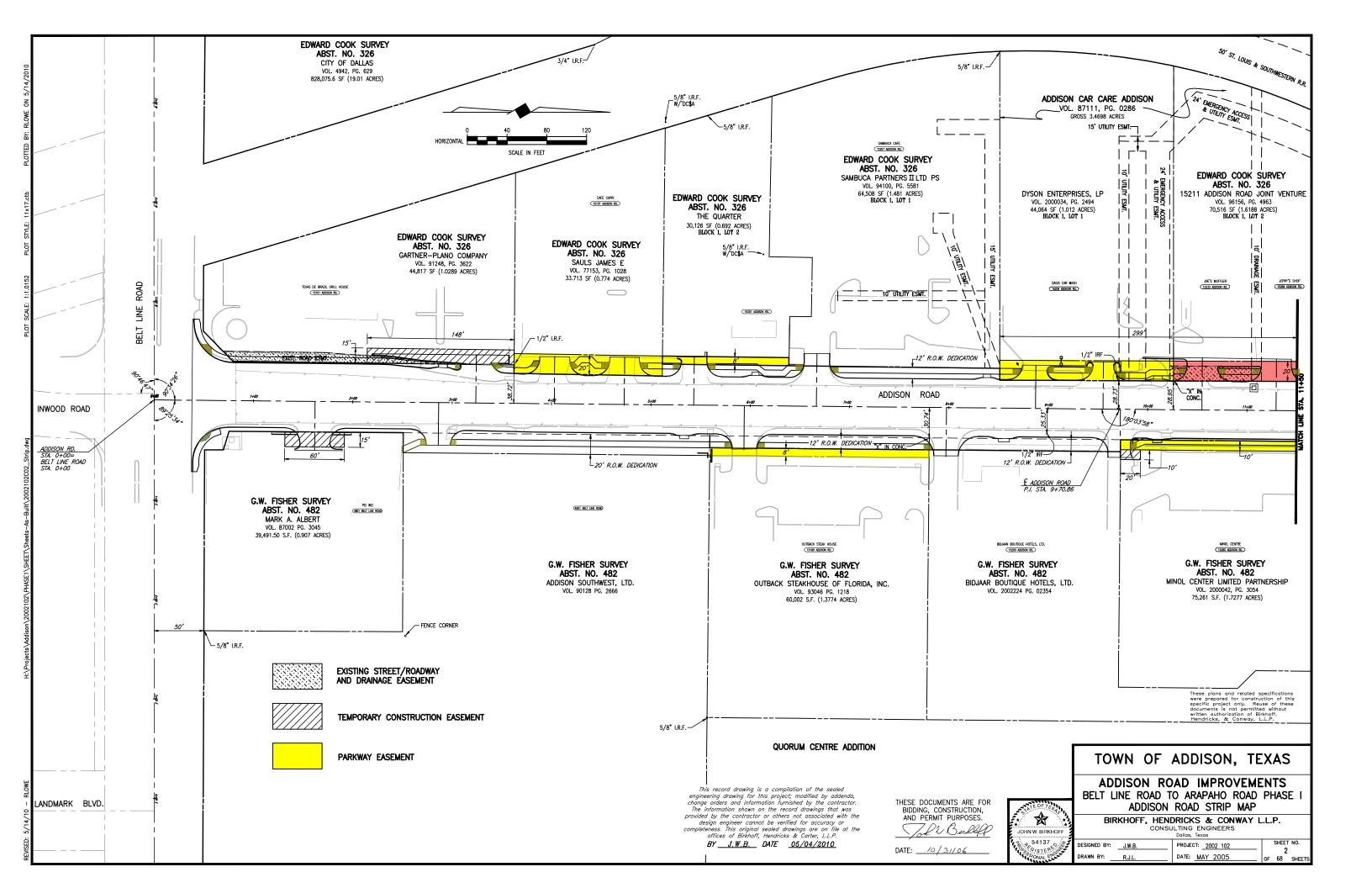
CONSULTING ENGINEERS

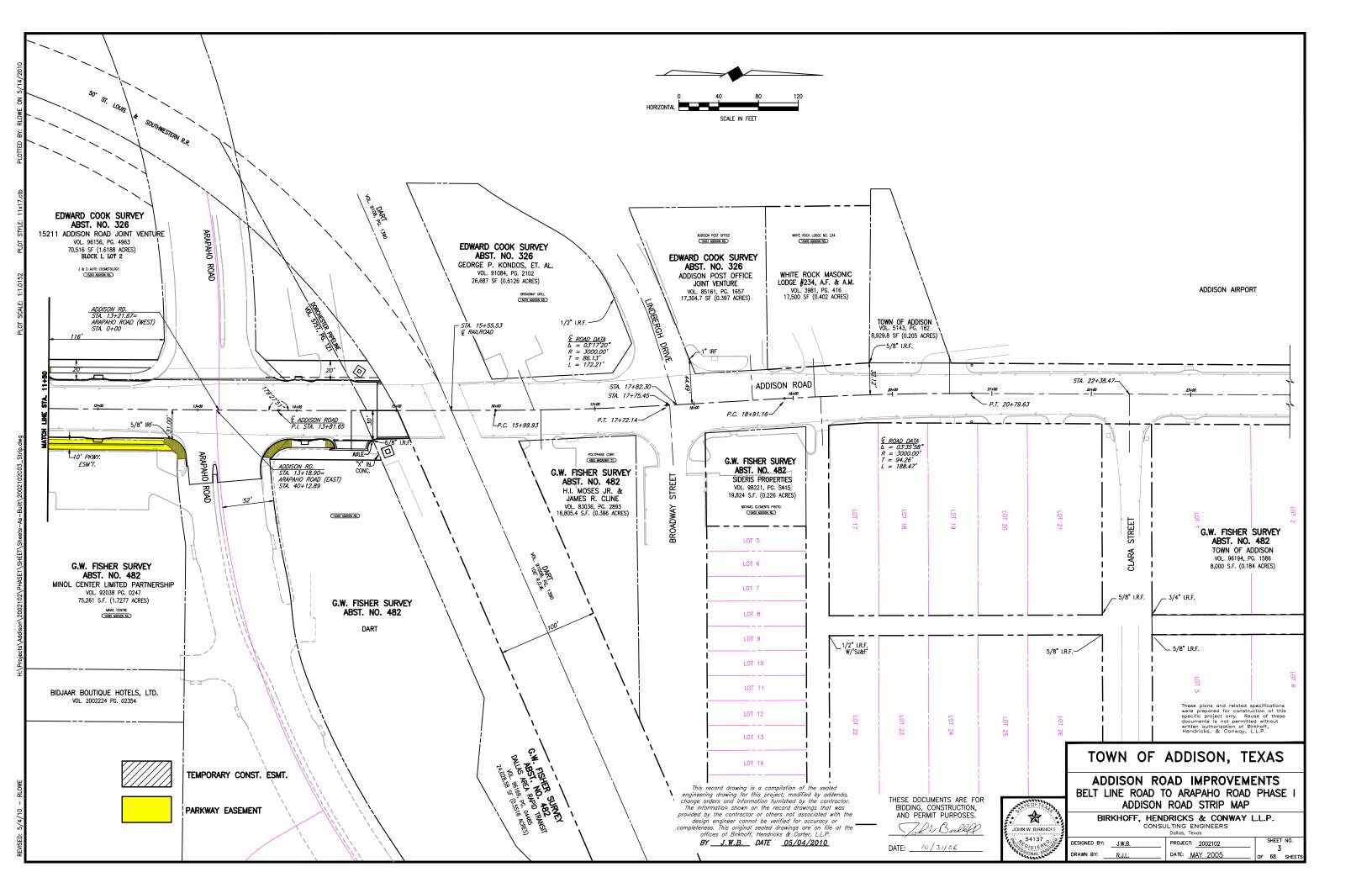
Dallas, Texas

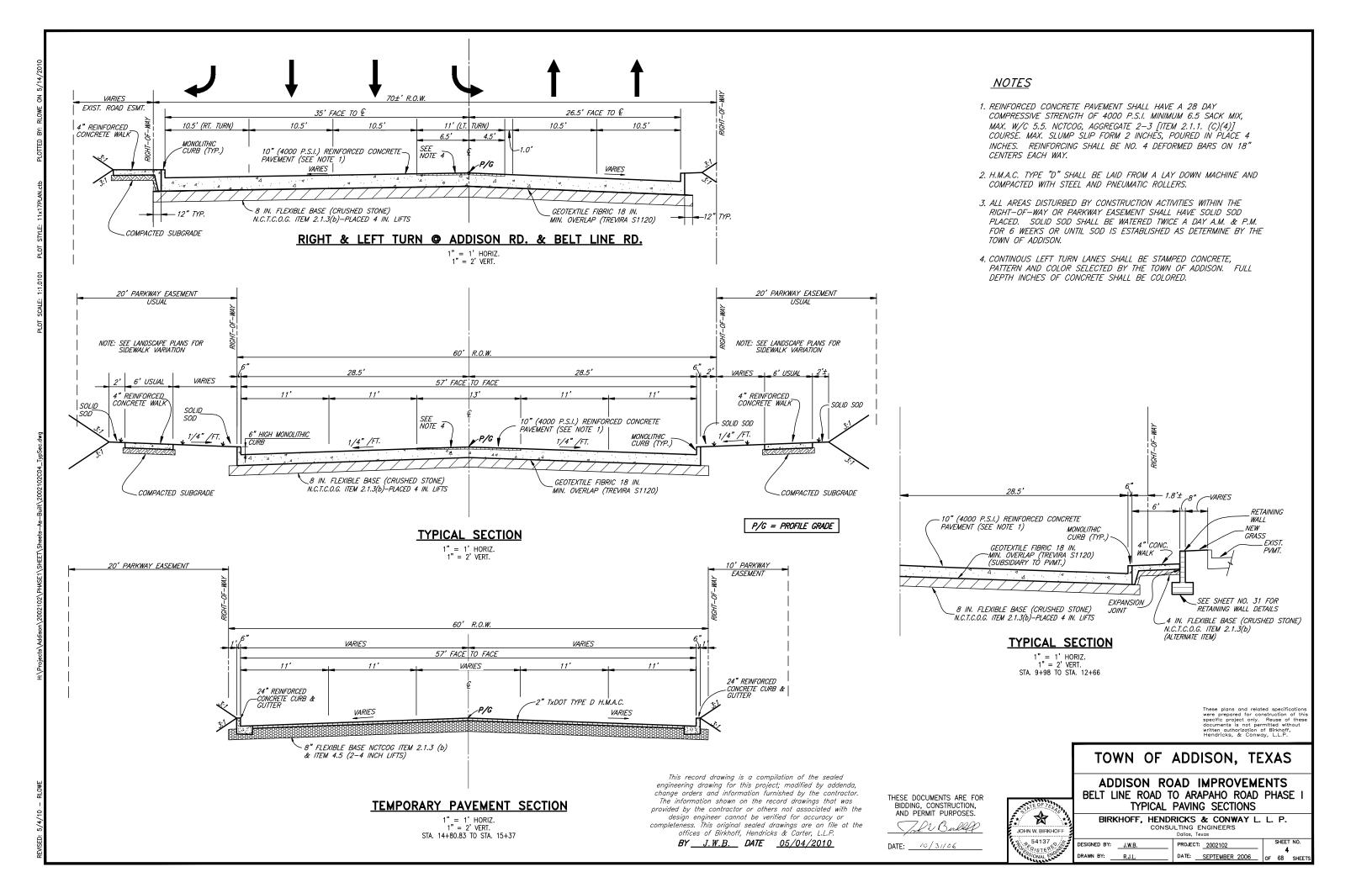
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 PROJECT:
 2002-102
 SHEET NO.

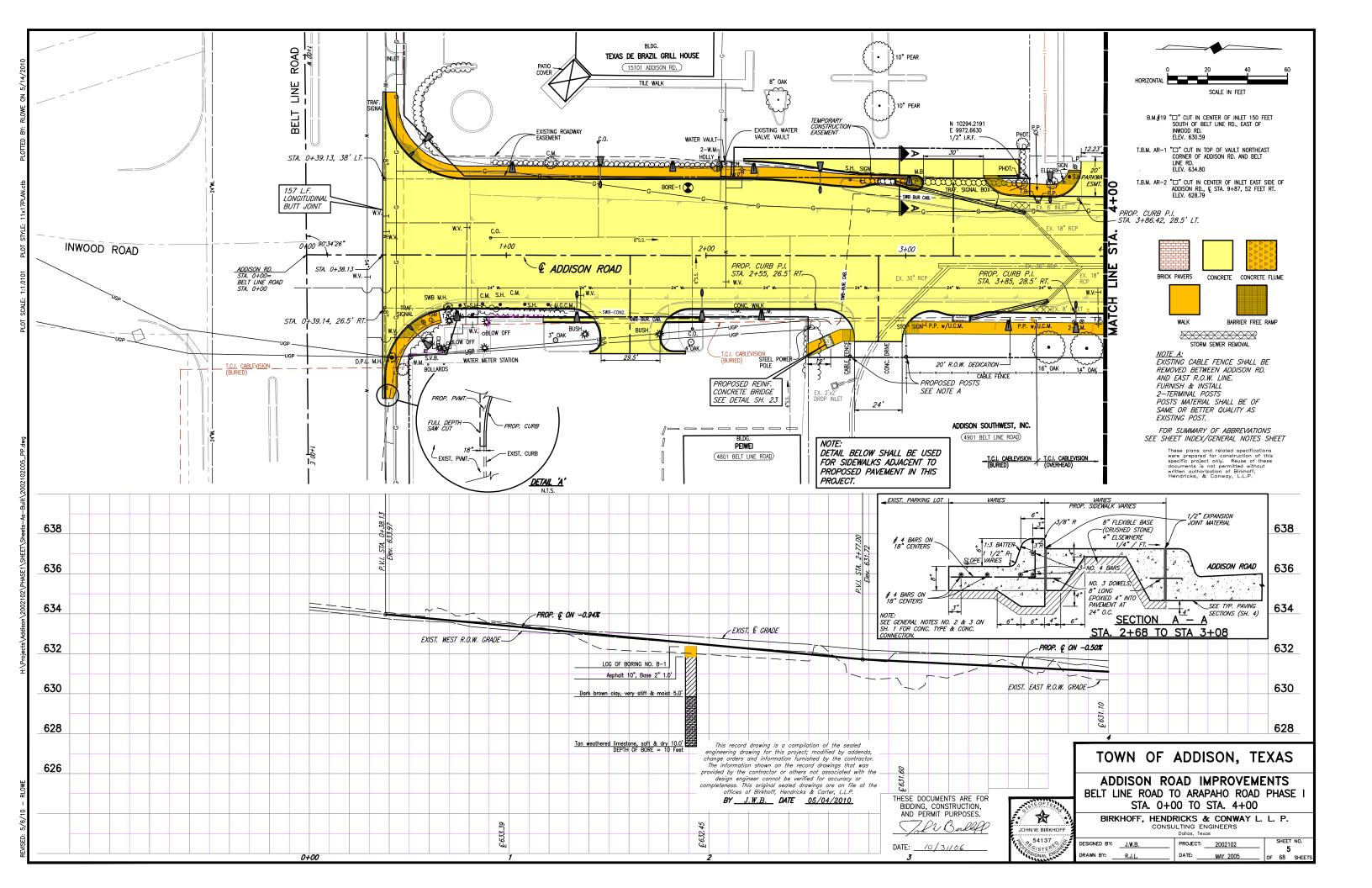
 DRAWN BY:
 R.J.L.
 DATE:
 SEPTEMBER 2006
 OF 68 SHEET

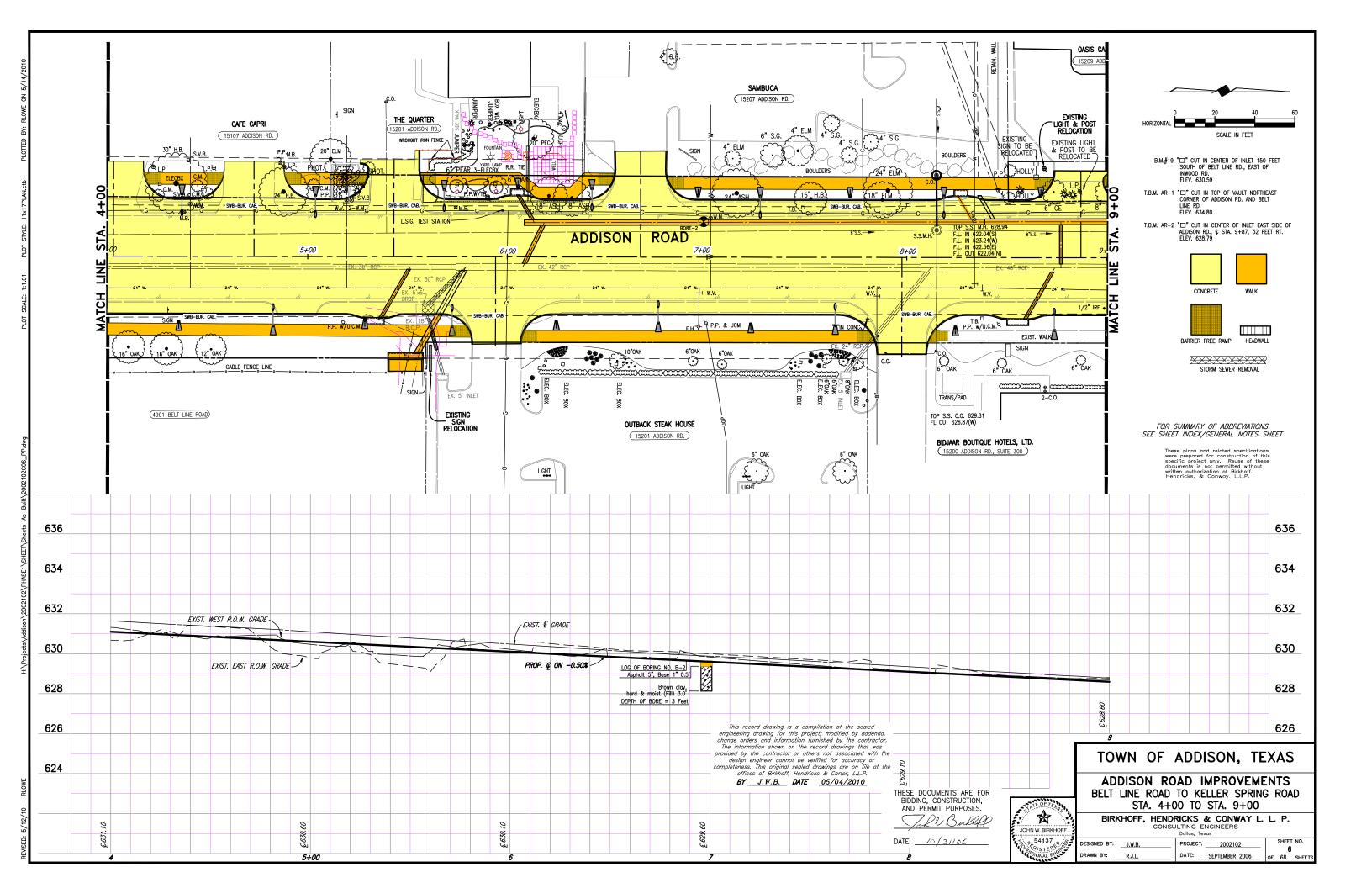
34"x22" FULL PLOT: X HALF PLOT: 11x17.PC

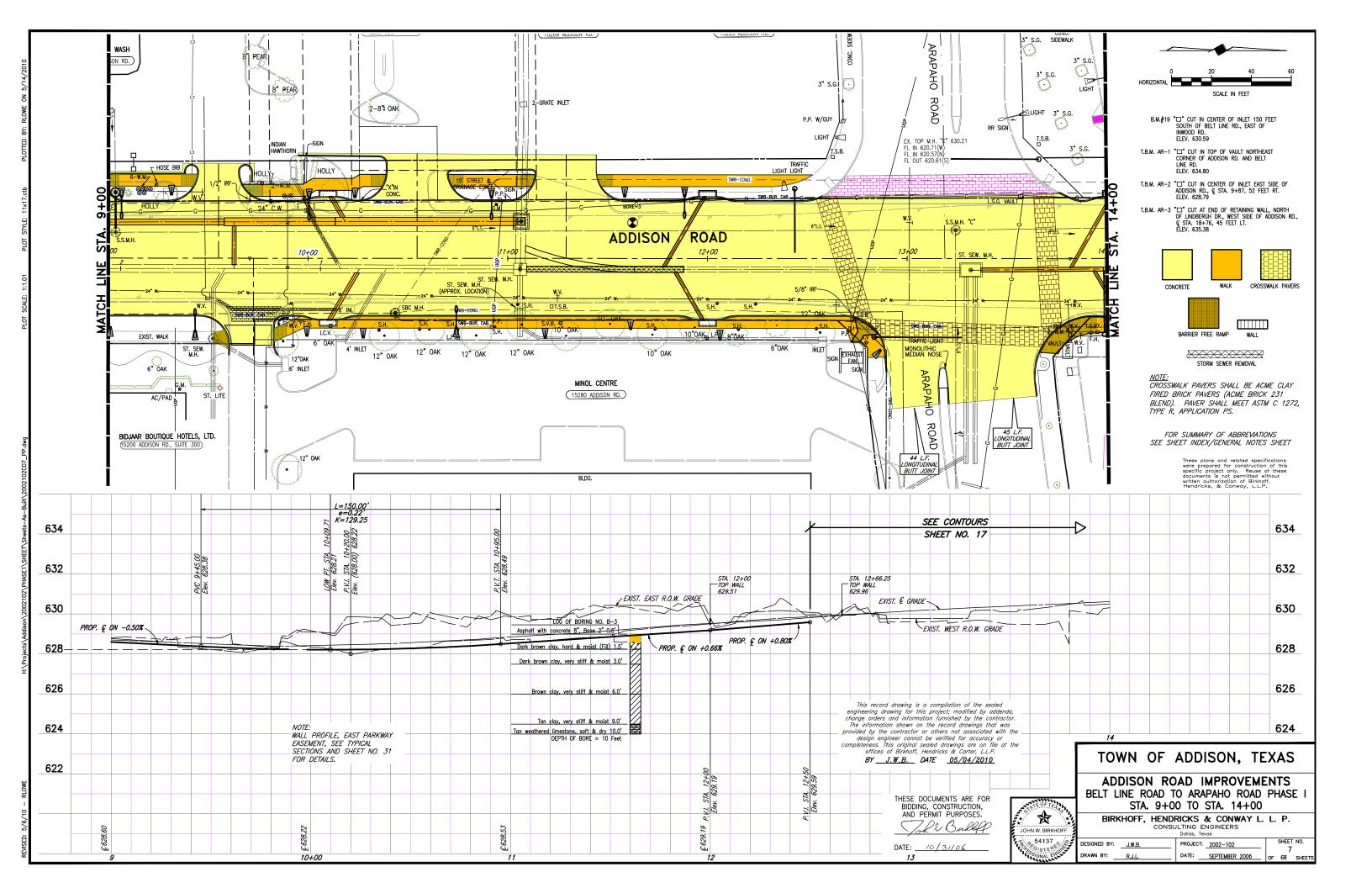


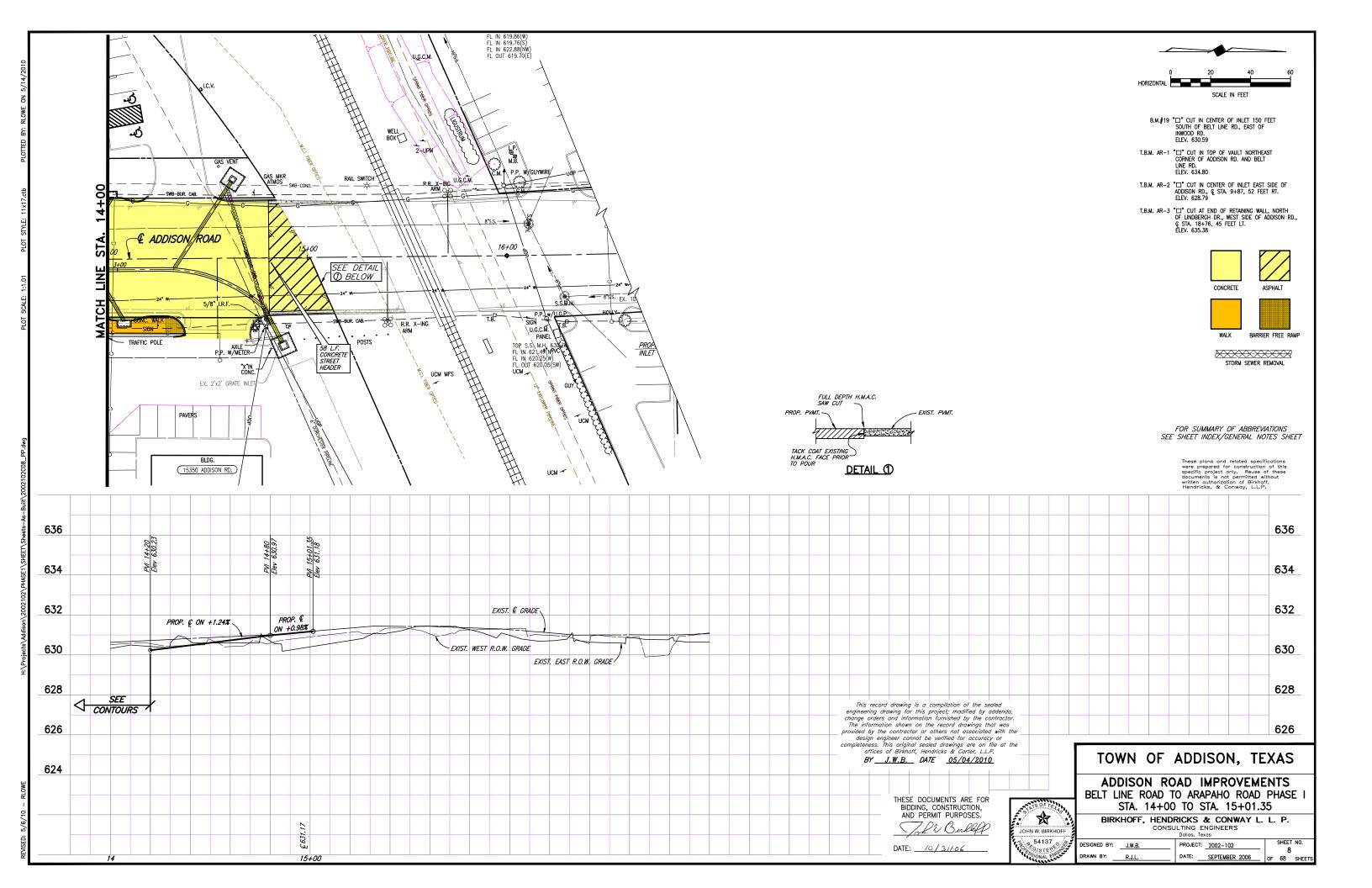


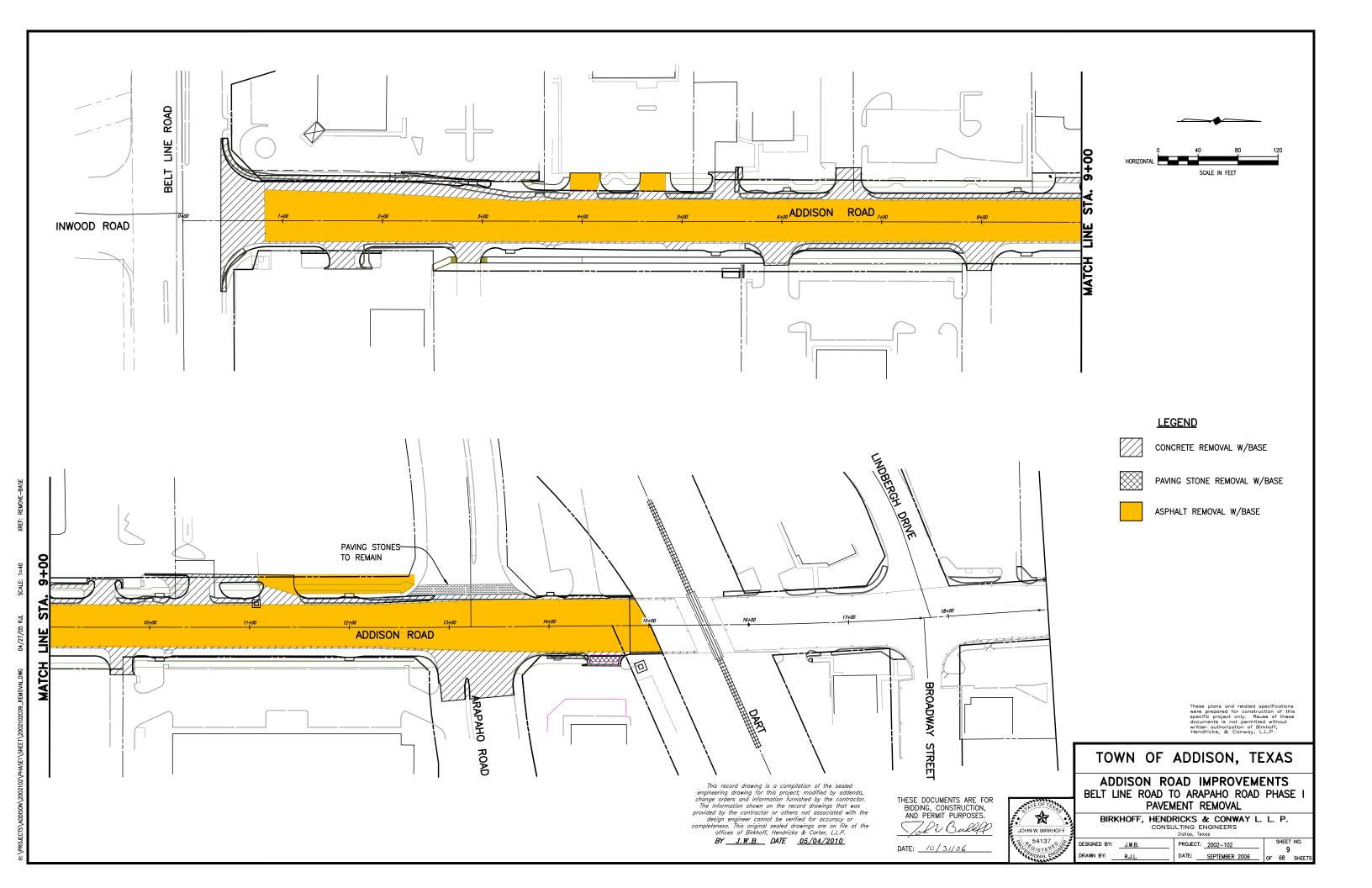


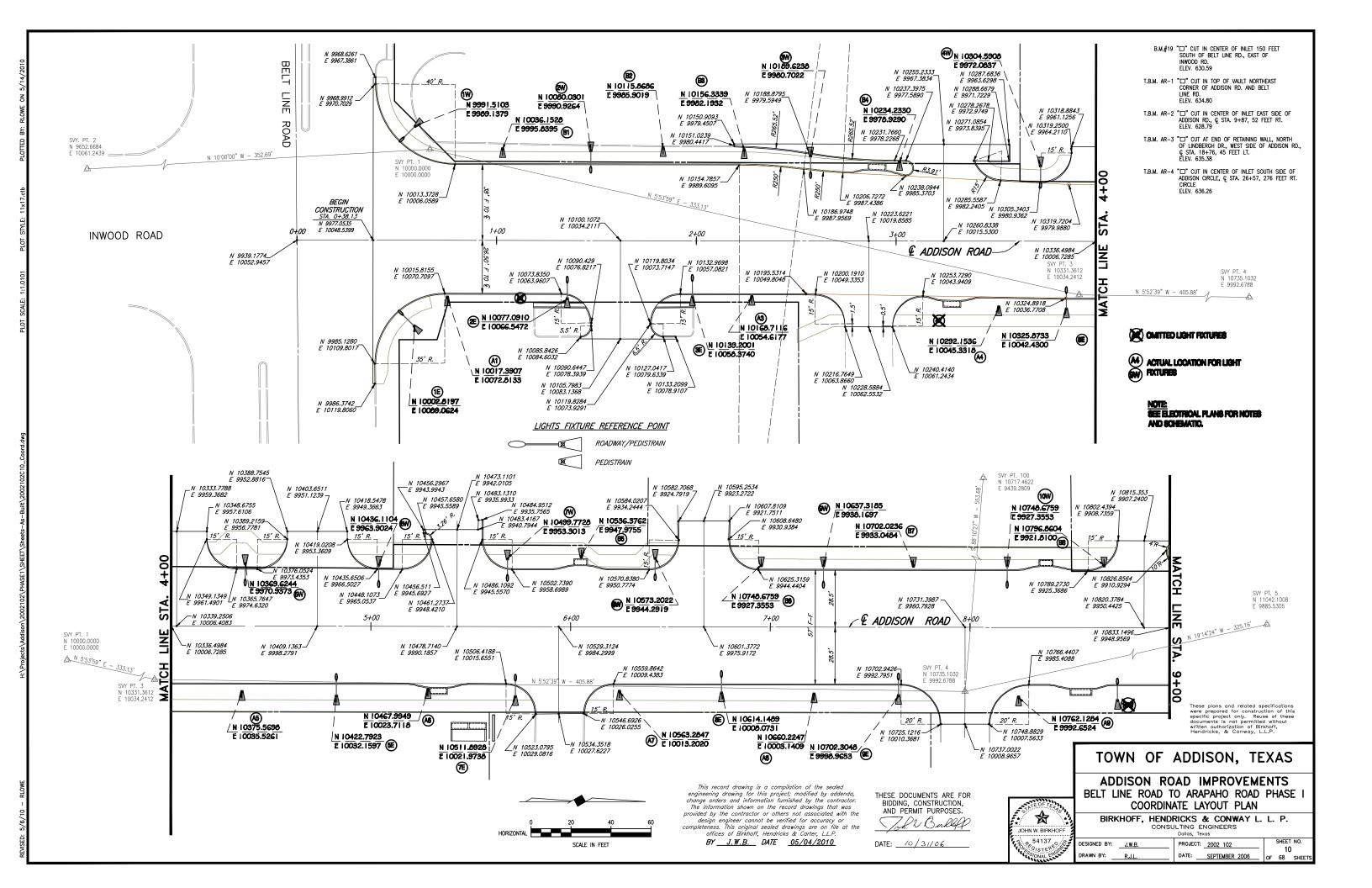


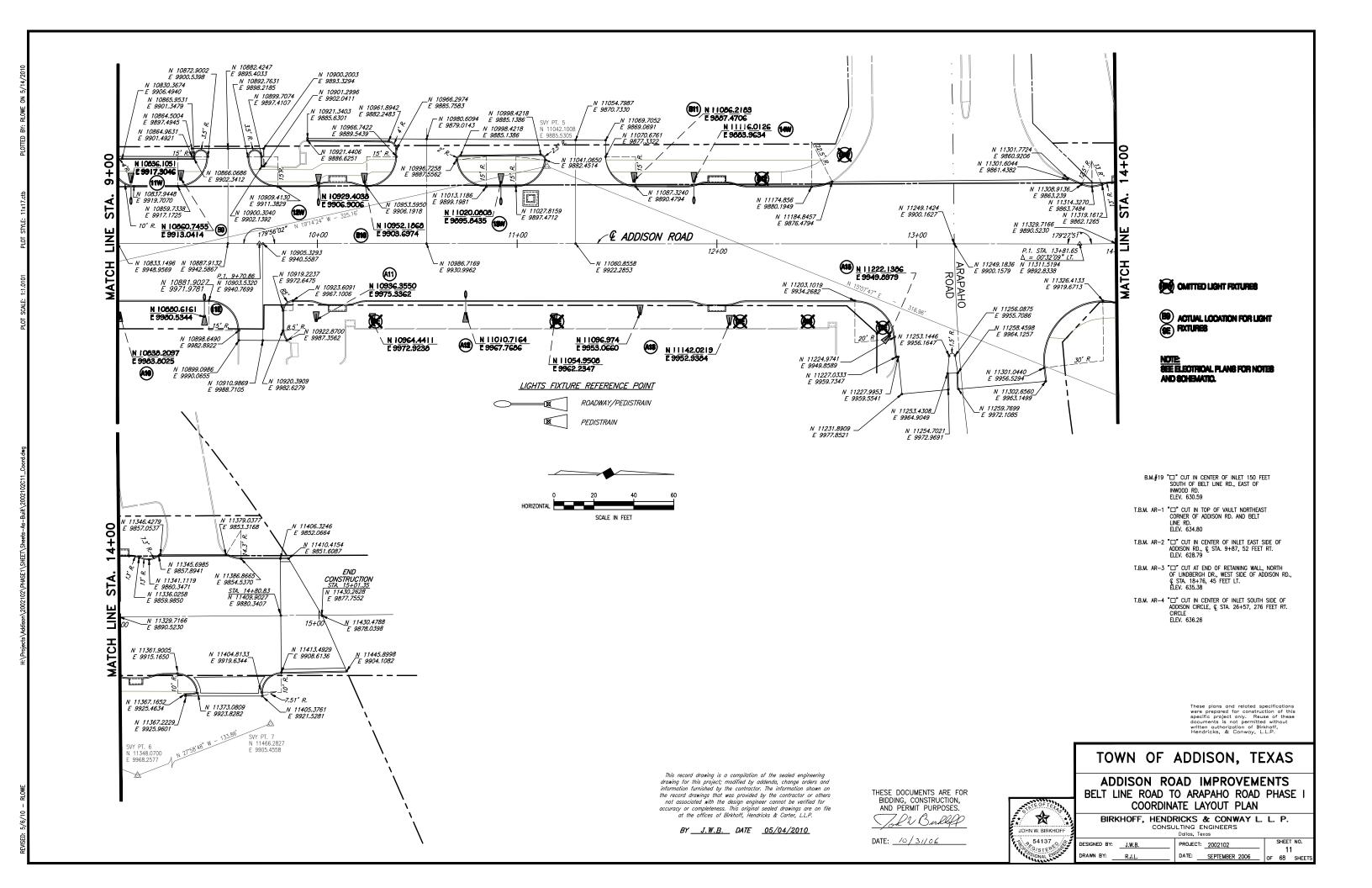


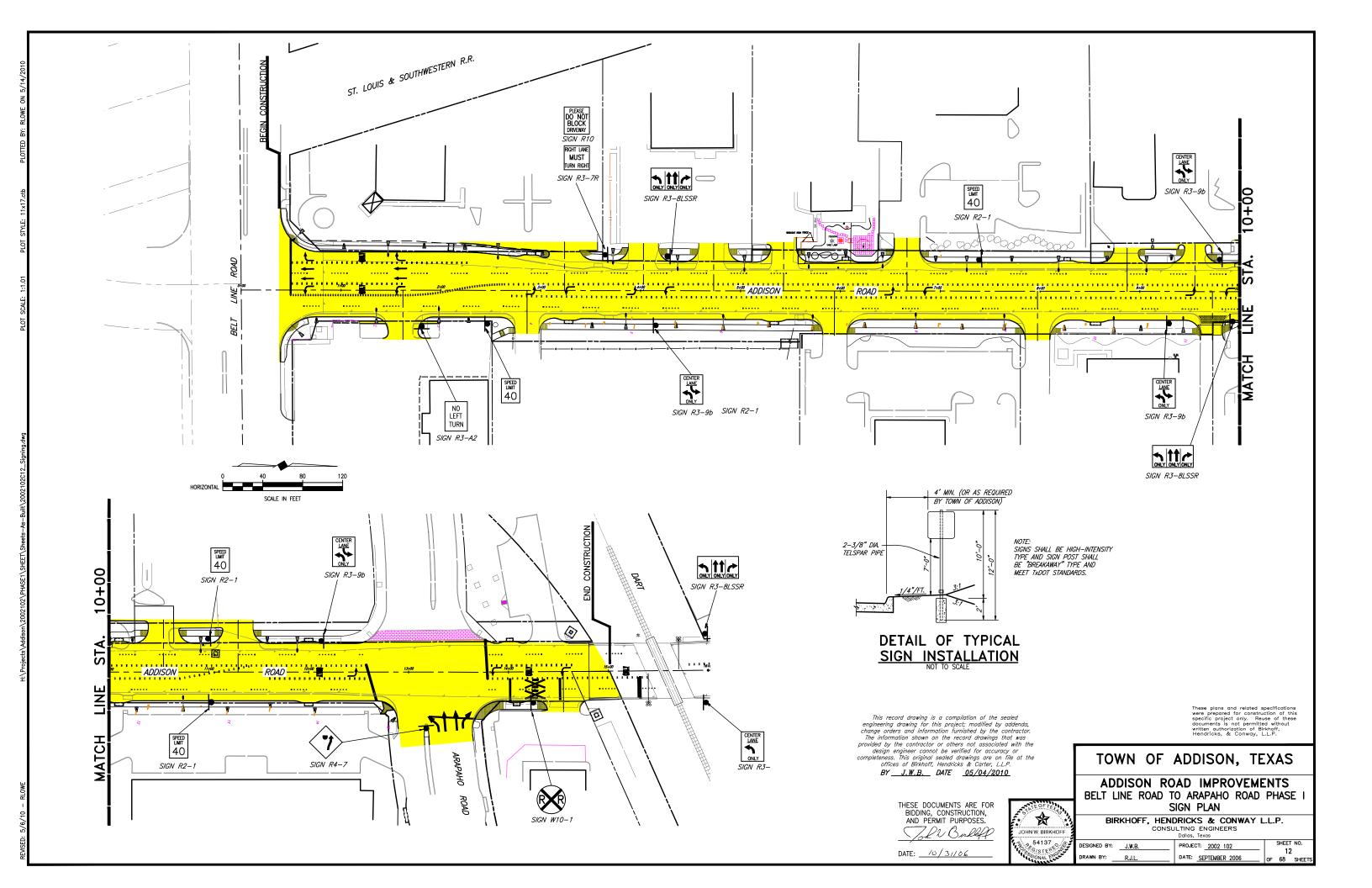


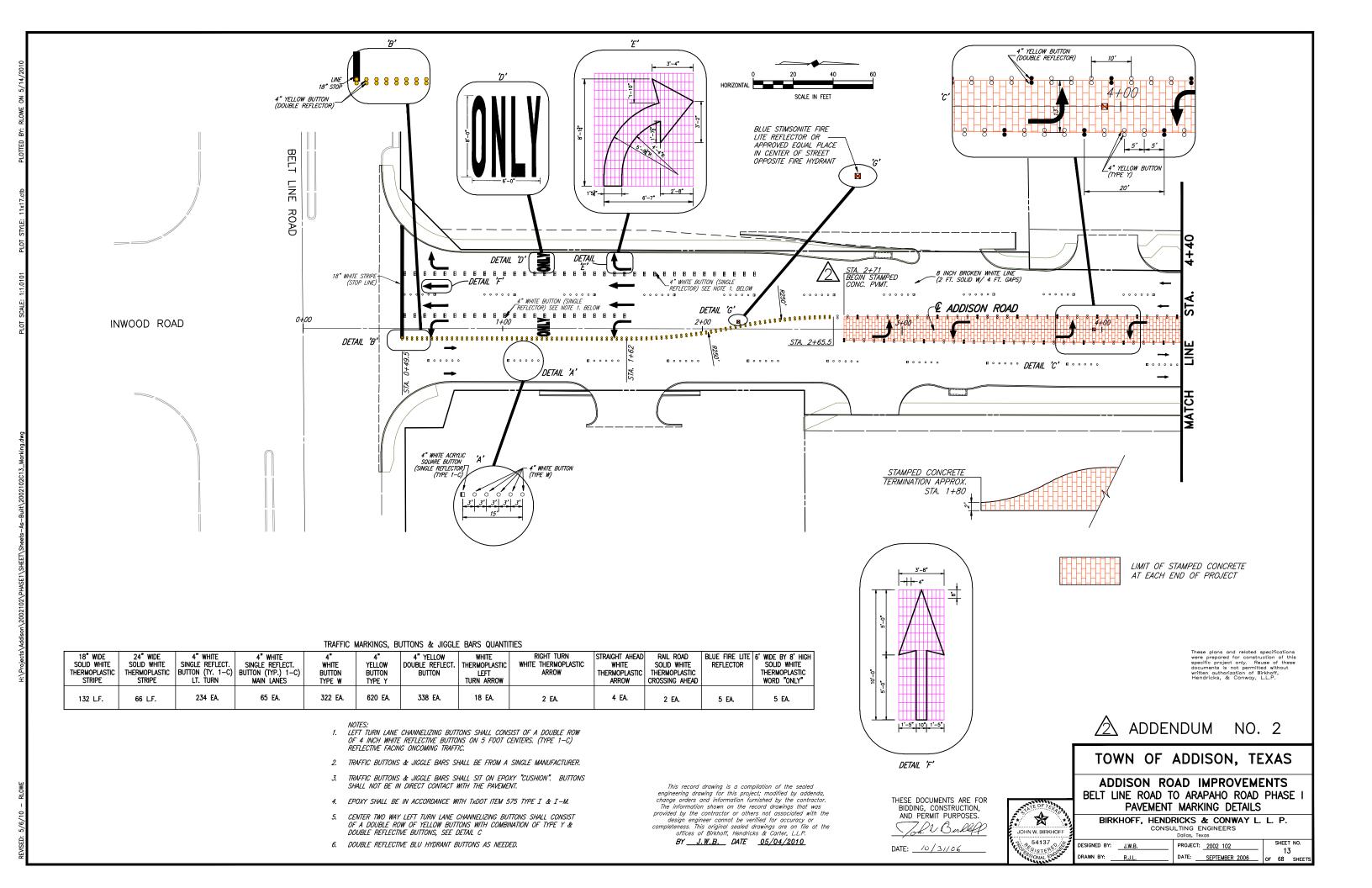


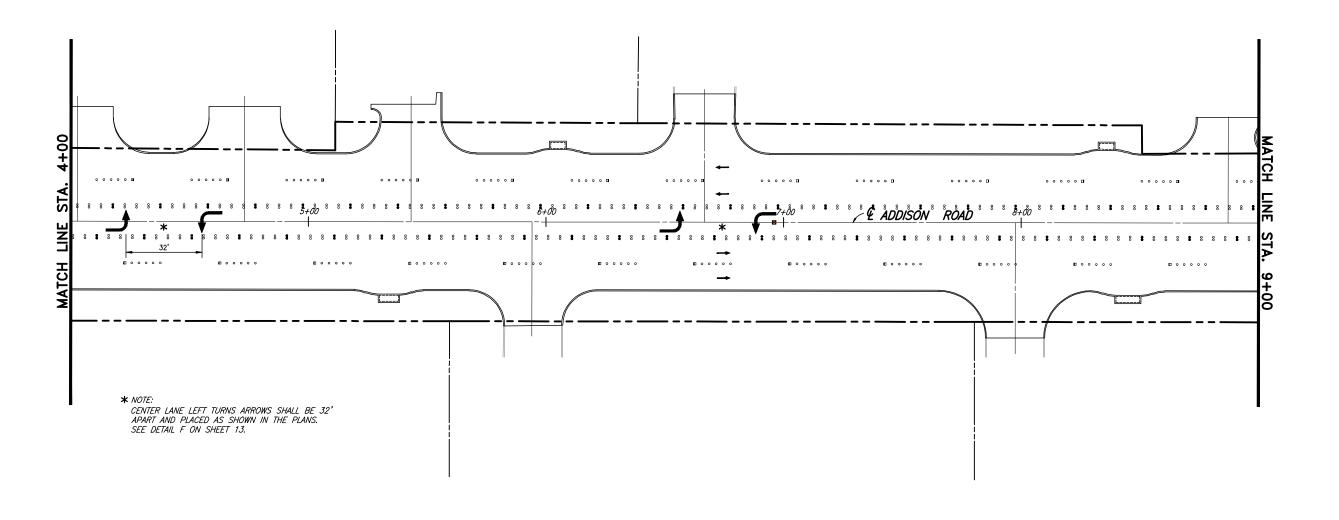












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BY J.W.B. DATE 05/04/2010

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# TOWN OF ADDISON, TEXAS

#### ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I PAVEMENT MARKING LAYOUT SHEET

BIRKHOFF, HENDRICKS & CONWAY L. L. P.

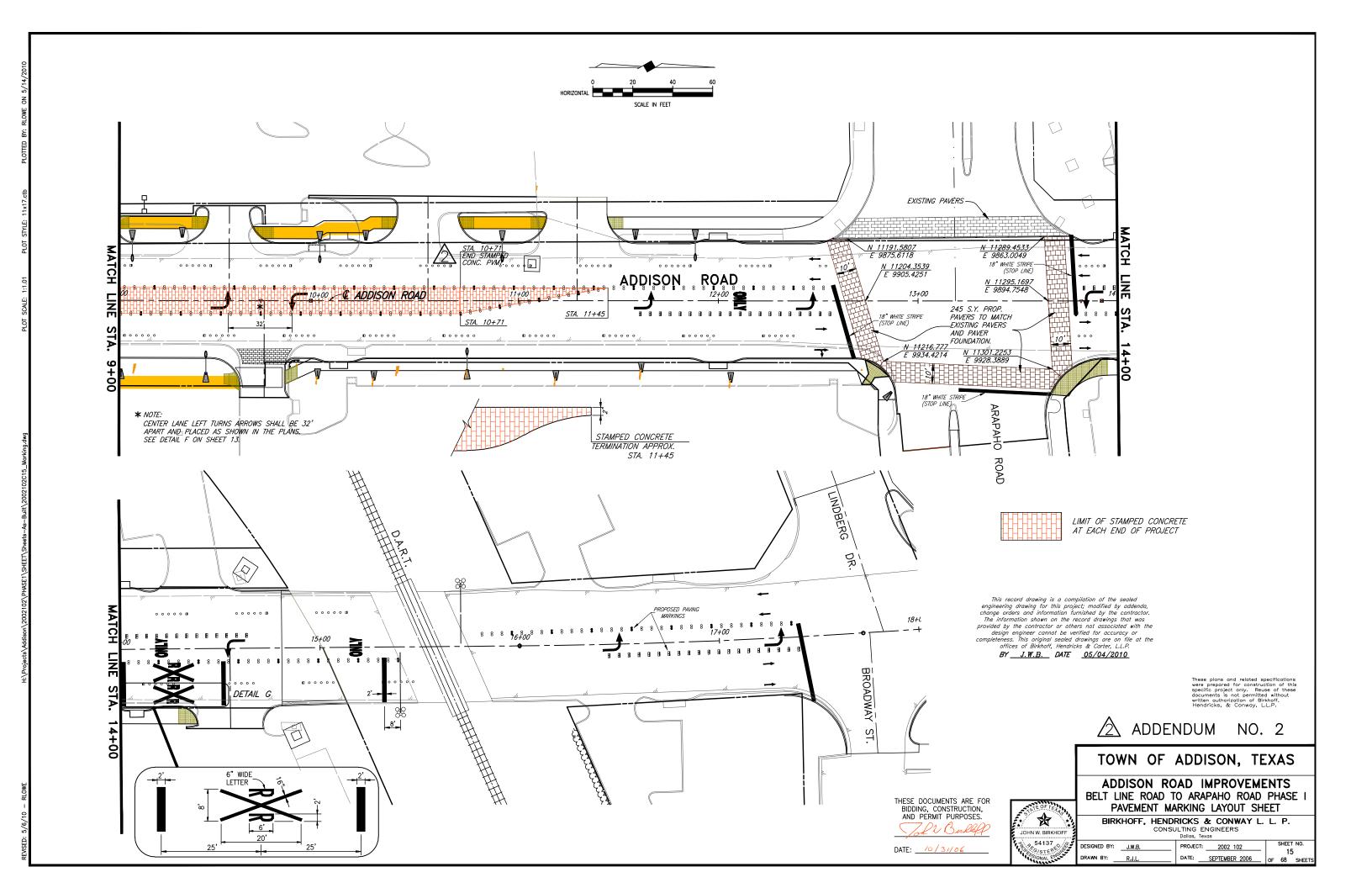
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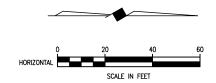
CONSULTING ENGINEERS
Dallas, Texas PROJECT: 2002 102

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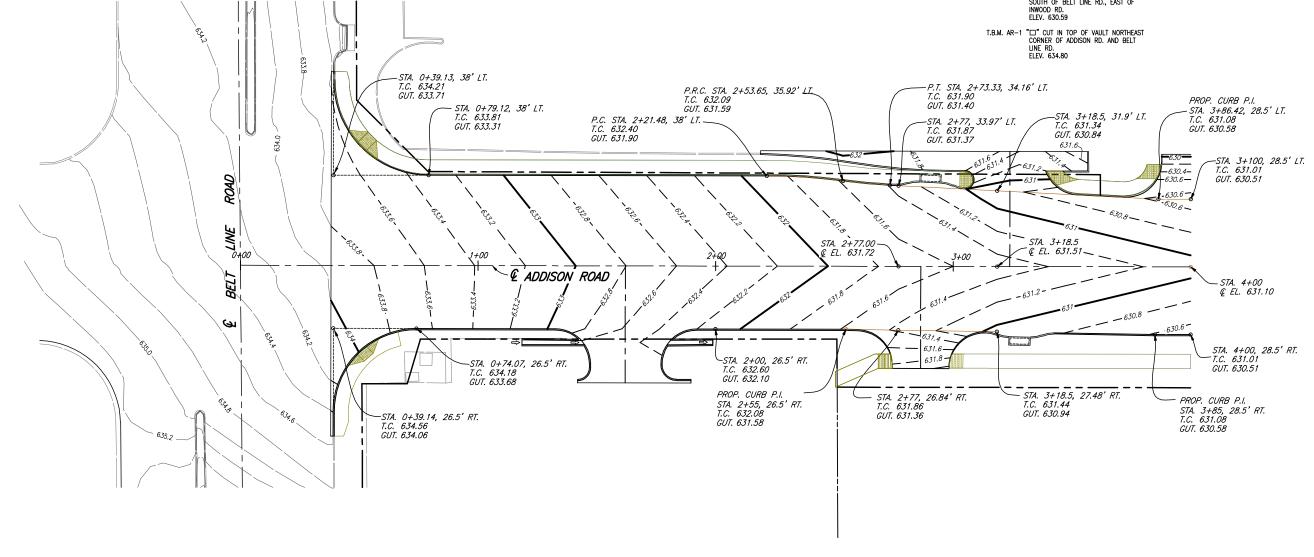
JOHN W. BIRKHOFI 54137 56/STEREO

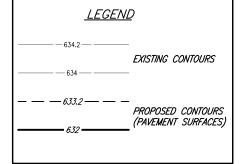
THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. John Bullett SCALE IN FEET DATE: 10/31/06





B.M.#19 "□" CUT IN CENTER OF INLET 150 FEET SOUTH OF BELT LINE RD., EAST OF INWOOD RD. ELEV. 630.59





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BY <u>J.W.B.</u> DATE <u>05/04/2010</u>

THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. John Bully

DATE: /0/31/06



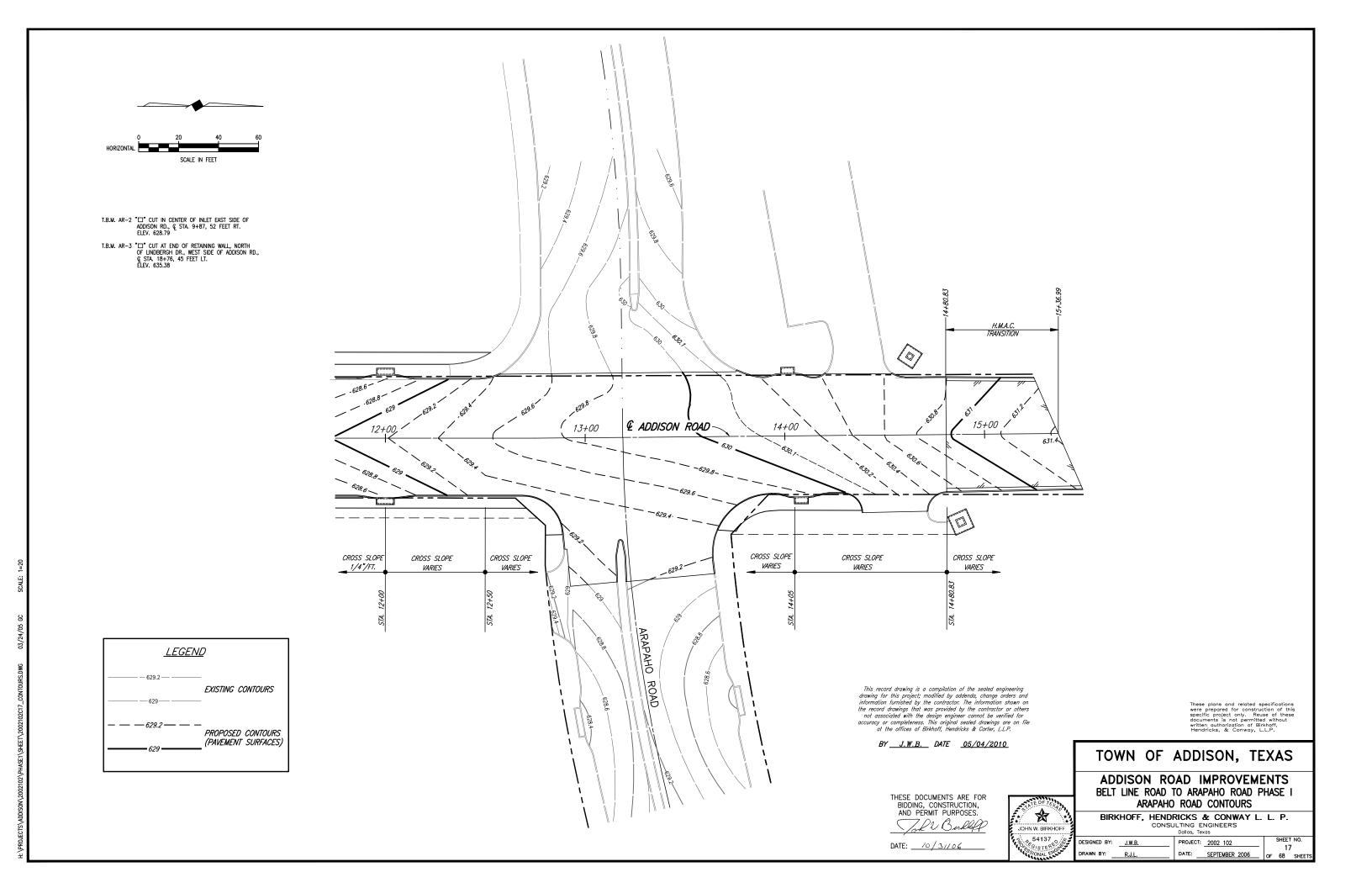
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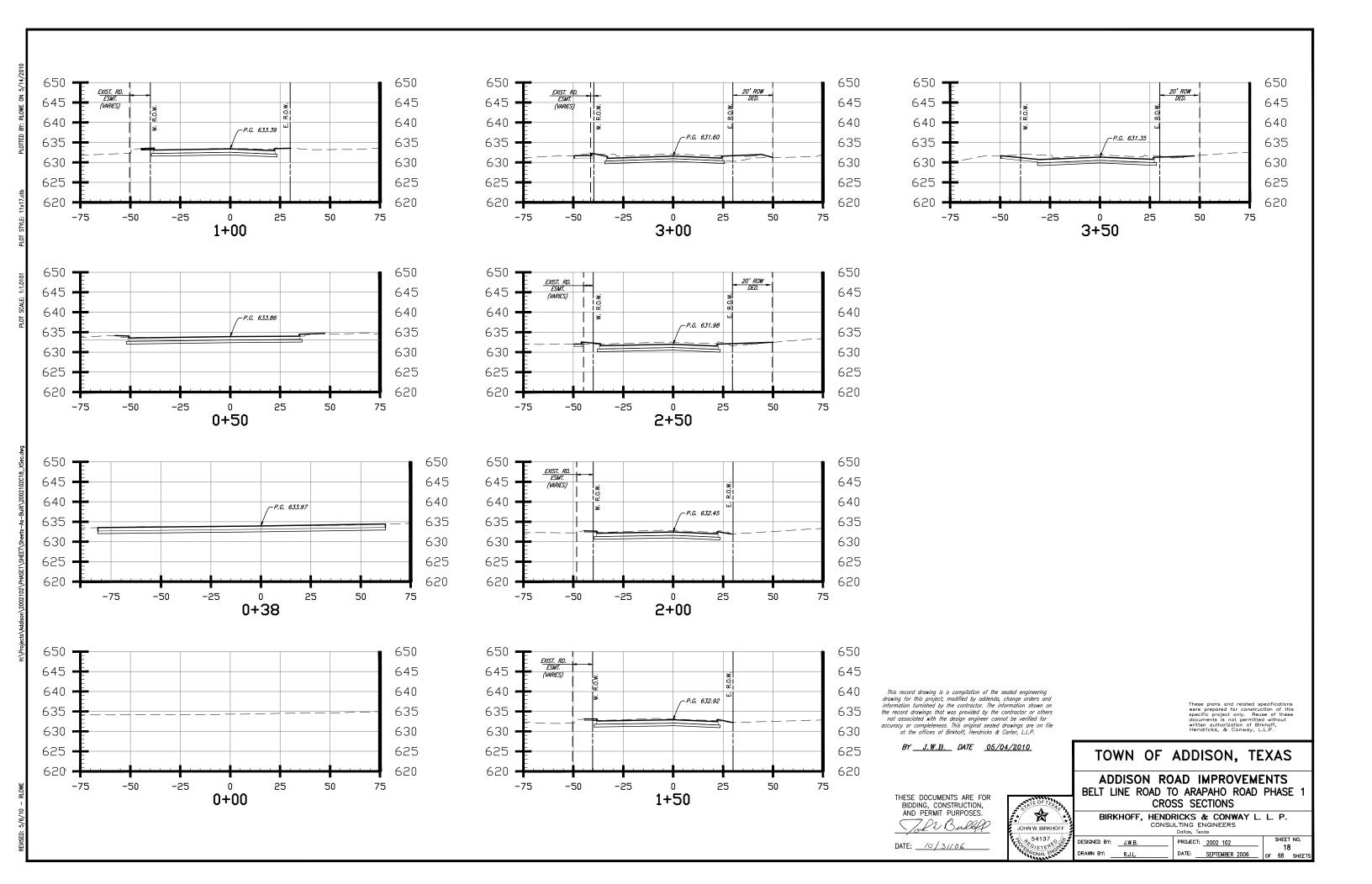
## TOWN OF ADDISON, TEXAS

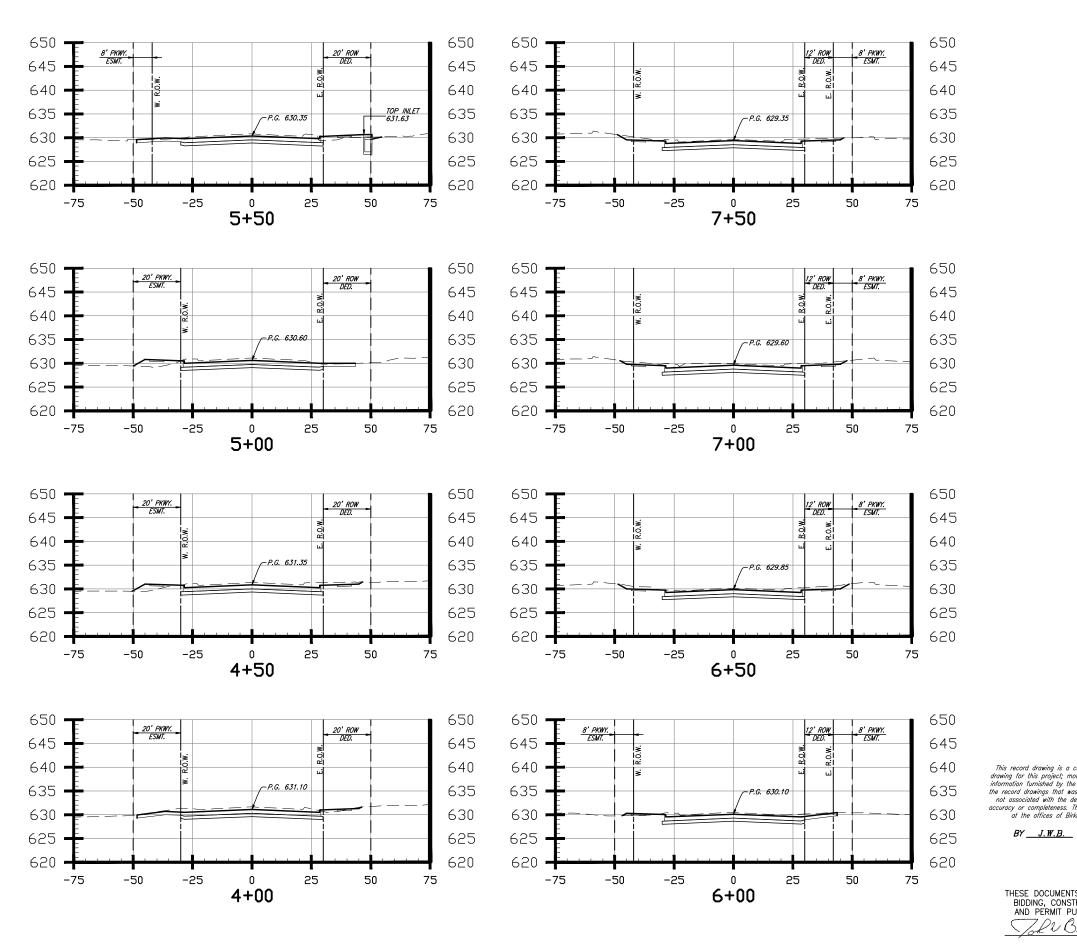
ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO KELLER SPRING ROAD BELT LINE ROAD CONTOURS

BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

DESIGNED BY: J.W.B. PROJECT: 2002 102 DATE: SEPTEMBER 2006







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BY J.W.B. DATE <u>05/04/2010</u>

THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. The Bullet

DATE: 10/31/06



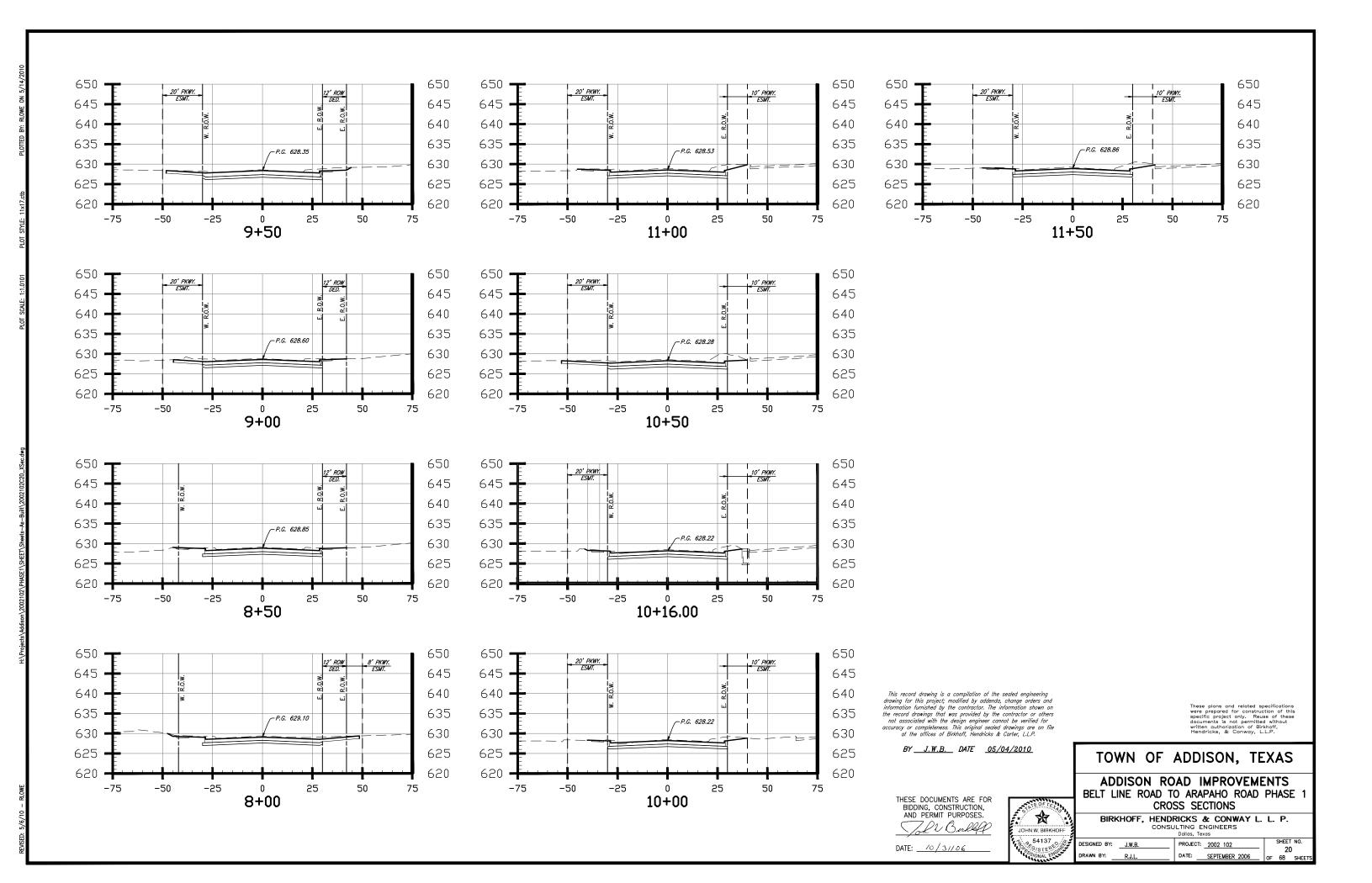
TOWN OF ADDISON, TEXAS

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ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE 1 CROSS SECTIONS

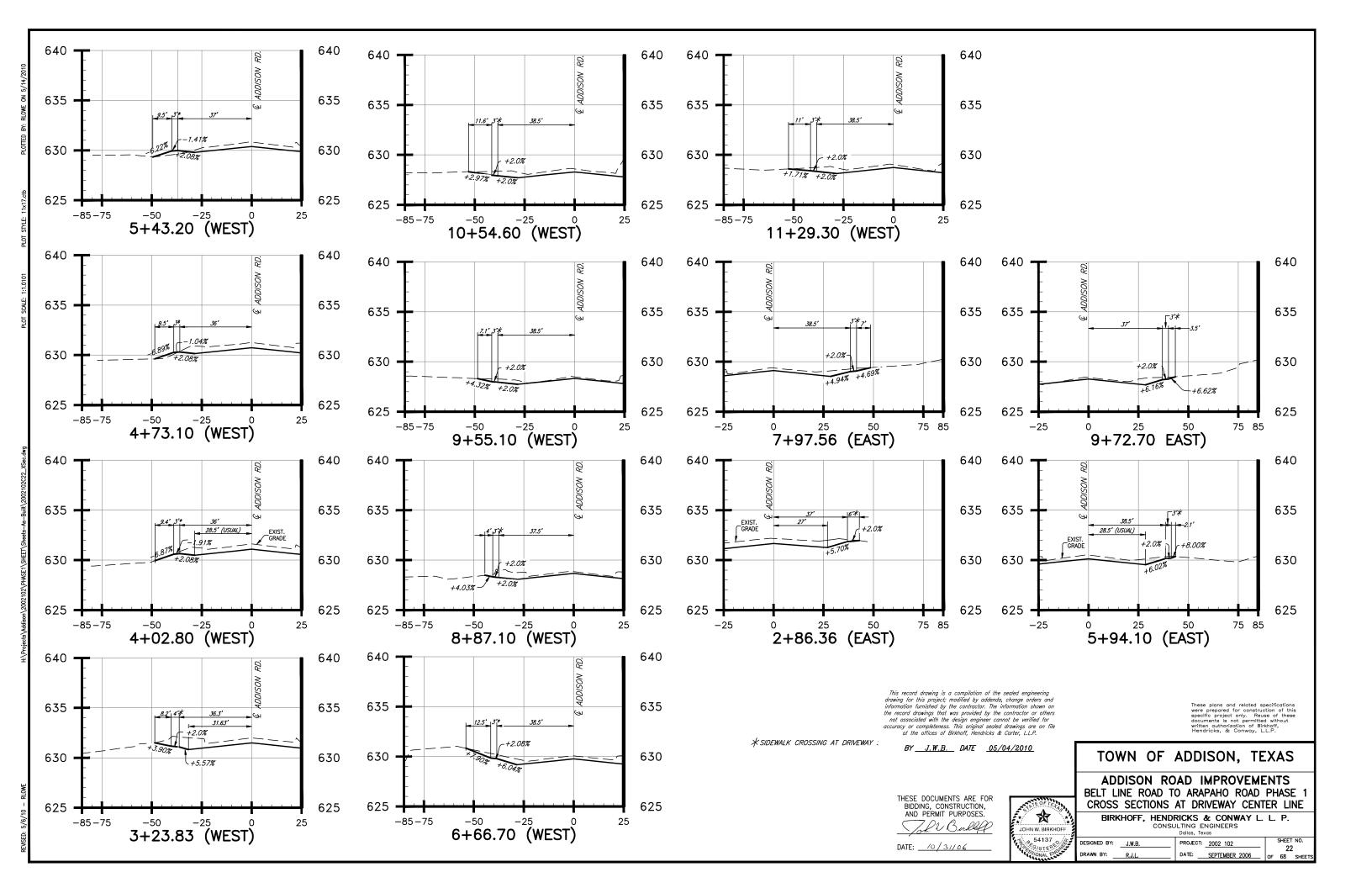
BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

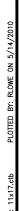
DESIGNED BY: J.W.B. PROJECT: 2002 102 19 DRAWN BY: R.J.L. DATE: SEPTEMBER 2006

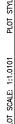


DATE: /0/31/06

SEPTEMBER 2006



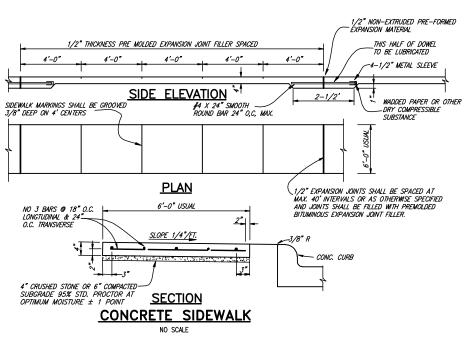


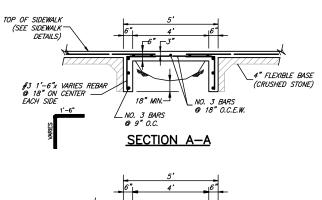


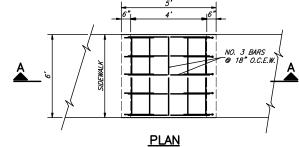




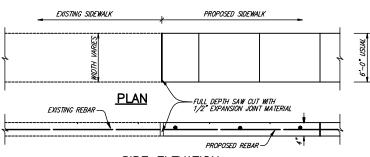








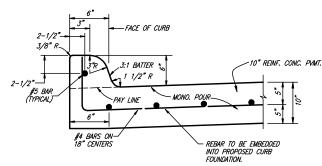
REINFORCED CONCRETE SIDEWALK BRIDGE



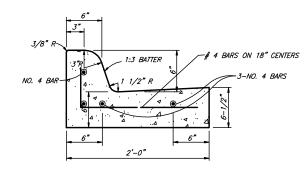
#### SIDE ELEVATION

# SIDEWALK BUTT JOINT DETAIL

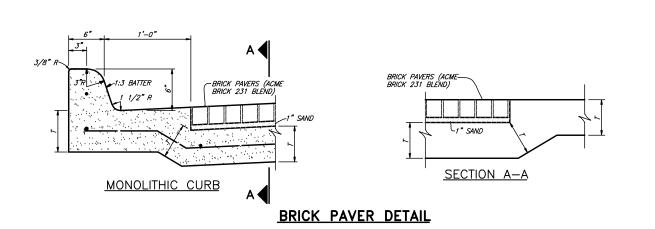
NOTE: ALL CONCRETE SHALL BE 4000 P.S.I. SEE TYPICAL SECTION SHEET FOR REQUIREMENTS

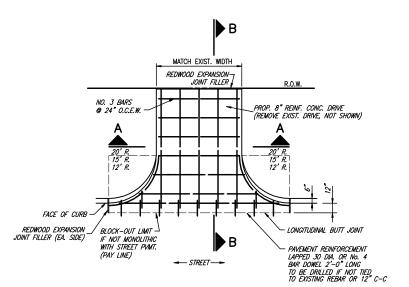


#### INTEGRAL CURB

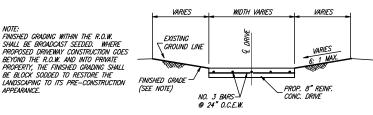


SEPARATE 24" CURB AND GUTTER

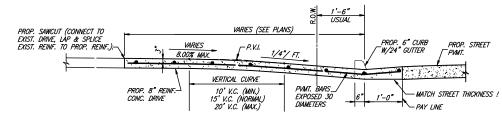




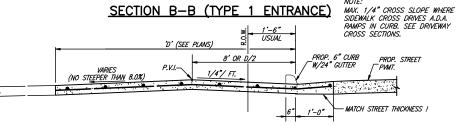
#### DRIVEWAY RETURN TO STREET



#### SECTION A-A



# SECTION B-B (TYPE 1 ENTRANCE)



# SECTION B-B (TYPE 2 ENTRANCE)

#### **DRIVEWAY RETURN SECTIONS**

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BY J.W.B. DATE 05/04/2010

THESE DOCUMENTS ARE FOR

BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.

DATE: /0/3//06

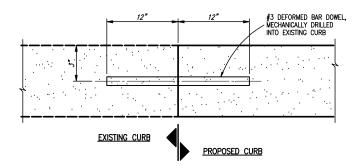
TOWN OF ADDISON, TEXAS ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE

DETAILS BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

These plans and related specifications were prepared for construction of this specific project only. Reuse of these documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.

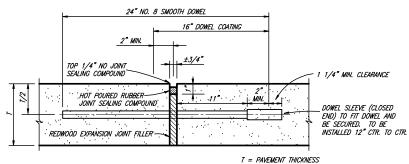
DESIGNED BY: J.W.B. PROJECT: 2002 102 23 DATE: SEPTEMBER 2006





#### **CURB DOWEL DETAIL**

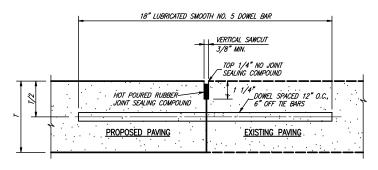
NOT TO SCALE



TRANSVERSE EXPANSION JOINT NOTES:

- 1. DOWELS AND REINFORCING BARS SHALL BE SUPPORTED BY AN
- 2. TRANSVERSE EXPANSION JOINTS SHALL BE SPACED AT 400 FT. MAXIMUM AND AT DRIVEWAY CONNECTIONS.

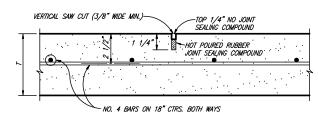
#### TRANSVERSE EXPANSION JOINT



NOTE: DOWEL BARS SHALL BE DRILLED INTO PAYEMENT HORIZONTALLY BY USE OF A MECHANICAL OR HAND RIG. HORIZONTAL HAND DRILLING METHOD IS ACCEPTABLE. PUSHING DOWEL BARS INTO GREEN CONCRETE IS NOT ACCEPTABLE.

## CONNECTION TO EXISTING PAVEMENT

NOT TO SCALE

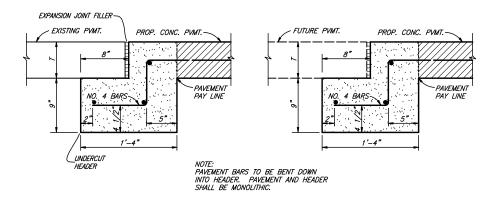


TRANSVERSE JOINTS SPACED 15 FT. C.—C. (MAX.) LONGITUDINAL JOINTS SPACED 20 FT. C.—C. (MAX.)

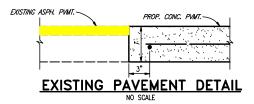
#### SAWED DUMMY JOINT

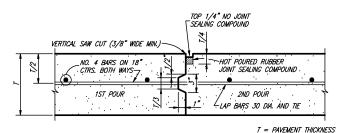
NO SCALE

- CONTRACTOR SHALL REMOVE AND PROPERLY DISPOSE THE EXISTING ASPHALT PAVEMENT, STRUCTURE, EXCAVATION, CONCRETE CURB & GUTTER. CONCRETE DRIVEWAYS AND CONCRETE SIDEWALK.
- ALL CONCRETE TO BE REMOVED SHALL BE WITH FULL DEPTH SAW CUT. IF JOINT IS WITHIN & OUTSIDE 5 FEET OF R.O.W. OR CURB RETURN, REMOVE CONCRETE AT JOINT. FINAL DETERMINATION TO BE MADE IN FIELD BY THE TOWN OF ADDISON.
- 3. CONTRACTOR SHALL PROTECT ALL UNDERGROUND UTILITIES DAMAGE CAUSED BY THE CONTRACTOR SHALL BE REPAIRED AT THE CONTRACTORS EXPENSE.
- 4. CONTRACTOR SHALL LAYOUT PROJECT BASED ON COORDINATES SHOWN IN THE PLANS. COORDINATES SHOWN AT
- a) CURB RETURNS. b) CENTER LINES OF PROPOSED ALIGNMENTS.
- AT BEGINNING AND ENDING OF PROPOSED ALIGNMENTS.
- 5. CONCRETE PAVING SHALL BE 10 INCHES N.C.T.C.O.G. CLASS "A" MIN. SACKS/C.Y.-5 MINIMI IM COMPRESSIVE
  - MINIMUM CUMPRESSIVE STRENGTH AT 28 DAYS-3600 P.S.I. MAX. W/C RATIO-6.5 REINFORCING SHALL BE NO. 4 DEFORMED BARS ON 18 INCH CENTERS
- 6. CONTRACTOR SHALL FURNISH AND PLACE SOLID SOD IN DISTURBED AREAS. (NO PAY ITEM)
- 7. PAVEMENT BASE SHALL BE SCARIFIED TO A DEPTH OF 6 INCHES AND COMPACTED TO 95% MODIFIED PROCTOR.
- 8. CONTRACTOR SHALL CONSTRUCT PROPOSED PAVEMENT 1/2 THE PROPOSED WIDTH AT A TIME.
- 9. CONSTRUCTION JOINTS SHALL HAVE A WELL FORMED KEY WAY OR BE A LONGITUDINAL BUTT JOINT.



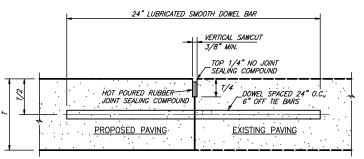
#### STREET HEADER





CONTRACTOR SHALL PROTECT KEYWAY PRIOR TO SECOND POUR. IF LONGTUDINAL KEYWAY IS DAMAGED, CONTRACTOR SHALL REPAIR WITH THE USE OF LONGTUDINAL BUTT JOINT (DRILL DOWELS INTO FIRST POUR).

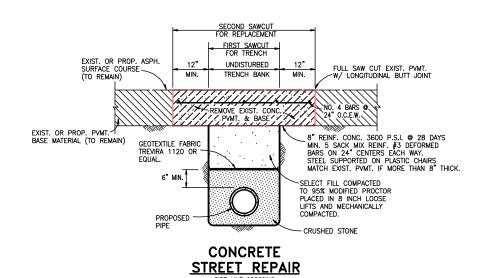
**CONSTRUCTION JOINT** 



NOTE: 1. T-8" AND GREATER NO. 6 BAR, T-6" AND LESS NO. 5 BAR

- 2. LONGITUDINAL BUTT CONSTRUCTION MAY BE UTILIZED IN PLACE OF LONGITUDINAL HINGED (KEYWAY) JOINT AT CONTRACTORS OPTION.
- DOWEL BARS SHALL BE DRILLED INTO PAVEMENT HORIZONTALLY BY USE OF A MECHANICAL RIG. HAND DRILLING NOT ACCEPTABLE. DAMAGE TO EXISTING PAVEMENT SHALL BE REMOVED BY CONTRACTOR AND JOINT CONSTRUCTED AT
- 4. DOWEL BAR SHOWN IS IN ADDITION TO TIE BARS (12" O.C.-6" OFF DOWELS).
- 5. TIE BARS SHALL BE NO. 5 BAR DEFORMED. TIE BAR SHALL HAVE A LENGTH OF 24 INCHES. TIE BARS ARE REQUIRED TO BE DEFORMED.

#### LONGITUDINAL BUTT JOINT



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BY J.W.B. DATE 05/04/2010

DATE: /0/3//06

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I **DETAILS** 

BIRKHOFF, HENDRICKS & CONWAY L. L. P.

PROJECT: 2002 102

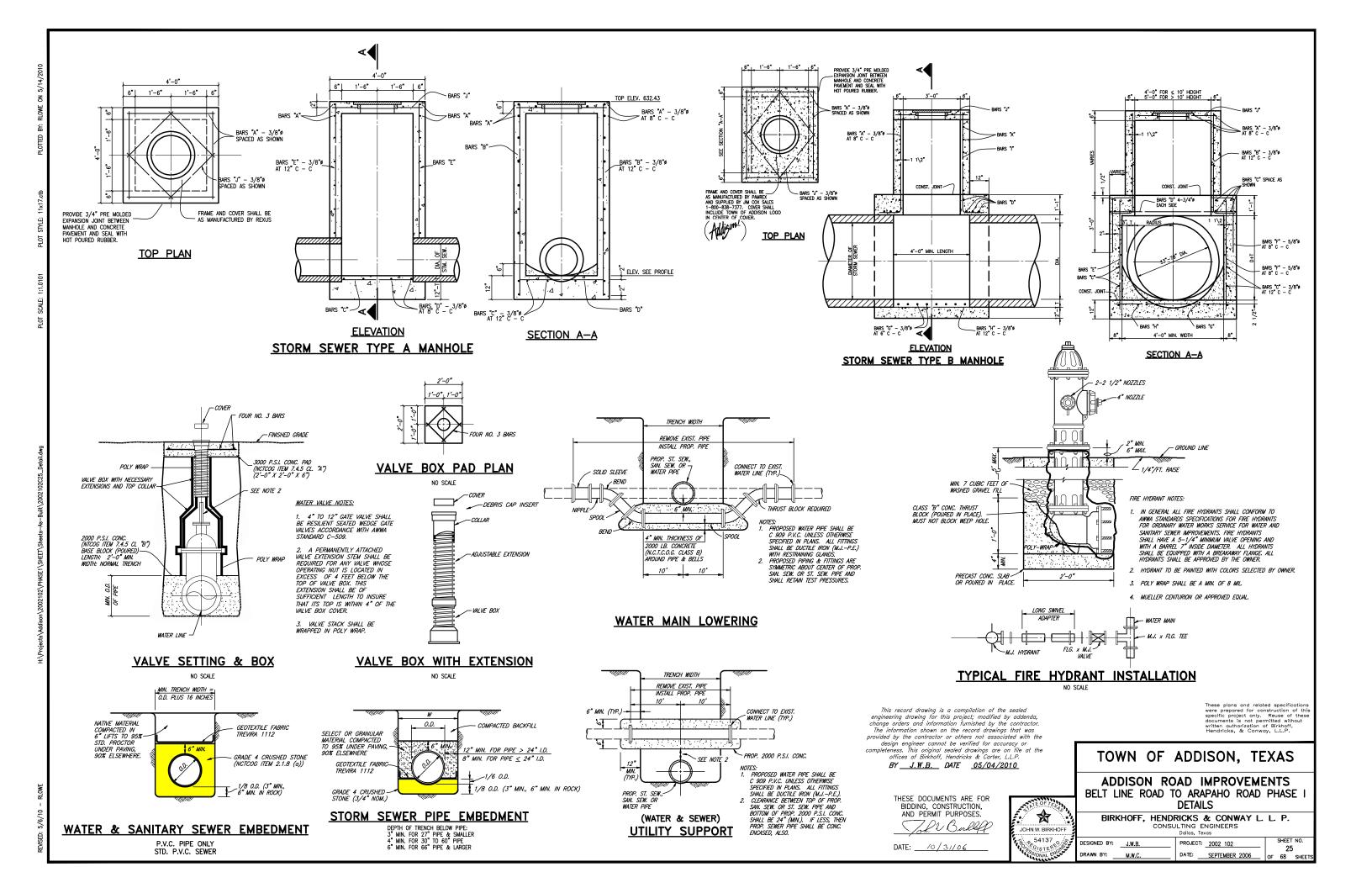
THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. JOHN W. BIRKHOFI 54137

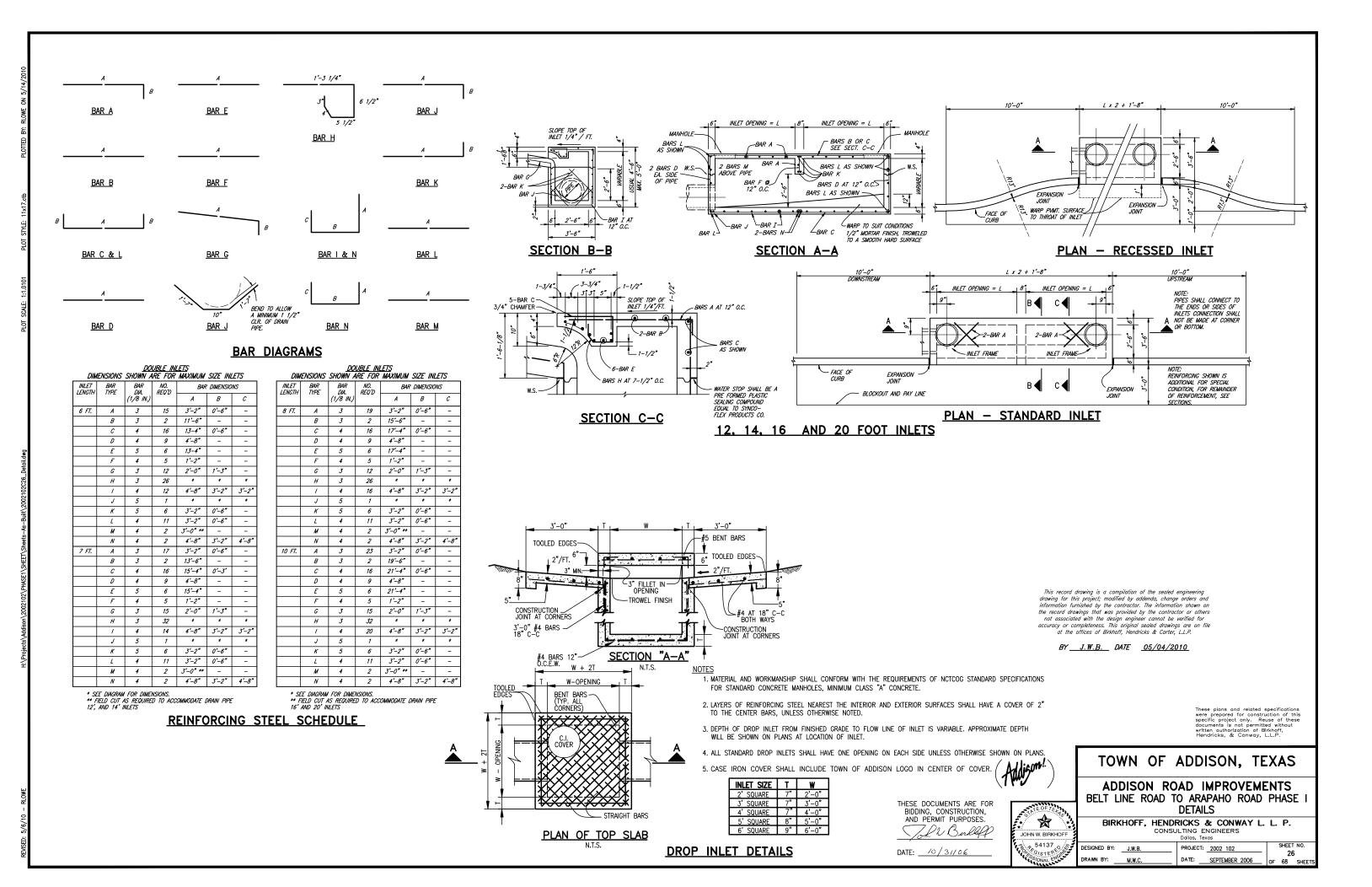
TOWN OF ADDISON, TEXAS

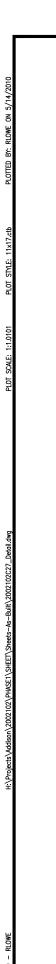
were prepared for construction of this specific project only. Reuse of these documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.

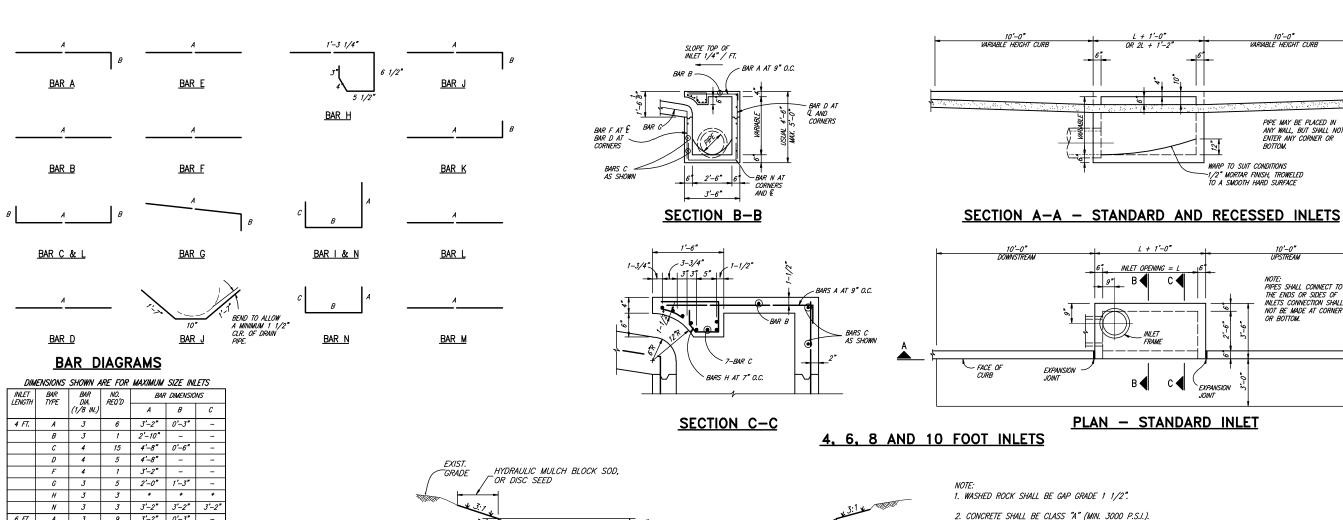
CONSULTING ENGINEERS

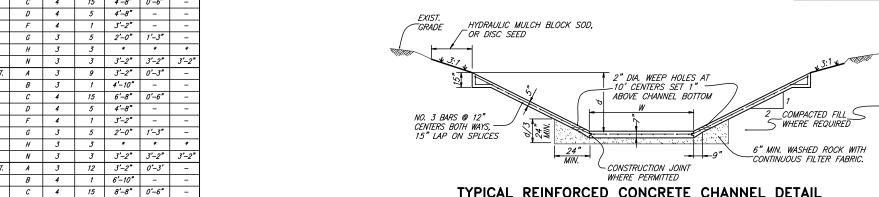
DATE: SEPTEMBER 2006



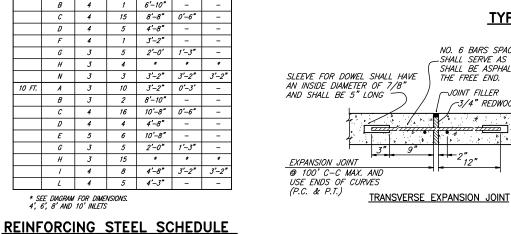








# TYPICAL REINFORCED CONCRETE CHANNEL DETAIL



4'-8"

1 3'-2"

2 8'-10"

6 10'-8"

5

16

3

3

3

5

\* SEE DIAGRAM FOR DIMENSIONS. 4'. 6'. 8' AND 10' INLETS

В

Ε

NO. 6 BARS SPACED 24" C TO C SHALL SERVE AS DOWELS. DOWELS
SHALL BE ASPHALT COATED 12" ON SPACE 12" C. TO C ~3/4" REDWOOD <del>|-</del>1-1/2" CONSTRUCTION JOINT SLAB EDGE DETAIL OPTIONAL

CONCRETE CHANNEL DETAIL

5. CONSTRUCTION JOINT SHOWN FOR CONVENIENCE ONLY. MONOLITHIC CONSTRUCTION MAY BE USED. 6. ALL VISIBLE SURFACES SHALL BE TROWEL FINISHED. This record drawing is a compilation of the sealed engineering 7. CONCRETE FOOTING SHALL BE CONSTRUCTED AT THE

not associated with the design engineer cannot be verified for accuracy or completeness. This original sealed drawings are on file at the offices of Birkhoff, Hendricks & Carter, L.L.P.

drawing for this project; modified by addenda, change orders and information furnished by the contractor. The information shown on

the record drawings that was provided by the contractor or others

BY J.W.B. DATE 05/04/2010

10'-0" VARIABLE HEIGHT CURB

PIPE MAY BE PLACED IN ANY WALL, BUT SHALL NOT ENTER ANY CORNER OR

*UPSTRFAM* 

NOTE: PIPES SHALL CONNECT TO THE ENDS OR SIDES OF

INLETS CONNECTION SHALL NOT BE MADE AT CORNER OR BOTTOM.

WARP TO SUIT CONDITIONS

1/2" MORTAR FINISH, TROWELED TO A SMOOTH HARD SURFACE

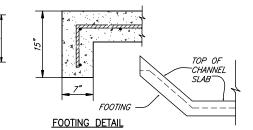
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# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I DETAILS

BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

PROJECT: 2002 102 27 DATE: SEPTEMBER 2006 M.W.C



TO CHANNEL EXCAVATION.

THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. DV Bullet

DATE: 10/31/06

3. FILL AREA SHALL BE COMPACTED TO 95% STD.
PROCTOR DENSITY AT OPTIMUM MOISTURE CONTENT PRIOR

4. WEEP HOLES CORE DRILL OR FORM WEEP HOLES.

BEGINNING AND THE END OF CONCRETE CHANNEL.

\* JOHN W. BIRKHOF 54137

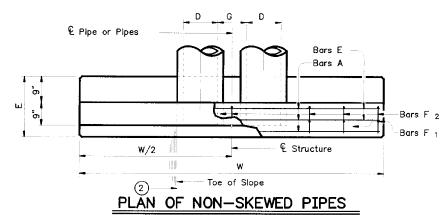
Г	T	ARIE OF	VART	ARLE	DIMENSI	NS	
ı	AND						
r	T	Values for			Values to I	be adde	ed e
OP F	무급	141405 101	Reinf	Conc	for each ad		Conc
<u>~</u>	DIA	w	(Lbs)	(CX)	w	(Lbs)	1(CX)
H	12"	9'- 0"	400	(1)	1'- 9"	1.5	(1)
ı	15"	10'-3"	122	1.1	2'- 2"	+	0.2
ı	18"	11'-6"	163	1.5	2'- 8"	+	0.3
	21"	12'- 9"	200	1.8	3'- 1"	DWALL be added ddt'l Pipe Reinf	0.4
ı	24"	14'- 0"	217	2.1	3'- 7"		0.4
ı	27"	15'- 3"	254	2.4	3'-11"	37	0.5
-	30" 33"	16'-6"	272	2.7	4'- 4" 4'- 8"		0.6
2:1	36"	17'-9" 19'-0"	314	3.1	5'- 1"		0.6
1	42"	21'- 6"	442	3.9	5'-10"		1.0
1	48"	25'- 0"	569	6.4	6'- 7"	<del>                                     </del>	1.3
ı	54"	27'- 6"	701	7.5	7'- 6"	t	1.6
	60"	30'- 0"	794	8.8	8'- 3"	90	1.8
•	66"	32'- 6"	894	10.2	8'- 9"	96	2.0
L	72"	35' - 0"	1055	11.7	9'- 4"	<del>                                     </del>	2.3
ı	12"	13'-0"	175	1.6	1'- 9"		0.2
	15" 18"	14'-9" 16'-6"	193 228	2.2	2'- 2" 2'- 8"	<del>                                     </del>	0.2
	21"	18' - 3"	299	2.6	3'- 1"		0.3
	24"	20'- 0"	323	3.0	3'- 7"	DWALL pe added to the added to	0.4
	27"	21'- 9"	371	3.5	3'-11"	-	0.5
	30"	23' - 6"	415	4.0	4'- 4"	40	0.5
;;  -	33"	25'- 3"	" 371 3.5 " 415 4.0 " 469 4.6 " 556 5.7 " 675 7.1	4'- 8"	43	0.6	
	36"	27'- 0"	<del></del>	_	5'- 1"		0.8
	42 <i>"</i> 48 <i>"</i>	30'- 6" 35'- 6"			5'-10" 6'- 7"		1.0
ı	54"	39'-0"	837 1015	9.2	7'- 6"		1.3
	60"	42'- 6"	1171	12.9	8'- 3"		1.8
ı	66"	46'- 0"	1298	14.9	8'- 9"		2.0
L	72"	49'- 6"	1561	17.1	9' 4"	103	2.3
1	12"	17'- 0"	229	2.0	1'- 9"	15	0.2
	15"	19' - 3" 21' - 6"	266	2.4	2'- 2"		0.2
	18" 21"	21'- 6" 23'- 9"	308 382	2.9 3.5	2'- 8" 3'- 1"		0.3
	24"	26'- 0"	430	3.9	3'- 7"		0.3
	27"	28'- 3"	486	4.7	3'-11"		0.5
	30"	30'- 6"	539	5.2	4'- 4"		0.6
<del>-</del> -	33″	32' 9"	603	6.0	4'- 8"	42	0.6
	36"	35'- 0"	738	7.5	5'- 1"	_	0.8
	42"	39'- 6"	881	9.3	5'-10"		1.0
	48″ 54″	46'- 0" 50'- 6"	1102 1364	12.1 14.4	6'- 7" 7'- 6"		1.3
	60″	55'- D"	1547	16.9	8'- 3"		1.6
	66"	59'- 6"	1741	19.5	8'- 9"		2.0
	72"	64'- D"	2069	22.4	9'- 4"		2.3
	12"	25'- D"	336	3.0	1'- 9"	14	0.2
	15"	28'- 3"	384	3.6	2'- 2"		0.2
	18"	31'- 6"	452	4.2	2'- 8" 3'- 1"		0.3
	21 <i>"</i> 24 <i>"</i>	34'- 9" 38'- 0"	581	5.1	3'- 1" 3'- 7"		0.4
	27"	41'- 3"	644 737	5.8 6.9	3-7	52 1. 61 1. 84 1. 91 1. 98 2. 102 2. 14 0. 17 0. 19 0. 31 0. 34 0. 37 0. 39 0.	0.4
	30"	44'- 6"	807	7.7	4'- 4"		0.5
6:1	33"	47'- 9"	912	8.9	4'- 8"		0.6
ω	36"	51'- 0"		11.0	5'- 1"		0.8
	42"	57'- 6"	1318	13.7	5'-10 <b>"</b>	54	1.0
	48"	67'- 0"		17.9	6'- 7"		1.3
	54"	73' - 6"		21.3	7'- 6"		1.6
	60" 66"	80'- 0" 86'- 6"		24.9	8'- 3" 8'- 9"		1.8
	72 <i>"</i>	93'- 0"		28.9 33.1	9'- 4"		2.0
H	15	JJ - U	J123	JJ 1	J - 4	101	2.3

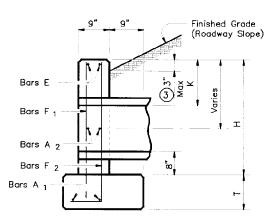
1 Quantities increase slightly for metal pipe

W/2 2, € Structure Bars E -Bars F Bars A 1

2Indicated slope is perpendicular to Q Pipe or Pipes

### **ELEVATION**





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BY <u>J.W.B.</u> DATE <u>05/04/2010</u>

3 For vehicle safety, curb heights and wall heights shall be reduced, if necessary, to provide a maximum 3" projection above finished grade.
No changes will be made in quantities and no additional compensation will be allowed for this work.

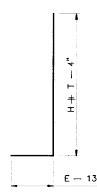
THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. John Bullett

DIA OF PIPE,D	G	К	н	т	E
12"	9"	1'- 0"	2'-8"	9"	1'- 9"
15"	11"	1'- 0"	2'-11"	9"	1'- 9"
18"	1'- 2"	1'- 0"	3'- 2"	9 "	1'- 9"
21"	1'- 4"	1'- 0"		9"	2'- 0"
24"	1'- 7"	1'- 0"			2'- 0"
27"	1'- 8"	1'-0"	3'-11"	9"	2'- 3"
30"	1'-10"	1'-0"	4'- 2"	9"	2'- 3"
33″	1'-11"	1'- 0"	4'-5"	9"	2'-6"
36"	2'-1"	1'- 0"		1'- 0"	2'- 6"
42"	2'- 4"	1'- 0"	5'- 2"	1'- 0"	2'- 9"
48"	2'- 7"	1'- 3"	5'-11"	ı – u	3'- 0"
54"	3'- 0"	1'-3"	6'-5"		3'-3"
60"	3'- 3"	1'-3"	6'-11"		3'- 6"
66"	3'- 3"	1'- 3"	7'- 5"	1'- 0"	3'- 9"
72"	3'- 4"	1'- 3"	7'-11"	1'- 0"	4'- 0"
		REI	) TABI NFORC		EEL

TABLE OF CONSTANT DIMENSIONS

> Bar Size Spa A1 #5 ~ A2 # 5 1'-6" E #5 ~ 2 F #5 1'-0" ~

4 Quantities shown are for one structure end. (One headwall)



BARS F2

\*

JOHN W. BIRKHOFF

54137

GENERAL NOTES:

Designed according to current AASHTO
Standard and Interim Specifications.

Reinforcing steel shall be placed with
the center of the outside layer of bars 2"
from the surface of the concrete.

All reinforcing steel shall be Grade 60.

All concrete shall be Class "C" and shall
have a minimum 28 day compressive strength
of 3600 psi.

# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I CH-PW-O CONCRETE HEADWALL DETAIL

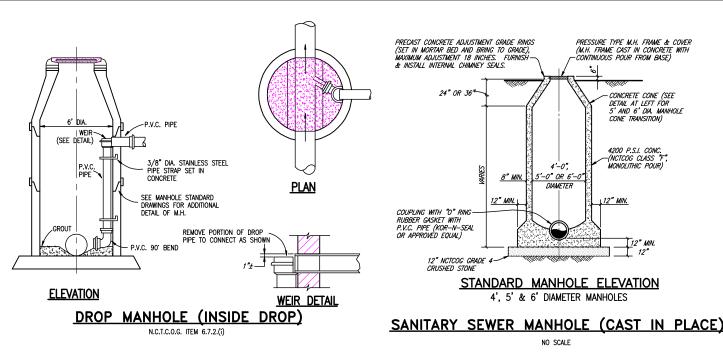
BIRKHOFF, HENDRICKS & CONWAY L. L. P.

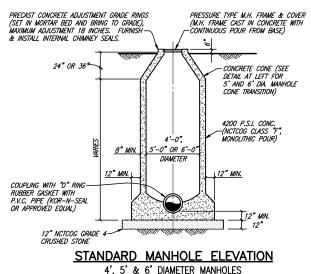
CONSULTING ENGINEERS Dallas, Texas

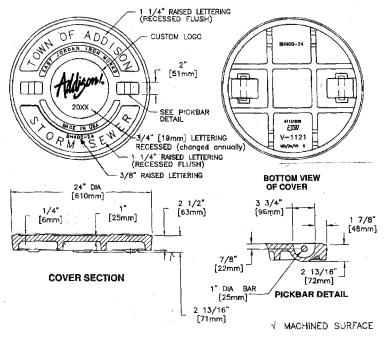
PROJECT: 2002 102 28 DATE: SEPTEMBER 2006

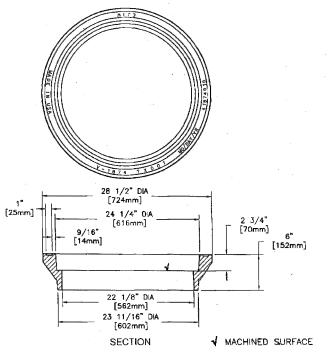
# **€** SECTION

DATE: /0/3//06



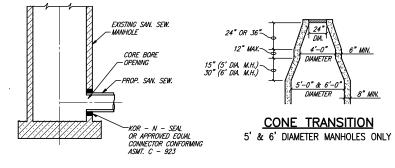


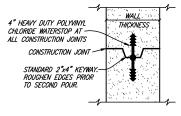




# STORM SEWER MANHOLE

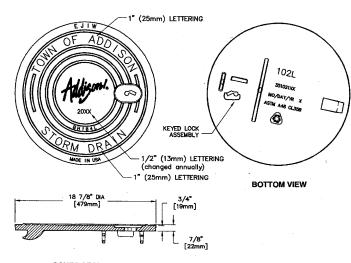
MANHOLE FRAME HEAVY DUTY

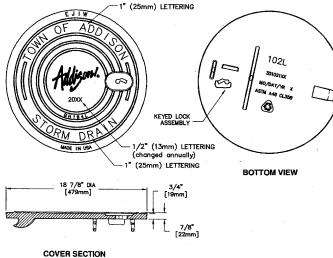




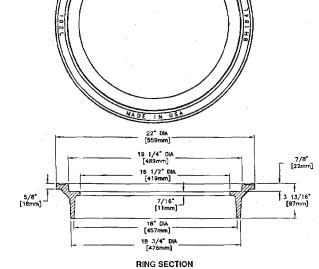
MANHOLE CONSTRUCTION JOINT KEYWAY WITH WATERSTOP

#### SAN. SEW. TO EXIST. MANHOLE CONNECTION DETAIL





#### LOCKING COVER LIGHT DUTY



#### **INLET RING** HEAVY DUTY

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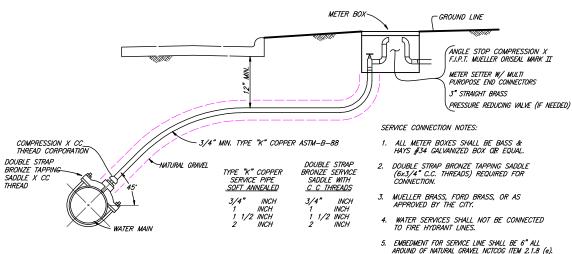
\* JOHN W. BIRKHOFF 54137 DATE: 10/31/06

# TOWN OF ADDISON, TEXAS ADDISON ROAD IMPROVEMENTS

BELT LINE ROAD TO ARAPAHO ROAD PHASE I **DETAILS** 

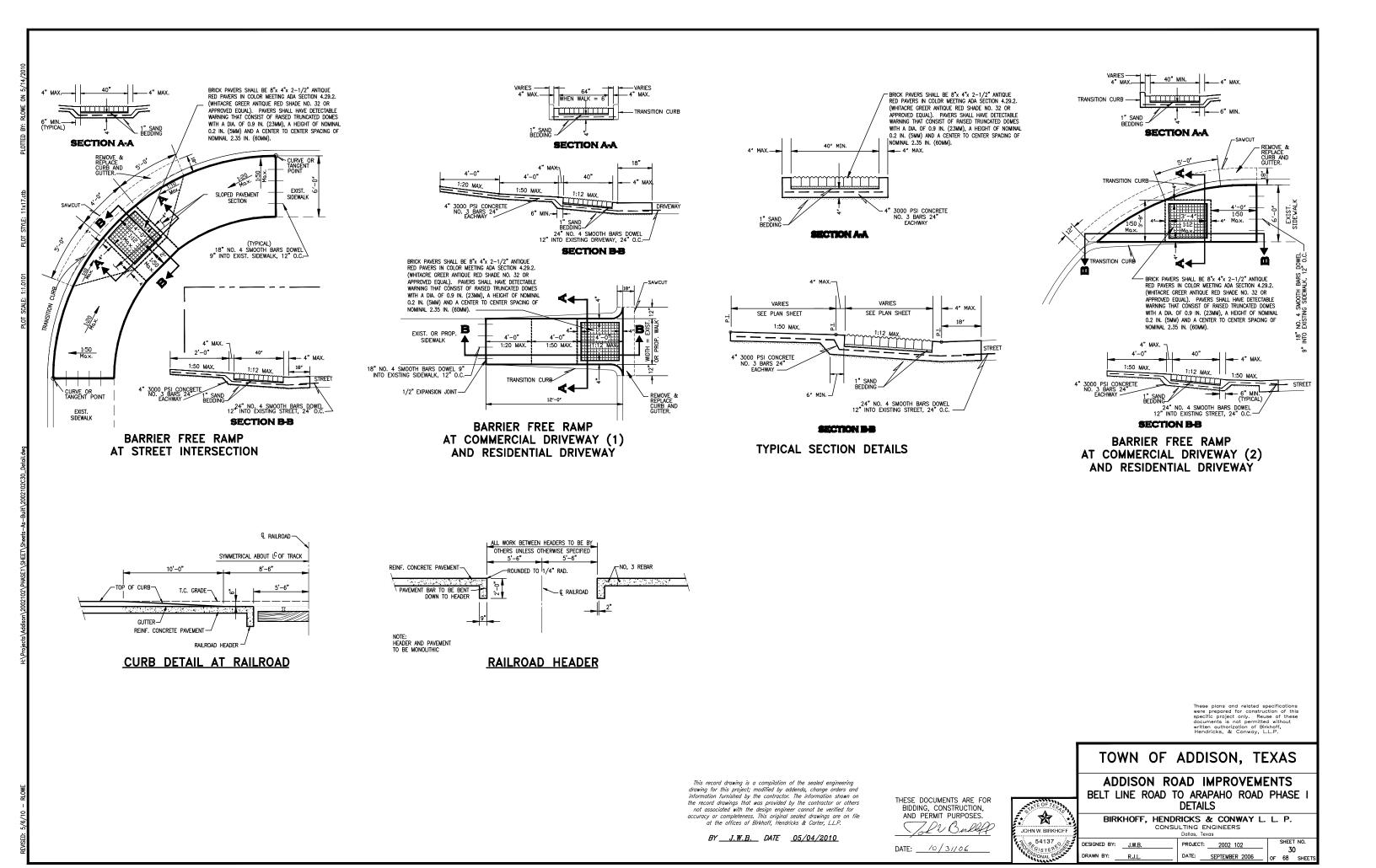
BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

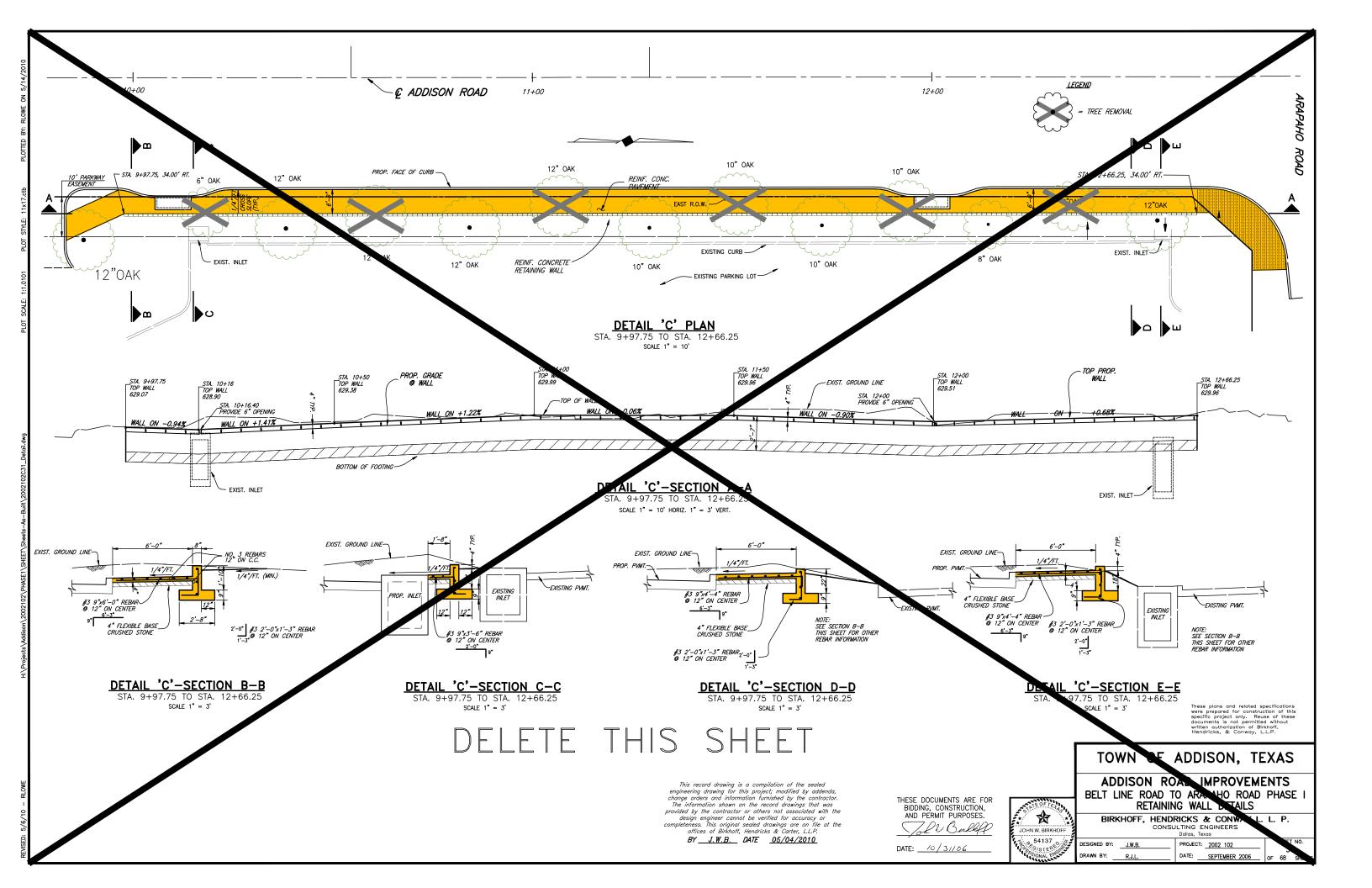
PROJECT: 2002 102 29 DATE: SEPTEMBER 2006

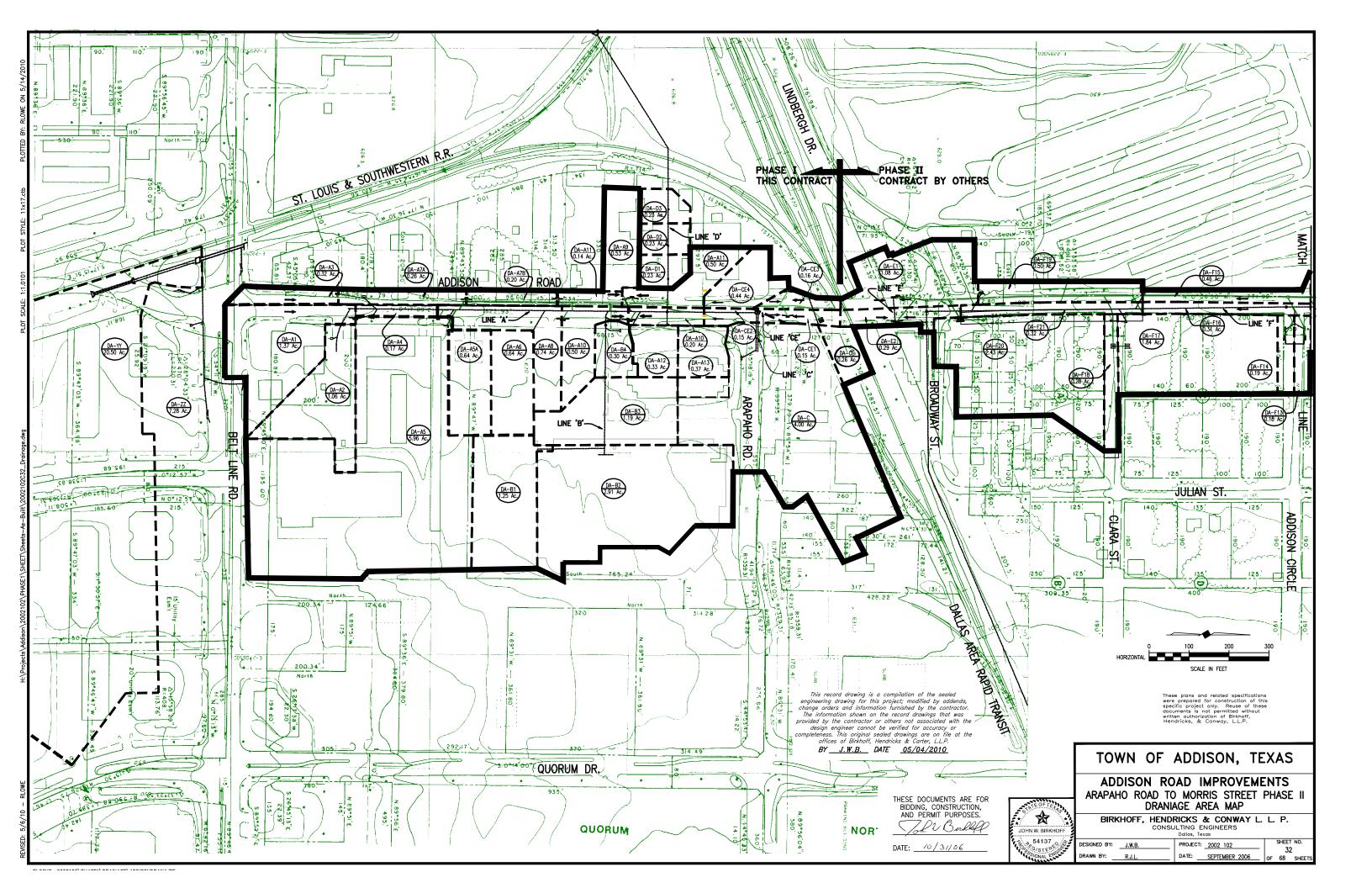


TYPICAL SERVICE CONNECTION WITH METER BOX

provided by the contractor or others not associated with the THESE DOCUMENTS ARE FOR design engineer cannot be verified for accuracy or completeness. This original sealed drawings are on file at the offices of Birkhoff, Hendricks & Carter, L.L.P. BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. BY J.W.B. DATE 05/04/2010







#### RUNOFF CALCULATIONS - BELT LINE ROAD TO ARAPAHO ROAD

AREA NUM.  A1  A2  A3  A4  A5  A5A  A6  A7A  A7B  A8  A9  A10  A11  A12  A13	AREA  (AC.)  1.37  1.06  0.32  0.17  5.96  0.64  0.26  0.20  0.74  0.53  0.70  0.64  0.33  0.37	RUNOFF COEFF. "C" 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.9	DESIGN STORM FREQ. (YEAR) 100 100 100 100 100 100 100 100 100 10	TIME OF CONC. (MIN.) 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00	INTENSITY  (IN./HR.)  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74  8.74	Q= CIA  (C.F.S.)  10.78  8.34  2.52  1.34  46.88  5.03  5.03  2.05  1.57  5.82  4.17  5.51  5.03  2.60  2.91	REMARKS  Commercial / Retail
A13	0.37	0.90	100	10.00	8.74	2.91	Commercial / Redail
B1	1.25	0.90	100	10.00	8.74	9.83	Commercial / Retail
B2	2.91	0.90	100	10.00	8.74	22.89	Commercial / Retail
B3	1.19	0.90	100	10.00	8.74	9.36	Commercial / Retail
B4	0.30	0.90	100	10.00	8.74	2.36	Commercial / Retail
C1	4.00	0.90	100	10.00	8.74	31.46	Commercial / Retail
CE1	0.15	0.90	100	10.00	8.74	1.18	Commercial / Retail
CE2	0.15	0.90	100	10.00	8.74	1.18	Commercial / Retail
CE3	0.16	0.90	100	10.00	8.74	1.26	Commercial / Retail
CE4	0.44	0.90	100	10.00	8.74	3.46	Commercial / Retail
				10.00	0.54	1.01	Commercial / Retail
D1	0.23	0.90	100	10.00	8.74	1.81	
D2	0.23	0.90	100	10.00	8.74	1.81	Commercial / Retail Commercial / Retail
D3	0.23	0.90	100	10.00	8.74	1.81	Commercial/Retail
CE	0.26	0.90	100	10.00	8.74	2.05	Commercial / Retail

This record drawing is a compilation of the sealed engineering drawing for this project; modified by addenda, change orders and information furnished by the contractor. The information shown on the record drawings that was provided by the contractor or others not associated with the design engineer cannot be verified for accuracy or completeness. This original sealed drawings are on file at the offices of Birkhoff, Hendricks & Corter, L.L.P.

BY <u>J.W.B.</u> DATE <u>05/04/2010</u>

THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. Tola Bulleto

DATE: /0/3//06



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# TOWN OF ADDISON, TEXAS

#### ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I RUNOFF CALCULATIONS

BIRKHOFF, HENDRICKS & CONWAY L. L. P.
CONSULTING ENGINEERS
Dallas, Texas

DRAWN BY: R.J.L.

PROJECT: 2002 102 DATE: SEPTEMBER 2006 OF 68 SHEETS

# STORM SEWER LINES "A,B,C & D" STORM SEWER CALCULATIONS

unoff Collection P	oint	Distance	Incre	mental Drainage	э Агеа			Time at	Design			Slope of	No	Selected	Velocity	Head	Velocity	Flow Time	Time at	2
		Between		Drainage	Runoff	Increm-	Accum-	Upstream	Storm	Intensity	Runoff	Hydraulic	of	Storm	Between	Loss	Head Loss	Distance/	Downstream	Remarks
Upstream	Downstream	Collection	Area.	Area	Coeff.	ental	ulated	Station	Freq.	(1)	(Q)	Gradient	Pipes	Sewer	Points	Coeff.	Upstream	( Vel*60 )	Station	
Station	Station	Points	No.	(Ac,)	"C"	"CA"	"CA"	(MIN.)	(Yrs.)	(ln./Hr.)	(c.f.s.)	(Ft./Ft.)		Size	(f.p.s.)	"K"	(Feet)	(Min.)	(Min.)	
1	2	3	4	5	- 6	7	8	9	10	11	12	13		14	15	16	17	18	19	20
				<del>                                     </del>				l			L		<u> </u>							
									Existing Line	A With/ Prop. P	arallel Line A1									
784.55	720.06	64.49	A2	1.06	0.90	0.95	0.95	10.00	100	8.74	8.34	0.0004	1	30	1.70	1	0.00	0.63	10.63	
720.06	717.50	2.56	A3	0.32	0.90	0.29	1.24	10.63	100	8.66	10.76	0.0007	1	30	2.19	1	0.07	0.02	10.65	
717.50	554.79	162.71	A1	1.37	0.90	1.23	2.48	10.65	100	8.65	21.41	0.0027	1	30	4.36	1	0.25	0.62	11.27	
554.79	526.88	27.91	A4	0.17	0.90	0.15	2.63	11.27	100	8.45	22.21	0.0029	1	30	4.52	1	0.02	0.10	11.38	
526.88	294.11	232.77	A5 & A5A	3.60	0.90	3.24	5.87	11.38	100	8.42	49.41	0.0024	1	42	5.14	1	0.09	0.76	12.13	3.00 Ac. To Prop. A1
294.11	242.72	51.39	A6	0.24	0.90	0.22	6.08	12.13	100	8.18	49.77	0.0012	1	48	3.96	1	0.00	0.22	12.35	0.40 Ac. To Prop. A1
242.72	147.84	94.88	A8	0.34	0.90	0.31	6.39	12.35	100	8.15	52.08	0.0013	1	48	4.14	1	0.02	0.38	12.73	0.40 Ac. To Prop. A1
147.84	80.51	67.33	LINE B	2.65	0.90	2.39	8.78	12.73	100	8.06	70.73	0.0024	1	48	5.63	1	0.23	0.20	12.93	3.00 Ac. To Prop. A1
80.51	67.01	13.50	A10	0.70	0.90	0.63	9.41	12.93	100	8.02	75.43	0.0028	1	48	6.00	1	0.07	0.04	12.97	
67.01	0.00	67.01	A12	0.33	0.90	0.30	9.70	12.97	100	8.01	77.71	0.0029	1	48	6.18	1	0.03	0.18	13.15	
				10.78					2											
													<u> </u>			· · · · · ·				
									·											
			TENER TO THE PERSON AND THE PERSON A							Proposed Line A		T								
512.75	495.18	17.57	A5 & A5A	3.00	0.90	2.70	2.70	10.00	100	8.74	23.60	0.0020	1	33	3.97	1	0.00	0.07	10.07	
495.18	264.23	230.95	A7A	0.26	0.90	0.23	2.93	10.07	100	8.70	25.53	0.0023	1	33	4.30	1	0.29	0.90	10.97	
264.23	240.42	23.81	A7B	0.20	0.90	0.18	3.11	10.97	100	8.42	26.22	0.0015	1	36	3.71	1	0.00	0.11	11.08	
240.42	144.00	96.42	A6 & A8	0.80	0.90	0.72	3.83	11.08	100	8.40	32.21	0.0023	1	36	4.56	1	0.11	0.35	11.43	
144.00	103.77	40.23	LINE B	3.00	0.90	2.70	6.53	11.43	100	8.30	54.23	0.0014	1	48	4.32	1	0.00	0.18	11.58	
103.77	89.32	14.45	A9	0.53	0.90	0.48	7.01	11.58	100	8.27	57.98	0.0016	1	48	4.61	1	0.04	0.05	11.64	
89.32	0.00	89.32	A11	0.64	0.90	0.58	7.59	11.64	100	8.25	62.59	0.0014	1	51	4.41	1	0.00	0.34	11.97	
				8.43																
											L									
		1							Endador 1	0 0 0 0			1	1						
200.00	000.07	100.70			0.00			1 40.00		Line C & Propos		1 00000	<del>                                     </del>		0.85		0.00	0.45	40.45	
398.00	288.27	109.73	CE	0.26	0.90	0.23	0.23	10.00	100	8.74	2.05	0.0002	1	21	0.85	1	0.00	2.15	12.15	
288.27	282.14	6.13	CE1	0.15	0.90	0.14	0.37	12.15	100	8.12	3.00	0.0002	1 1	24	0.95	1	0.01	0.11	. 12.26	
282.14	226.93	55.21	CE3	0.16	0.90	0.14	0.51	12.26	100	8.09	4.15	0.0003	1	24	1.32	1	0.02	0.70	12.95	
226.93	78.22	148.71	C	4.00	0.90	3.60	4.11	12.95	100	7.94	32.66	0.0063	1 -	30	6.65	1	0.67	0.37	13.33	· · · · · · · · · · · · · · · · · · ·
78.22	72.50	5.72	CE2	0.15	0.90	0.14	4.25	13.33	100	7.85	33.35	0.0025	1 1	36	4.72	1	0.32	0.02	13.35	
72.50	0.00	72.50	CE4	0.44	0.90	0.40	4.64	13.35	100	7.84	36.41	0.0030	1	36	5.15	1	0.00	0.23	13.58	
<u> </u>				5.16		<u> </u>											·		<del> </del>	
				ļ		<u> </u>	<u> </u>	<u> </u>		l	<u></u>	I	<del> </del>							
		j								Existing Line D					11.67					
749.90	667.30	82.60	A, A13,C&CE	24.67	0.90	22.20	21.97	12.21	100	8.10	177.96	0.0047	1	60	9.06	1	0.00	0.15	12.36	
667.30	584.60	82.70	D1	0.23	0.90	0.21	22.41	12.36	100	8.07	180.85	0.0047	1	60	9.21	<b>⊢</b> ;─	0.04	0.15	12.51	
										8.06	182.29	0.0049	1	60	9.28	1	0.02	0.16	12.68	
584.60	493.50	91.10	D2	0.23	0.90	0.21	22.62	12.51	100			0.0050	<del>                                     </del>	60	9.36	4	0.02	0.16	13.55	
493.50	0.00	493.50	D3	0.23	0.90	0.21	22.82	12.68	100	8.05	183.73	0.0050	<del> '</del>	90	9.30		0.02	0.00	10.00	
			1	25.36				<del> </del>	<del> </del>		<del></del>		<del></del> -							
		L	<u></u>	<u></u>	<u> </u>			<u> </u>	L	<u>L</u>	<u> </u>	<u> </u>	L		L			-		

This record drawing is a compilation of the seoled engineering drawing for this project; modified by addenda, change orders and information furnished by the contractor. The information shown on the record drawings that was provided by the contractor or others not associated with the design engineer cannot be verified for accuracy completeness. This original seoled drawings are on file at the offices of Birkhoff, Hendricks & Carter, L.L.P.

BY <u>J.W.B.</u> DATE <u>05/04/2010</u>

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# TOWN OF ADDISON, TEXAS

# ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I STORM SEWER CALCULATIONS

BIRKHOFF, HENDRICKS & CONWAY L. L. P.

CONSULTING ENGINEERS Dallas, Texas

| SHEEL NO. | PROJECT: 2002 102 | SHEEL NO. | 34 | SHEEL NO. | SHE

## INLET CALCULATIONS

Part	11	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	20	21	22	23	24	25	26	27	28	29
Real Ranker Number No. Carter Line Carter								ons							Gu	ter Cap												
A2 2-64.5 -65.1 °R.T. 1.06 0.00 0.90 0.95 0.95 10.00 100 8.74 8.34 0.00 8.74 48.0 320 0.0050 0.36 0.42 1.49 5.60 8.00 1.43 1.17 1.43 1.19 0.00 Existing 2'x2' Drop finte By Others A3 A3 2-90.3 + 35.3 *** 1.17 1.17 1.17 1.17 1.17 1.17 1.17			Center Line	Area (Ac.)	Area (Ac.)	Runoff Coeff.	Increm- ental	ulated	Upstr. Sta.	Storm Freq.	(I)	"Q"	Over	Flow "Qa"	z	Z/N	Slope "S"	Depth of Flow in Gut		qL (c.f.s.)	Length Required	Supplied La		a/Y	Q/Qa	` `	_	Remarks
A2 2-64.5 -65.1 °R.T. 1.06 0.00 0.90 0.95 0.95 10.00 100 8.74 8.34 0.00 8.74 48.0 320 0.0050 0.36 0.42 1.49 5.60 8.00 1.43 1.17 1.43 1.19 0.00 Existing 2'x2' Drop finte By Others A3 A3 2-90.3 + 35.3 *** 1.17 1.17 1.17 1.17 1.17 1.17 1.17																					<u> </u>							
A3	A1	A1	3+28.29 - 29.62' RT.	1.37	0.00	0.90	1.23	1.23	10.00	100	8.74	10.78	0.00	10.78	48.0	3200	0.0050	0.40	0.42	1.60	6.76	8.00	1.18	1.06	1.18	12.76	0.00	8 ft. Recessed Inlet On Grade
A4 S+33.89 - 30.50° RT	A2	A2	2+64.5 - 65.1' RT.	1.06	0.00	0.90	0.95	0.95	10.00	100	8.74	8.34	0.00	8.34	48.0	3200	0.0050	0.36	0.42	1.49	5.60		1.43	1.17	1.43	11.92	0.00	Existing 2'x2' Drop Inlet By Others
A5 A5 \$-52.75 - 47.8" RT 5.96 0.00 0.90 5.36 5.36 10.00 100 8.74 46.88 0.00 46.88 0.0 0 0.4417 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	A3	A3	2+90.34 - 35.31' LT.	0.32	0.00	0.90	0.29	0.29	10.00	100	8.74	2.52	0.00	2.52	48.0	3200	0.0050	0.23	0.42	1.13	2.22	8.00	3.60	1.83	3.60	9.06	0.00	8 ft. Recessed Inlet On Grade
A5A A5A \$467.65 - 61.96 RT 0.64 0.00 0.90 0.58 0.58 0.58 0.00 100 8.74 5.03 0.00 5.03 48.0 320 0.0050 0.30 0.42 1.32 3.83 5.00 1.31 1.41 1.31 6.58 0.00 Exist. 5 ft. Standard Inlet Low Pt. A7A A7A 6+05 - 30.50 LT. 0.26 0.00 0.90 0.58 0.58 1.00 100 8.74 2.05 0.00 0.00 0.50 48.0 3200 0.0050 0.30 0.42 1.32 3.83 5.00 1.31 1.41 1.31 6.58 0.00 Exist. 5 ft. Standard Inlet Low Pt. A7A A7A 6+05 - 30.50 LT. 0.26 0.00 0.90 0.58 0.58 1.00 100 8.74 2.05 0.00 0.50 48.0 3200 0.0050 0.30 0.42 1.32 3.83 5.00 1.31 1.41 1.31 6.58 0.00 Exist. 5 ft. Standard Inlet Low Pt. A7A A7A 6+05 - 30.50 LT. 0.20 0.00 0.90 0.58 0.58 1.00 100 8.74 2.05 0.00 0.55 48.0 3200 0.0050 0.30 0.42 1.09 1.88 6.00 3.19 1.97 3.19 6.53 0.00 6 ft. Recessed Inlet On Grade A8 A8 8+55.61 - 30.50 RT 0.74 0.00 0.90 0.48 0.48 1.00 100 8.74 1.75 0.00 1.75 48.0 3200 0.0050 0.32 0.42 1.36 4.27 10.00 2.34 1.33 2.34 13.62 0.00 10 ft. Recessed Inlet On Grade A9 9-91.07 42.65 RT 0.53 0.00 0.90 0.48 0.48 10.00 100 8.74 4.17 0.00 4.17 48.0 3200 0.0050 0.28 0.42 1.36 4.27 10.00 2.34 1.33 2.34 13.62 0.00 10 ft. Recessed Inlet On Grade A9 9-91.07 42.65 RT 0.53 0.00 0.90 0.48 0.48 10.00 100 8.74 4.17 0.00 4.17 48.0 3200 0.0050 0.28 0.42 1.26 3.31 5.00 1.51 1.51 1.51 1.51 6.30 0.00 Exist. 5 ft. Standard Inlet Low Pt. A10 1.01 10-10-30.50 RT. 0.50 0.00 0.90 0.45 0.45 10.00 100 8.74 1.37 0.00 4.17 48.0 3200 0.0050 0.28 0.42 1.26 3.31 5.00 1.51 1.51 1.51 1.51 6.30 0.00 Exist. 5 ft. Standard Inlet Low Pt. A11 1.41 1.01 10-30.50 RT. 0.50 0.00 0.90 0.45 0.45 10.00 100 8.74 1.30 0.00 1.57 48.0 3200 0.0050 0.28 0.42 1.24 3.16 8.00 2.53 1.55 2.53 9.55 0.00 8ft. Recessed Inlet Low Pt. A11 1.41 1.01 1.01 0.30.50 RT. 0.50 0.00 0.90 0.45 0.45 10.00 100 8.74 1.75 0.00 1.75 48.0 3200 0.0066 0.15 0.42 0.95 1.15 8.00 1.51 5.15 1.51 1.51 0.50 0.00 8ft. Recessed Inlet Low Pt. A11 1.41 1.01 1.01 0.30.50 RT. 0.50 0.00 0.90 0.45 0.45 1.00 1.00 1.00 8.74 1.75 0.00 1.00 0.00 0.50 0.00 0.50 0.00 0.50 0.5	A4	A4	5+33.89 - 30.50' RT.	0.17	0.00	0.90	0.15	0.15	10.00	100	8.74	1.34	0.00	1.34	48.0	3200	0.0050	0.18	0.42	1.01	1.33	8.00	6.03	2.31	6.03	8.06	0.00	8 ft. Recessed Inlet On Grade
A6 A6 746949 - 60.83° KT	<b>A</b> 5	A5	5+52.75 - 47.83' RT.	5.96	0.00	0.90	5.36	5.36	10.00	100	8.74	46.88	0.00	46.88	0.0	0	0.4417	0.00	0.00	0.00	0.00	2.75	0.00	0.00	0.00	0.00	0.00	16 ft. Standard Inlet Low Pt.
A7A	A5A	A5A	5+67.65 - 61.96' RT	0.64	0.00	0.90	0.58	0.58	10.00	_100	8.74	5.03	0.00	5.03	48.0	3200	0.0050	0.30	0.42	1.32	3.83	5.00	1.31	1.41	1.31	6.58	0.00	Exist. 5 ft. Standard Inlet Low Pt.
A7B	A6	A6	7+69.49 - 60.83' RT.	0.64	0.00	0.90	0.58	0.58	10.00	100	8.74	5.03	0.00	5.03	48.0	3200	0.0050	0.30	0.42	1.32	3.83	5.00	1.31	1.41	1.31	6.58	0.00	Exist, 5 ft. Standard Inlet Low Pt.
A8 A8 8+55.61-30.50 RT. 0.74 0.00 0.90 0.67 0.67 10.00 100 8.74 5.82 0.00 5.82 48.0 3200 0.050 0.32 0.42 1.36 4.27 10.00 2.34 1.33 2.34 13.62 0.00 10 fi. Recessed Inlet On Grade  A9 A9 9+91.07-42.65 RT 0.53 0.00 0.90 0.48 0.48 10.00 100 8.74 4.17 0.00 4.17 48.0 3200 0.050 0.28 0.42 1.26 3.31 5.00 1.51 1.51 1.51 6.30 0.00 Exist 5 ft. Standard Inlet Low Pt.  A10 A10 10+10-30.50 RT. 0.20 0.00 0.90 0.45 0.45 10.00 100 8.74 1.57 0.00 1.57 48.0 3200 0.066 0.18 0.42 1.01 1.55 8.00 5.15 2.29 5.15 8.10 0.00 8 ft. Recessed Inlet Low Pt.  A11 A11 10+10-30.50 LT. 0.50 0.00 0.90 0.45 0.45 10.00 100 8.74 1.10 0.00 1.10 48.0 3200 0.0066 0.16 0.42 0.95 1.15 8.00 6.94 2.62 6.94 7.64 0.00 8 ft. Recessed Inlet Low Pt.  A12 A12 10+16-37-41.75 RT 0.33 0.00 0.90 0.33 0.33 10.00 100 8.74 2.91 0.00 8.74 2.91 0.00 2.91 48.0 3200 0.0066 0.16 0.42 0.95 1.15 8.00 6.94 2.62 6.94 7.64 0.00 8 ft. Recessed Inlet Low Pt.  A13 A13 12+57.80 4.1.72 RT 0.37 0.00 0.90 0.33 0.33 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.95 1.15 2.00 1.73 1.20 3.50 0.00 8 ft. Recessed Inlet Low Pt.  A14 A15 10+10-30.50 LT. 0.50 0.00 0.90 0.33 0.33 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.26 0.26 0.24 1.17 2.50 3.00 1.20 1.73 1.20 3.50 0.00 Exist 4 ft. Standard Inlet Low Pt.  A15 A15 12+57.80 4.1.72 RT 0.37 0.05 0.00 0.90 0.14 0.14 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.97 1.22 6.00 4.91 2.56 4.91 5.79 0.00 Exist 4 ft. Standard Inlet Low Pt.  CEI CEI 14+08-30.50 RT 0.15 0.00 0.90 0.14 0.14 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.97 1.22 6.00 4.91 2.56 4.91 5.79 0.00 6 ft. Recessed Inlet Low Pt.  CE3 CE3 14+01.60 3.050 LT 0.16 0.00 0.90 0.14 0.14 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.97 1.22 6.00 4.91 2.56 4.91 5.79 0.00 6 ft. Recessed Inlet Low Pt.  CE4 CE4 12+00-30.50 LT 0.16 0.00 0.90 0.40 0.40 0.40 0.40 1.00 1.00 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.97 1.22 6.00 4.91 2.56 6.54 7.72 0.00 8 ft. Recessed Inlet Low Pt.	A7A	A7A	6+05 - 30.50' LT.	0.26	0.00	0.90	0.23	0.23	10.00	100	8.74	2.05	0.00	2.05	48.0	3200	0.0050	0.21	0.42	1.09	1.88	6.00	3.19	1.97	3.19	6.53	0.00	6 ft. Recessed Inlet On Grade
A9 9-9-1.07 - 42.65° RT 0.53 0.00 0.90 0.48 0.48 10.00 100 8.74 4.17 0.00 4.17 48.0 3200 0.055 0.28 0.42 1.26 3.31 5.00 1.51 1.51 1.51 6.30 0.00 Exist. 5 ft. Standard Inlet Low Pt.  A10 A10 10+10 - 30.50° RT 0.50 0.00 0.00 0.00 0.00 0.00 0.00 0.0	A7B	A7B	8+35.94 - 30.50' LT.	0.20	0.00	0.90	0.18	0.18	10.00	100	8.74	1.57	0.00	1.57	48.0	3200	0.0050	0.19	0.42	1.04	1.52	6.00	3.95	2.18	3.95	6.22	0.00	6 ft. Recessed Inlet On Grade
A10	A8	A8	8+55.61 - 30.50 RT.	0.74	0.00	0.90	0.67	0.67	10.00	100	8.74	5.82	0.00	5.82	48.0	3200	0.0050	0.32	0.42	1.36	4.27	10.00	2.34	1.33	2.34	13.62	0.00	10 ft. Recessed Inlet On Grade
A10 A10 10+10-30.50°RT. 0.20 0.00 0.90 0.18 0.18 10.00 100 8.74 1.57 0.00 1.57 48.0 3200 0.0066 0.18 0.42 1.01 1.55 8.00 5.15 2.29 5.15 8.10 0.00 8.6. Recessed Inlet Low Pt.  A11 A11 10+10-30.50°LT. 0.14 0.00 0.90 0.13 0.13 10.00 100 8.74 1.10 0.00 1.00 48.0 3200 0.0066 0.16 0.42 0.95 1.15 8.00 6.94 2.62 6.94 7.64 0.00 8.6. Recessed Inlet Low Pt.  A12 10+10-30.50°LT. 0.50 0.00 0.90 0.45 0.45 10.00 100 8.74 2.60 0.00 8.74 2.60 0.00 3.94 4.00 3.20 0.0066 0.26 0.26 0.24 1.21 3.26 8.00 2.45 1.63 2.45 9.65 0.00 8.6. Recessed Inlet Low Pt.  A13 A13 12+57.80 -41.72°RT 0.37 0.37 0.00 0.90 0.33 0.33 10.00 10.00 8.74 2.61 0.00 8.74 2.61 0.00 2.45 48.0 3200 0.0056 0.24 0.42 1.17 2.50 3.00 1.20 1.25 1.25 1.25 1.25 1.25 1.25 1.25 1.25	A9	A9	9+91.07 - 42.65' RT	0.53	0.00	0.90	0.48	0.48	10.00	_100	8.74	4.17	0.00	4.17	48.0	3200	0.0050	0.28	0.42	1.26	3.31	5.00	1.51	1.51	1.51	6.30	0.00	Exist. 5 ft. Standard Inlet Low Pt.
A11  A11  10+10 -30.50' LT.  0.14  0.00  0.90  0.13  0.13  10.00  100  8.74  1.10  0.00  1.10  48.0  3200  0.066  0.16  0.42  0.95  1.15  8.00  6.94  2.62  6.94  7.64  0.00  8 ft. Recessed Inlet Low Pt.  A12  10+16.37 - 41.75' RT  0.33  0.00  0.90  0.30  0.30  1.00  1.00  8.74  2.60  0.00  2.60  48.0  3200  0.056  0.23  0.23  1.17  2.50  3.00  1.20  1.73  1.20  3.50  0.00  8 ft. Recessed Inlet Low Pt.  A13  A13  12+57.80 - 41.72' RT  0.37  0.00  0.90  0.33  0.33  1.00  1.00  1.00  8.74  2.91  0.00  2.91  48.0  3200  0.056  0.24  0.24  1.21  3.26  8.00  2.45  1.63  2.45  9.65  0.00  8 ft. Recessed Inlet Low Pt.  CE1  CE1  14+08 - 30.50' RT  0.15  0.00  0.90  0.14  0.14  10.00  1.00  8.74  1.18  0.00  1.18  48.0  3200  0.066  0.16  0.42  0.97  1.22  6.00  4.91  2.56  4.91  5.79  0.00  6 ft. Recessed Inlet Low Pt.  CE3  CE3  14+01.60 - 30.50' LT  0.16  0.00  0.90  0.40  0.40  1.00  1.00  8.74  1.26  0.00  1.26  48.0  3.00  0.066  0.15  0.25  0.42  1.17  2.95  8.00  2.72  1.71  2.72  9.40  0.00  8 ft. Recessed Inlet Low Pt.	A10	A10	10+10 - 30.50' RT.	0.50	0.00	0.90	0.45	0.45	10.00	100	8.74	3.93	0.00	3.93	48.0	3200	0.0050	0.27	0.42	1.24	3.16	8.00	2.53	1.54	2.53	9.95	0.00	8 ft. Recessed Inlet Low Pt.
A11	A10	A10	10+10 - 30.50' RT.	0.20	0.00	0.90	0.18	0.18	10.00	100	8.74	1.57	0.00	1.57	48.0	3200	0.0066	0.18	0.42	1.01	1.55	8.00	5.15	2.29	5.15	8.10	0.00	8 ft. Recessed Inlet Low Pt.
A12	A11	A11	10+10 -30.50' LT.	0.14	0.00	0.90	0.13	0.13	10.00	100	8.74	1.10	0.00	1.10	48.0	3200	0.0066	0.16	0.42	0.95	1.15	8.00	6.94	2.62	6.94	7.64	0.00	8 ft. Recessed Inlet Low Pt.
A13	A11	A11	10+10 -30.50' LT.	0.50	0.00	0.90	0.45	0.45	10.00	100	8.74	3.93	0.00	3.93	48.0	3200	0.0066	0.26	0.42	1.21	3.26	8.00	2.45	1.63	2.45	9.65	0.00	8 ft. Recessed Inlet Low Pt.
CE1 CE1 14+08 - 30.50' RT 0.15 0.00 0.90 0.14 0.14 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.97 1.22 6.00 4.91 2.56 4.91 5.79 0.00 6 ft. Recessed Inlet On Grade  CE2 CE2 12+00 - 30.50' LT 0.16 0.00 0.90 0.14 0.14 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.97 1.22 8.00 6.54 2.56 6.54 7.72 0.00 8 ft. Recessed Inlet Low Pt.  CE3 CE3 14+01.60 - 30.50' LT 0.16 0.00 0.90 0.40 0.40 10.00 100 8.74 1.26 0.00 1.26 48.0 3200 0.0066 0.17 0.42 0.98 1.29 6.00 4.65 2.49 4.65 5.85 0.00 6 ft. Recessed Inlet On Grade  CE4 CE4 12+00 - 30.50' LT 0.44 0.00 0.90 0.40 0.40 10.00 100 8.74 3.46 0.00 3.46 48.0 3200 0.0066 0.25 0.42 1.17 2.95 8.00 2.72 1.71 2.72 9.40 0.00 8 ft. Recessed Inlet Low Pt.	A12	A12	10+16.37 - 41.75' RT	0.33	0.00	0.90	0.30	0.30	10.00	100	8.74	2.60	0.00	2.60	48.0	3200	0.0050	0.23	0.42	1.14	2.28	4.00	1.76	1.80	1.76	4.56	0.00	Exist. 4 ft. Standard Inlet Low Pt.
CE2 12+00 - 30.50' RT 0.15 0.00 0.90 0.14 0.14 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.97 1.22 8.00 6.54 2.56 6.54 7.72 0.00 8 ft. Recessed Inlet Low Pt.  CE3 CE3 14+01.60 - 30.50' LT 0.16 0.00 0.90 0.14 0.14 10.00 100 8.74 1.26 0.00 1.26 48.0 3200 0.0066 0.17 0.42 0.98 1.29 6.00 4.65 2.49 4.65 5.85 0.00 6 ft. Recessed Inlet On Grade  CE4 CE4 12+00 - 30.50' LT 0.44 0.00 0.90 0.40 0.40 10.00 100 8.74 3.46 0.00 3.46 48.0 3200 0.0066 0.25 0.42 1.17 2.95 8.00 2.72 1.71 2.72 9.40 0.00 8 ft. Recessed Inlet Low Pt.	A13	A13	12+57.80 - 41.72' RT	0.37	0.00	0.90	0.33	0.33	10.00	100	8.74	2.91	0.00	2.91	48.0	3200	0.0050	0.24	0.42	1.17	2.50	3.00	1.20	1.73	1.20	3.50	0.00	Exist. 3 ft. Standard Inlet Low Pt.
CE2 12+00 - 30.50' RT 0.15 0.00 0.90 0.14 0.14 10.00 100 8.74 1.18 0.00 1.18 48.0 3200 0.0066 0.16 0.42 0.97 1.22 8.00 6.54 2.56 6.54 7.72 0.00 8 ft. Recessed Inlet Low Pt.  CE3 CE3 14+01.60 - 30.50' LT 0.16 0.00 0.90 0.14 0.14 10.00 100 8.74 1.26 0.00 1.26 48.0 3200 0.0066 0.17 0.42 0.98 1.29 6.00 4.65 2.49 4.65 5.85 0.00 6 ft. Recessed Inlet On Grade  CE4 CE4 12+00 - 30.50' LT 0.44 0.00 0.90 0.40 0.40 10.00 100 8.74 3.46 0.00 3.46 48.0 3200 0.0066 0.25 0.42 1.17 2.95 8.00 2.72 1.71 2.72 9.40 0.00 8 ft. Recessed Inlet Low Pt.																												
CE3	CE1	CE1	14+08 - 30.50' RT	0.15	0.00	0.90	0.14	0.14	10.00	_100	8.74	1.18	0.00	1.18	48.0	3200	0.0066	0.16	0.42	0.97	1.22	6.00	4.91	2.56	4.91	5.79	0.00	6 ft. Recessed Inlet On Grade
CE4 CE4 12+00 - 30.50' LT. 0.44 0.00 0.90 0.40 0.40 10.00 100 8.74 3.46 0.00 3.46 48.0 3200 0.0066 0.25 0.42 1.17 2.95 8.00 2.72 1.71 2.72 9.40 0.00 8 ft, Recessed Inlet Low Pt.	CE2	CE2	12+00 - 30.50' RT	0.15	0.00	0.90	0.14	0.14	10.00	100	8.74	1.18	0.00	1.18	48.0	3200	0.0066	0.16	0.42	0.97	1.22	8.00	6.54	2.56	6.54	7.72	0.00	8 ft. Recessed Inlet Low Pt.
	CE3	CE3	14+01.60 - 30.50' LT	0.16	0.00	0.90	0.14	0.14	10.00	100	8.74	1.26	0.00	1.26	48.0	3200	0.0066	0.17	0.42	0.98	1.29	6.00	4.65	2.49	4.65	5.85	0.00	6 ft. Recessed Inlet On Grade
CE CE 14+91.47 - 41.31'RT 0.26 0.00 0.90 0.23 0.23 10.00 100 8.74 2.05 0.00 2.05 48.0 3200 0.3333 0.10 0.75 1.68 1.21 12.00 9.88 7.75 9.88 20.21 0.00 3 ft,x 3 ft. Drop Inlet Low Pt.    A	CE4	CE4	12+00 - 30.50' LT.	0.44	0.00	0.90	0.40	0.40	10.00	100	8.74	3.46	0.00	3.46	48.0	3200	0.0066	0.25	0.42	1.17	2.95	8.00	2.72	1.71	2.72	9.40	0.00	8 ft. Recessed Inlet Low Pt.
	CE	CE	14+91.47 - 41.31' RT	0.26	0.00	0.90	0.23	0.23	10.00	100	8.74	2.05	0.00	2.05	48.0	3200	0.3333	0.10	0.75	1.68	1.21	12.00	9.88	7.75	9.88	20.21	0.00	3 ft.x 3 ft. Drop Inlet Low Pt.
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# TOWN OF ADDISON, TEXAS

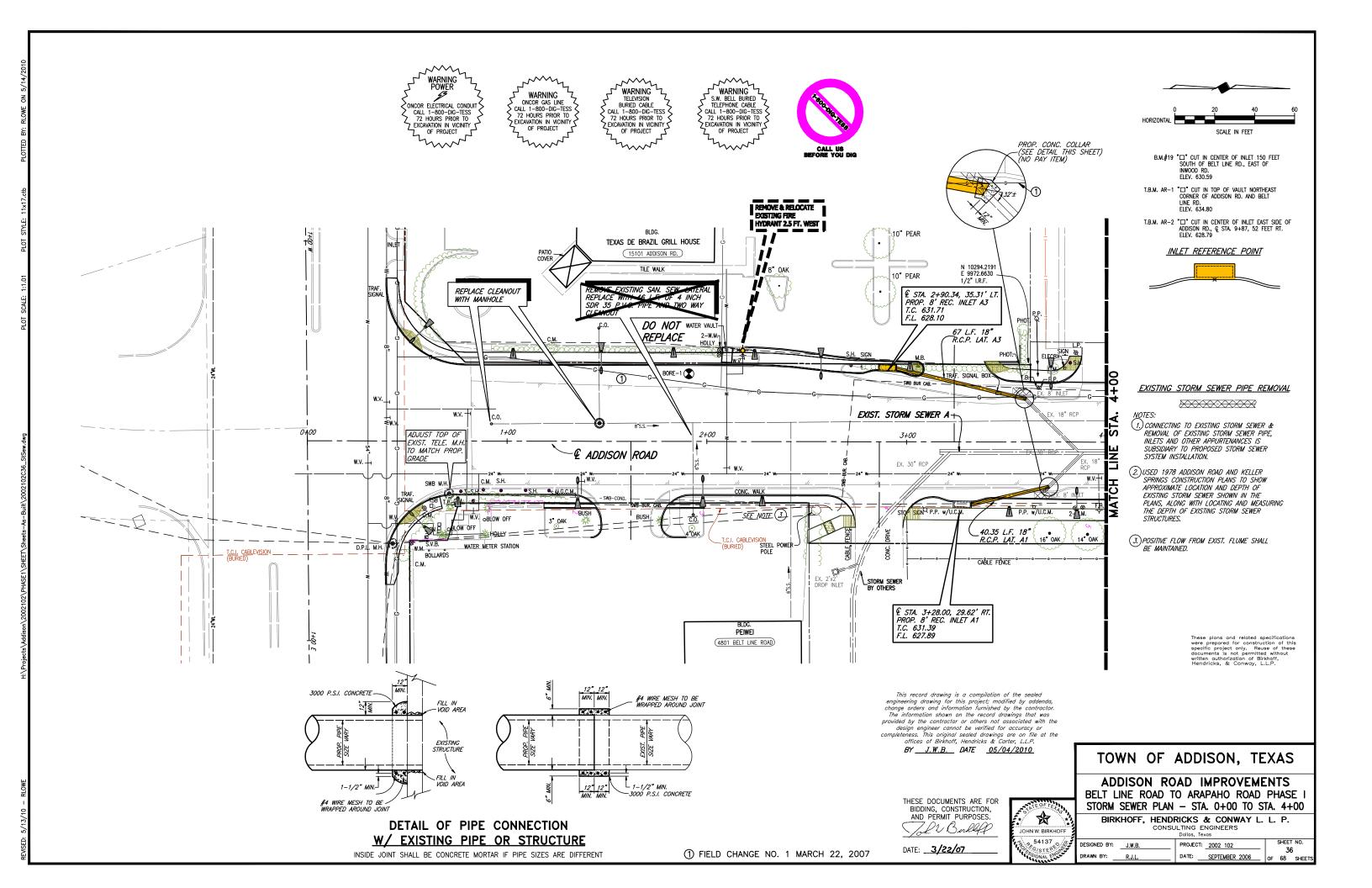
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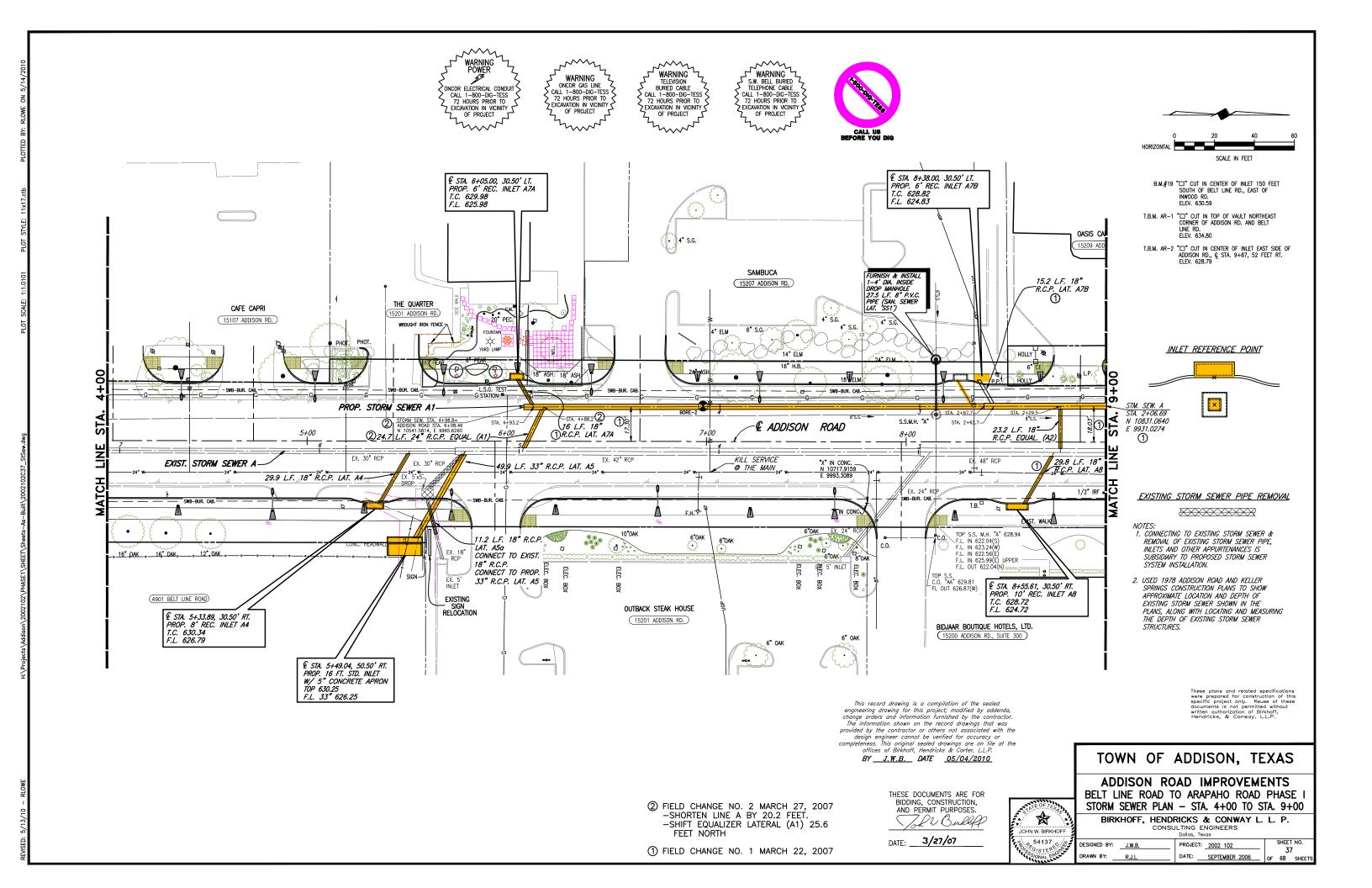
BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS
Dollos, Texos

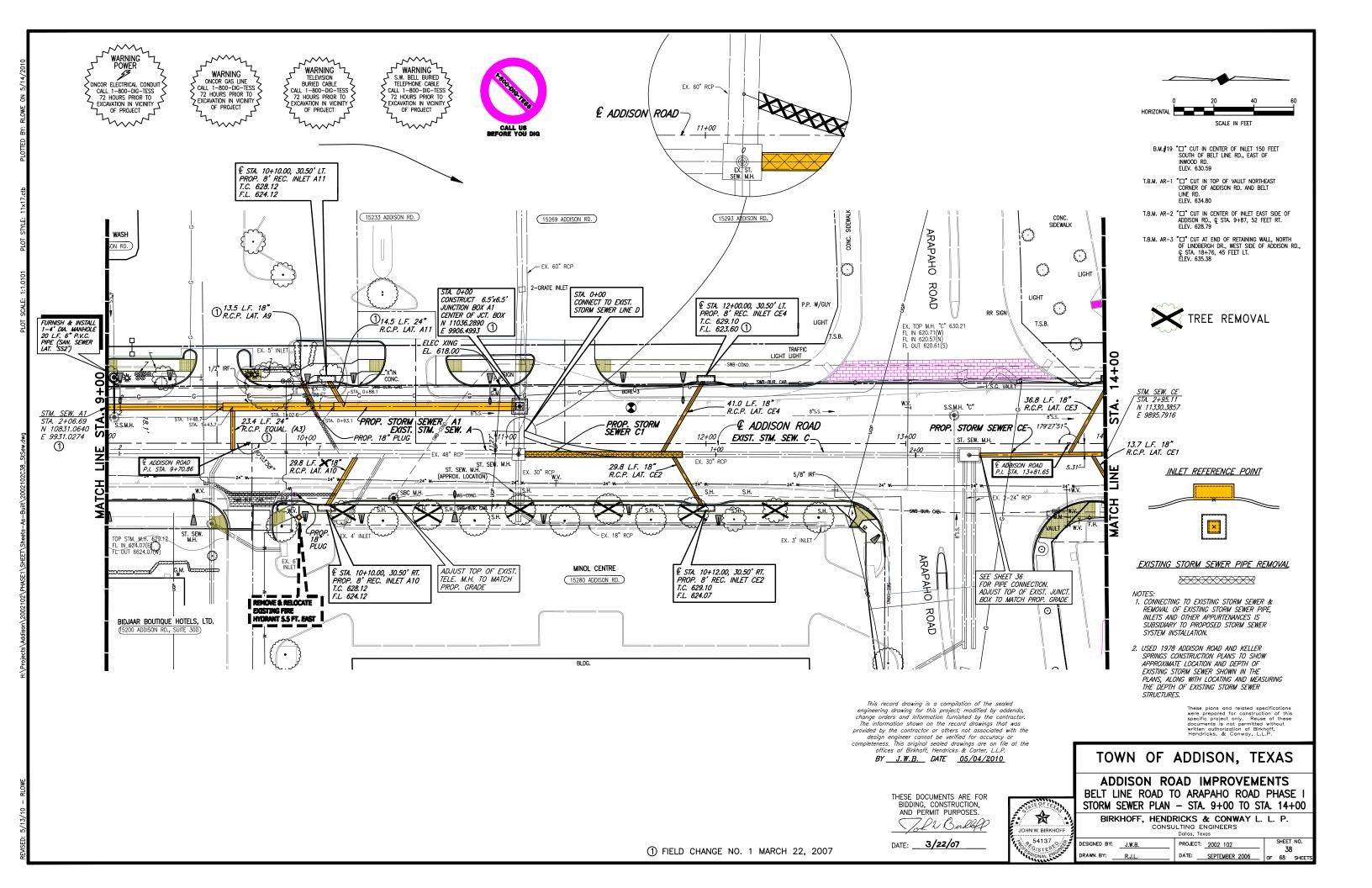
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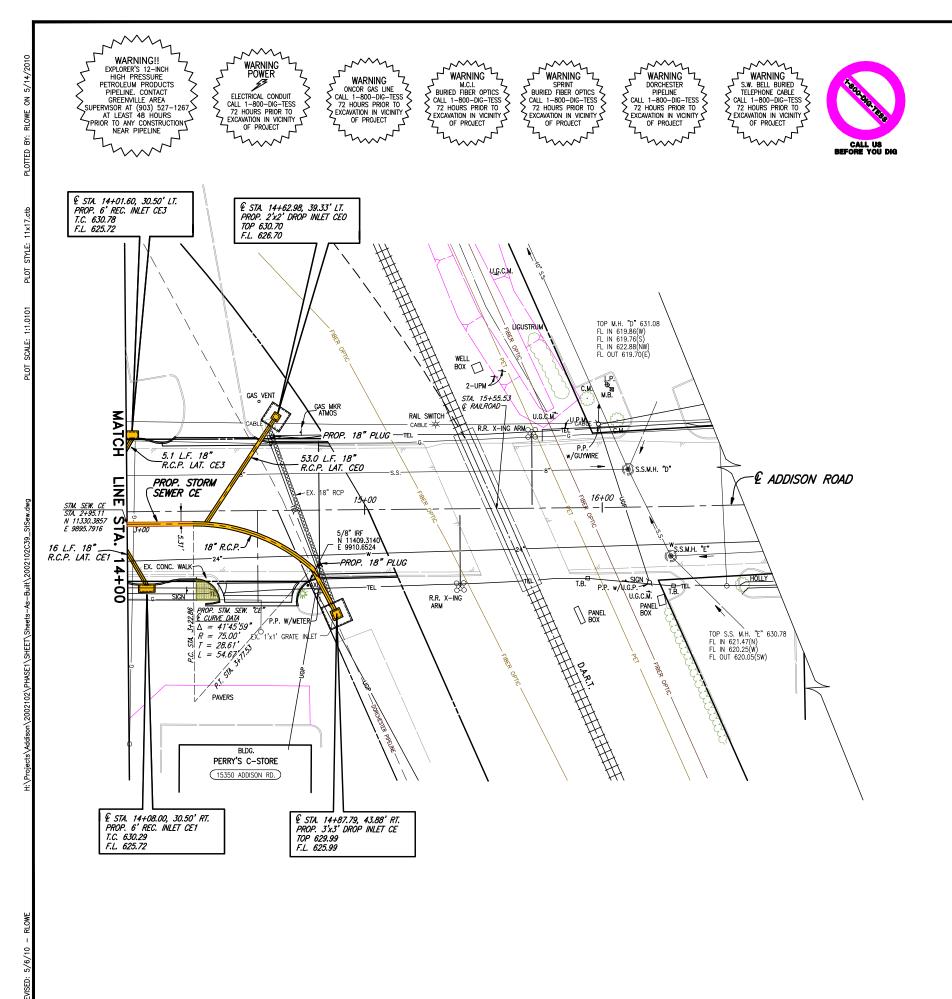
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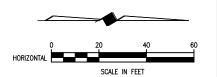
JOHN W. BIRKHOFF DATE: \_\_\_











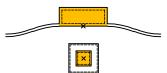
B.M.#19 <sup>™</sup>□ CUT IN CENTER OF INLET 150 FEET SOUTH OF BELT LINE RD., EAST OF INWOOD RD. ELEV. 630.59

T.B.M. AR-1 """ CUT IN TOP OF VAULT NORTHEAST CORNER OF ADDISON RD. AND BELT LINE RD. ELEY. 634.80

T.B.M. AR-2 "□" CUT IN CENTER OF INLET EAST SIDE OF ADDISON RD., © STA. 9+87, 52 FEET RT. ELEV. 628.79

T.B.M. AR-3 "[]" CUT AT END OF RETAINING WALL, NORTH OF LINDBERCH DR., WEST SIDE OF ADDISON RD., © STA. 18+76, 45 FEET LT. ELEV. 655.38

#### INLET REFERENCE POINT



#### EXISTING STORM SEWER PIPE REMOVAL

#### NOTES

- 1. CONNECTING TO EXISTING STORM SEWER & REMOVAL OF EXISTING STORM SEWER PIPE, INLETS AND OTHER APPURTENANCES IS SUBSIDIARY TO PROPOSED STORM SEWER SYSTEM INSTALLATION.
- 2. USED 1978 ADDISON ROAD AND KELLER SPRINGS CONSTRUCTION PLANS TO SHOW APPROXIMATE LOCATION AND DEPTH OF EXISTING STORM SEWER SHOWN IN THE PLANS, ALONG WITH LOCATING AND MEASURING THE DEPTH OF EXISTING STORM SEWER STRUCTURES.

These plans and related specifications were prepared for construction of this specific project only. Reuse of these documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.

This record drawing is a compilation of the sealed engineering drawing for this project; modified by addenda, change orders and information furnished by the contractor. The information shown on the record drawings that was provided by the contractor or others not associated with the design engineer cannot be verified for accuracy or completeness. This original sealed drawings are on file at the offices of Birkhoff, Hendricks & Carter, L.L.P.

BY <u>J.W.B.</u> DATE <u>05/04/2010</u>

THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.

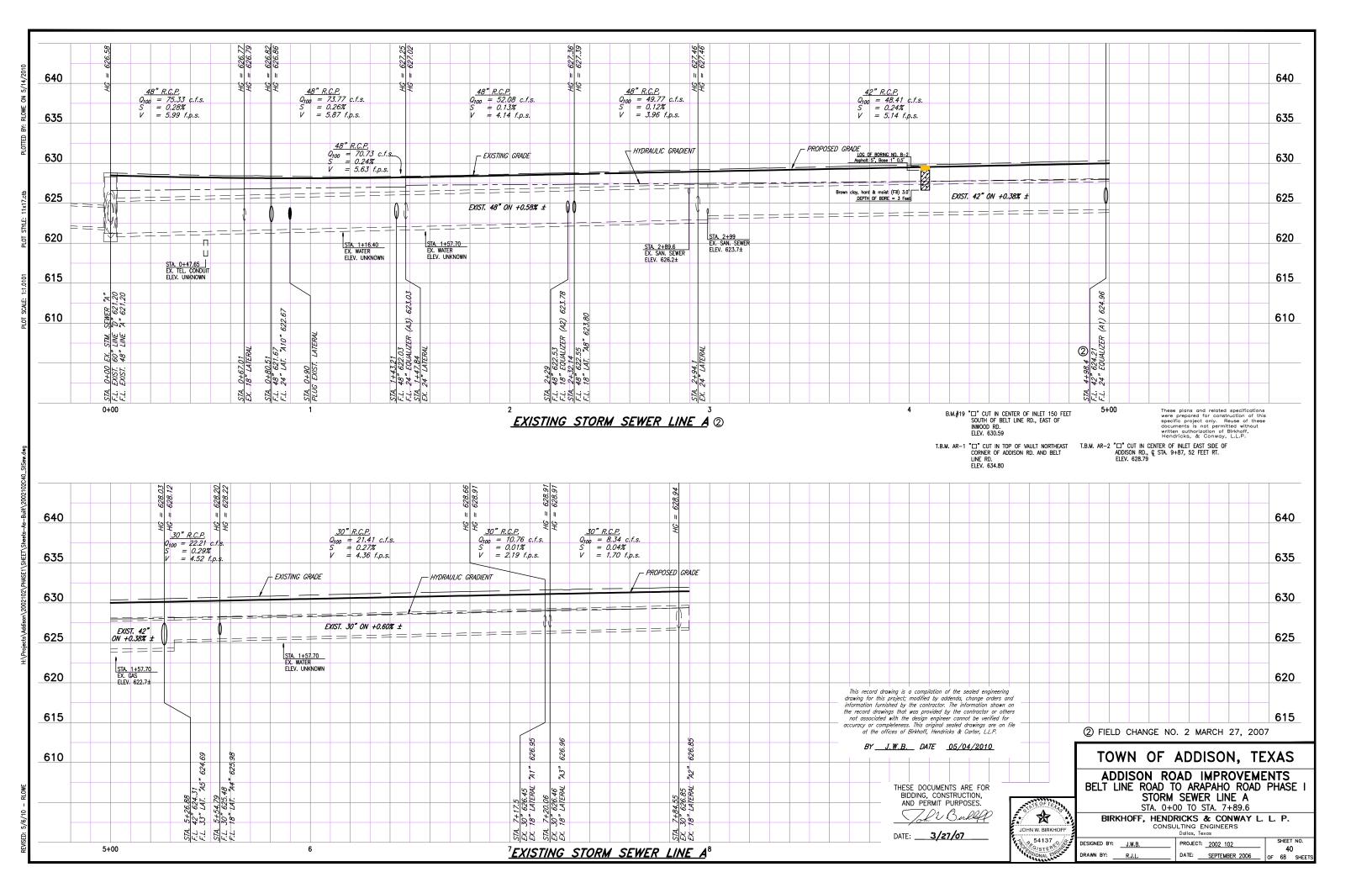
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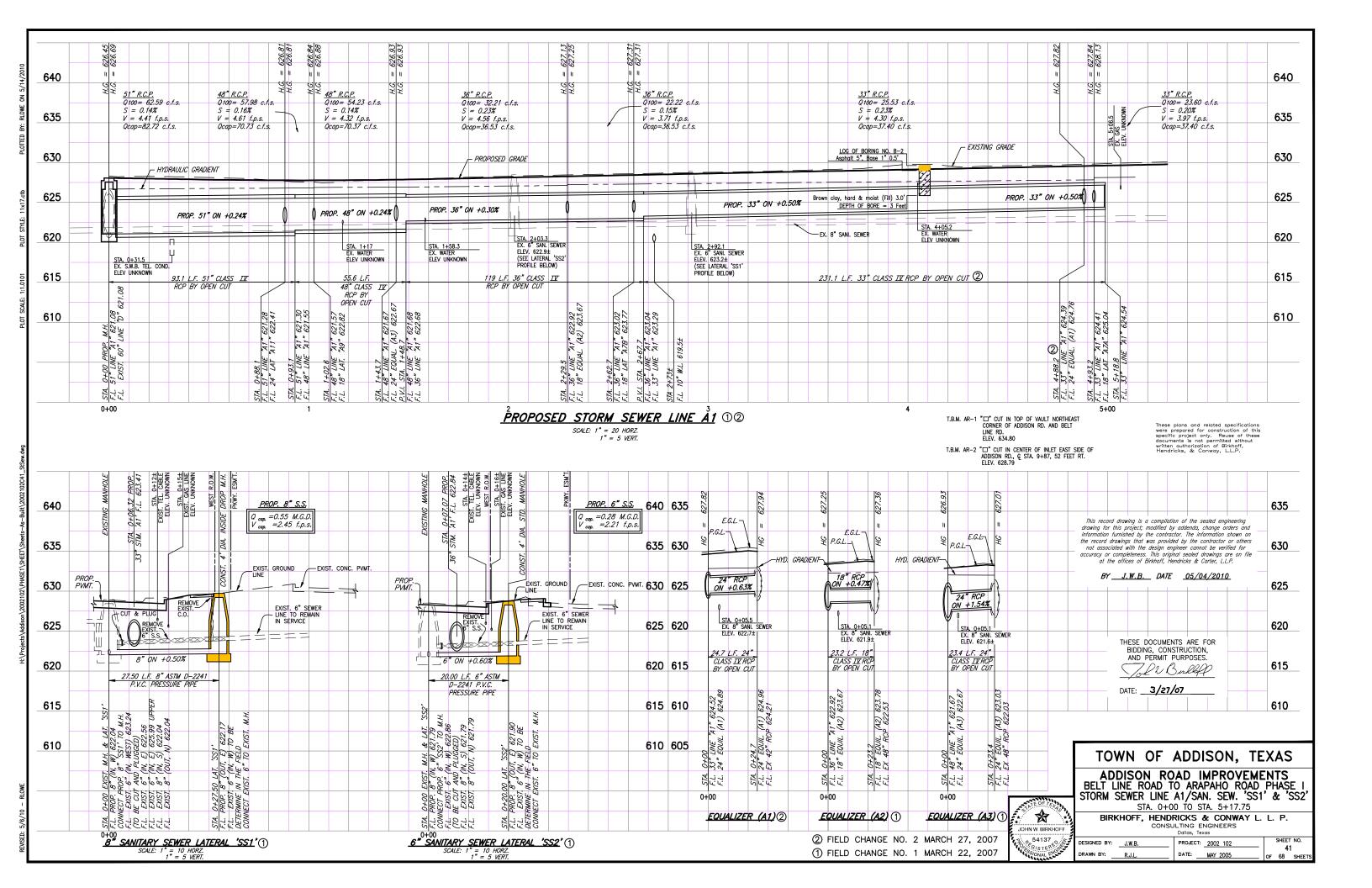


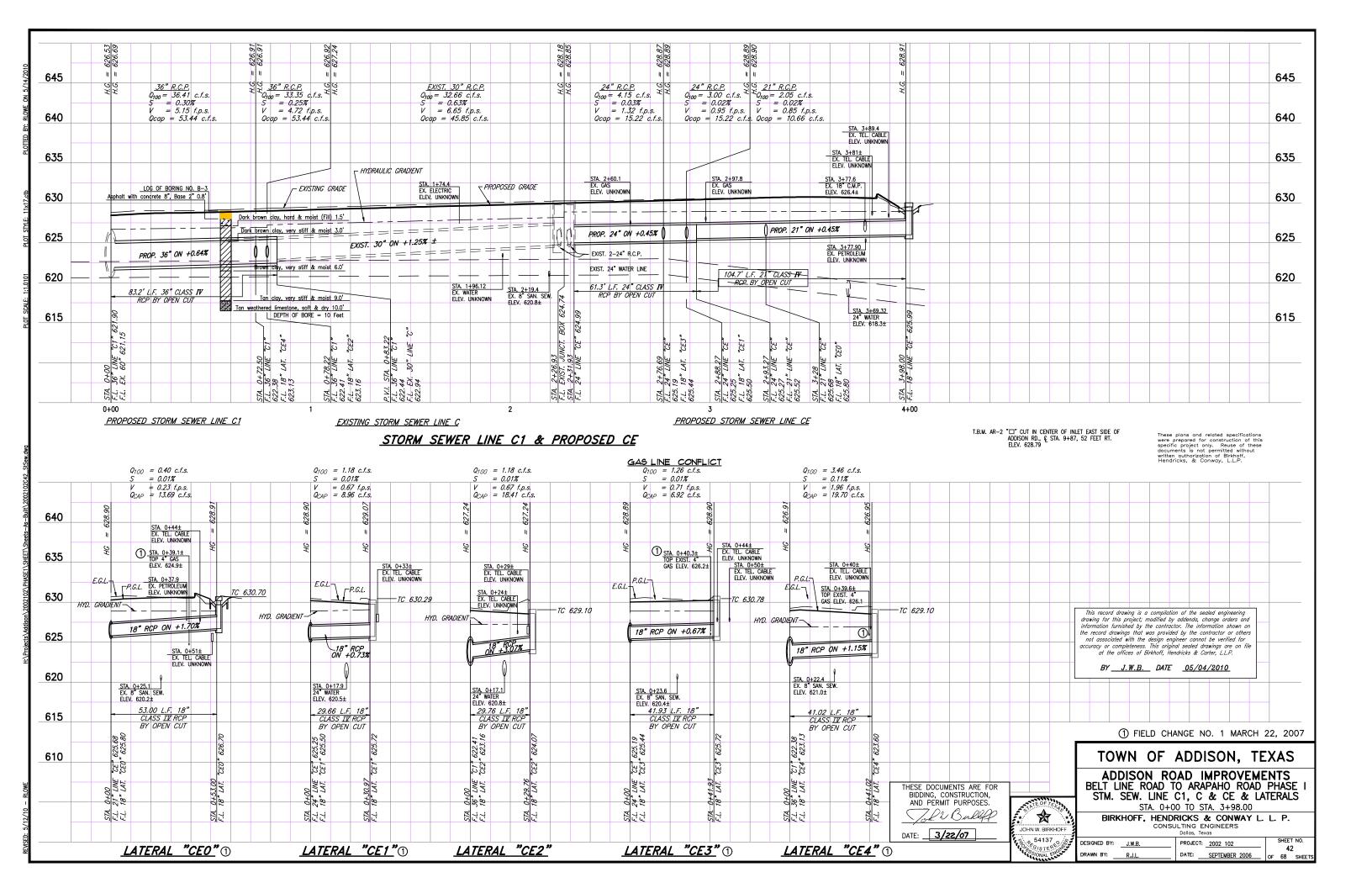
## TOWN OF ADDISON, TEXAS

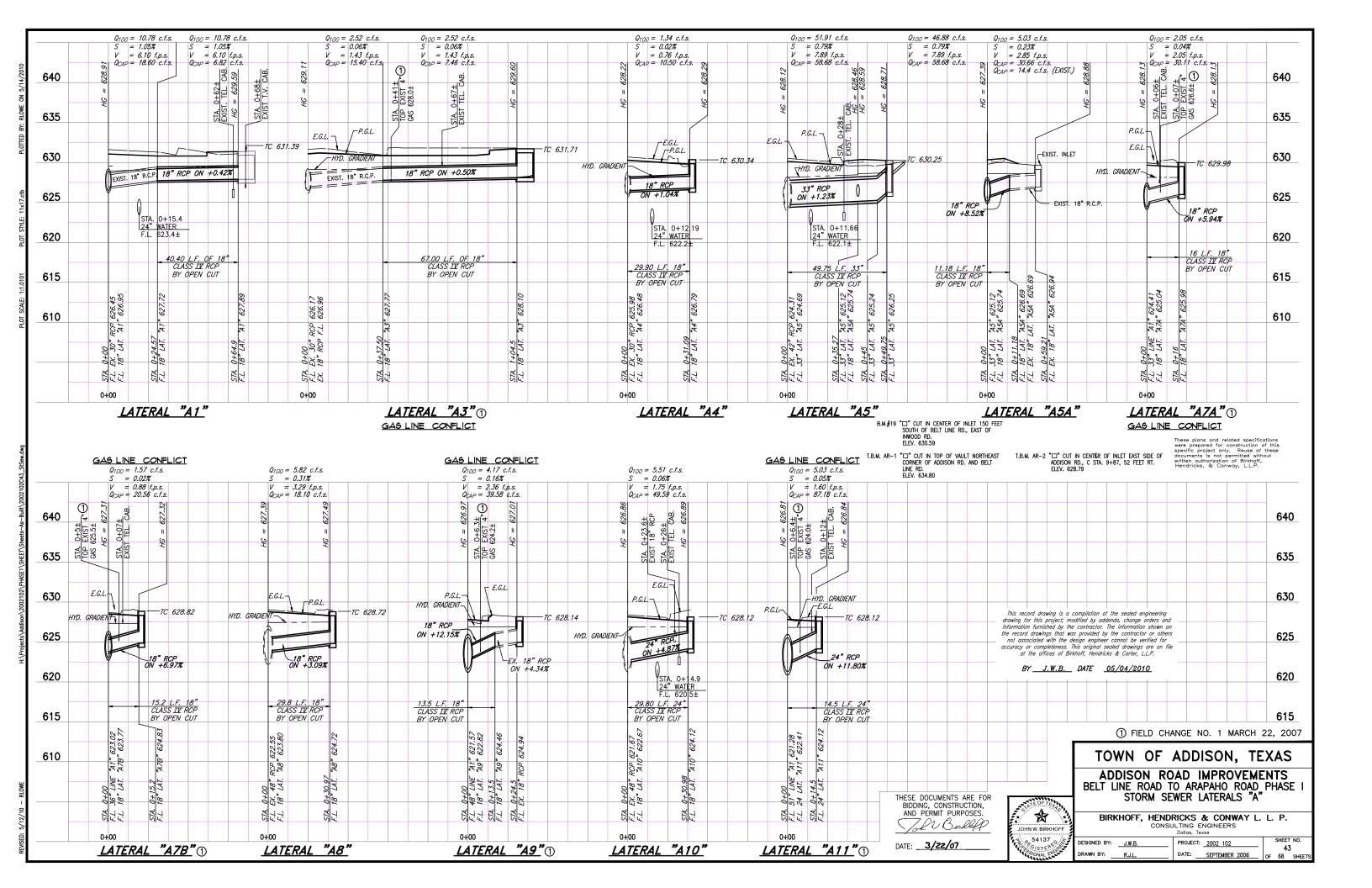
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ARAPAHO ROAD TO ARAPAHO ROAD PHASE I
STORM SEWER PLAN - STA. 14+00 TO STA. 15+37

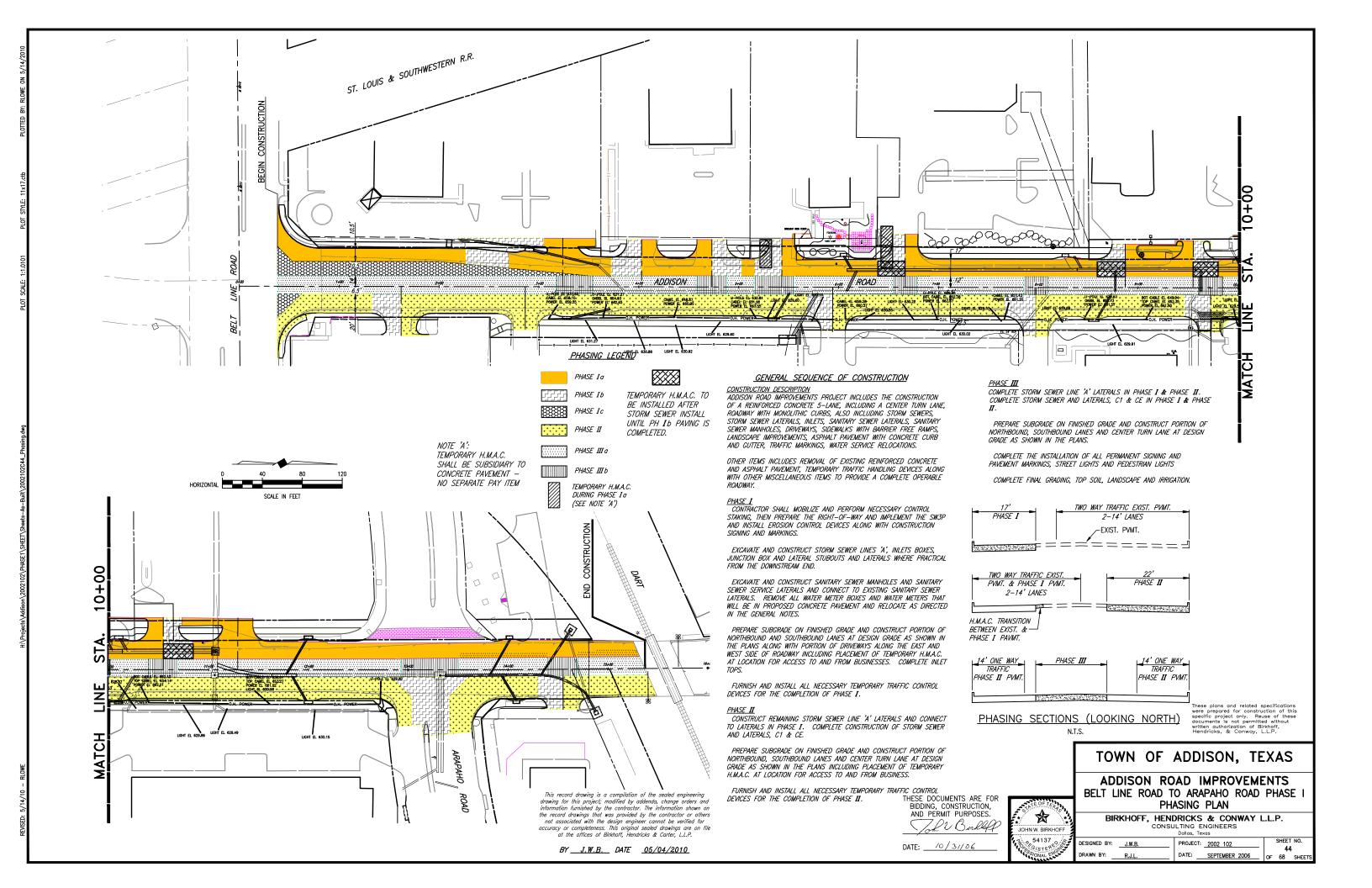
BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

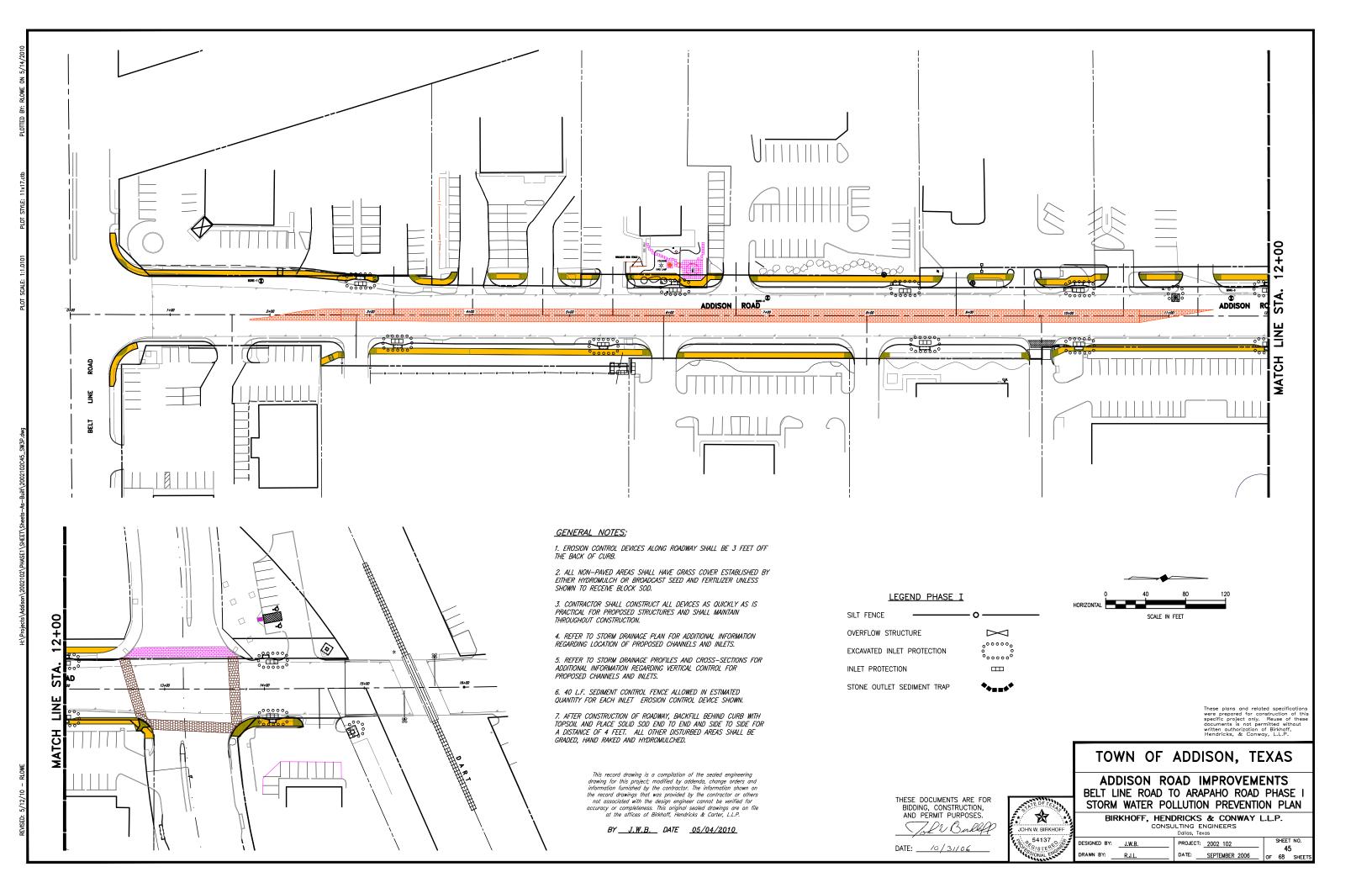












PROJEC	T DESCRIPTION: THE CONSTRUCTION GENERALLY OF A 5 LANE REINFORCED CONCRETE ROADWAY REPLACING A 4 LANE ASPHALT ROADWAY
=	
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-	
=	
_	
_ _ _	
_ _ _	
MAJOR	SOIL DISTURBING ACTIVITIES: SOIL DISTURBING ACTIVITIES WILL INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING:
_	PREPARATION OF RIGHT-OF-WAY, EXCAVATION AND EMBANKMENT FOR ROADWAY STORM SEWER CONSTRUCTION, UTILITY CONSTRUCTION AND ACCESS ENTRANCE CONSTRUCTION FROM EXISTING ROADWAYS.
_	Continued from Enterine floribinities.
_	
_	
_	
_	
TOTAL	DRAINAGE AREA: 50 ACRES
	Drainage area: <i>50 ACRES</i> Drainage area to be disturbed: <i>4 ACRES</i>
TOTAL I	DRAINAGE AREA TO BE DISTURBED: <u>4 ACRES</u> ED RUNOFF COEFFICIENT PAVED AREAS 0.90 (1.7 ACRES)
TOTAL I	ED RUNOFF COEFFICIENT  PAVED AREAS 0.90 (1.7 ACRES)  GRASS AREAS 0.50 (2.3 ACRES)
TOTAL I	DRAINAGE AREA TO BE DISTURBED: <u>4 ACRES</u> ED RUNOFF COEFFICIENT PAVED AREAS 0.90 (1.7 ACRES)
TOTAL I	ED RUNOFF COEFFICIENT  PAVED AREAS 0.90 (1.7 ACRES)  GRASS AREAS 0.50 (2.3 ACRES)
TOTAL I	ED RUNOFF COEFFICIENT  PAVED AREAS 0.90 (1.7 ACRES)  GRASS AREAS 0.50 (2.3 ACRES)
TOTAL I	ED RUNOFF COEFFICIENT  PAVED AREAS 0.90 (1.7 ACRES)  GRASS AREAS 0.50 (2.3 ACRES)
WEIGHTI EXISTIN COVER	ED RUNOFF COEFFICIENT  PAVED AREAS 0.90 (1.7 ACRES)  GRASS AREAS 0.50 (2.3 ACRES)
WEIGHTI EXISTIN COVER	DRAINAGE AREA TO BE DISTURBED: 4 ACRES  ED RUNOFF COEFFICIENT PAVED AREAS 0.90 (1.7 ACRES) GRASS AREAS 0.50 (2.3 ACRES)  G CONDITION OF SOIL & VEGETATIVE AND % OF EXISTING VEGETATIVE COVER: THE EXISTING SOILS ARE GENERALLY BROWN CLAY WITH LIMESTONE FRAGMENTS.

# SEDIMENT CONTROLS

	<u> </u>	ROSION	AND	SEDIMENT	CO
SOIL STAL	BILIZATION PRACTICES:		1 (	OTHER EROSION A	ND SEI
	TEMPORARY SEEDING		,	MAINTENANCE: <u>ALL ER</u>	OSION A
	PERMANENT PLANTING, SODDING, OR SEEDING			<u>IF A REPAIR IS</u> THAN 7 CALEN	
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	BUFFER ZONES			AND DRAINAGE	WAYS S
	PRESERVATION OF NATURAL RESOURCES				
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	ALL BE EITHER SEEDED OR SODDED WITHIN 14 DAYS UNLESS CONSTRUCTION ACTIVI HEDULED TO RESUME WITHIN 21 DAYS.	TIES ARE	.   '	NSPECTION: <u>AN INSE</u> <u>WITH THE MOS</u>	<u> PECTION</u> T. RECEN
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X	CURBS AND GUTTERS			HAZARDOUS, TI	HE CONT
<u>X</u>	STORM SEWERS VELOCITY CONTROL DEVICES				
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THE	ORDER OF ACTIVITIES WILL BE AS FOLLOWS:			X_ HAUL ROAI LOADED HA	
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	PREPARE THE RIGHT-OF-WAY. INSTALL INLET PROTECTION			STABILIZED	CONST
				OTHER:	NTRACTO
<u>2)</u>	CONSTRUCT UTILITIES, STORM SEWERS AND DRAINAGE CHANNEL			<u>MEASURES FOR</u> SUCH AS FOR	
			·	PRACTICES SHA	ILL BE A
<u>3)</u>	BEGIN EXCAVATION OF ROADWAY, STABILIZE AND PLACE CONCRETE PAVEMENT.				
4)	AFTER ALL WORK HAS BEEN COMPLETED, INCLUDING STABILIZATION OF THE SITE, (	COMPLETION OF	. ا	REMARKS: <i>DISPOSAL</i>	ARFAS.
	A STAND OF GRASS AND AS APPROVED BY THE OWNER, THE CITY SHALL DIRECT	THAT ALL	`   '	WILL MINIMIZE	AND CO
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STORM WATE	ER MANAGEMENT: MANAGEMENT OF THE STORM WATER RUNOFF WILL BE PROVIDED		.		
<i>COI</i>	<u>NSTRUCTION OF TRAPEZOIDAL CHANNELS, INLETS, AND THE EXTENSION OF THE EXIS</u>	TING	. Th	is record drawing is a compila ving for this project; modified .	tion of the by addenda
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			no	nt associated with the design e pracy or completeness. This original	ngineer car
EARTHWORK:	CUT: 5,200 CUBIC YARDS FILL: 20 CUBIC YARDS			at the offices of Birkhoff, I	
			.	BY J.W.B. DATE	05/
SIDE SLOPE	S: SEE ROADWAY CROSS SECTIONS (100 FOOT INTERVALS) GENERALLY FLAT				
_	SEE CHANNEL CROSS SECTIONS 3:1 SIDE SLOPES			THESE DOCUMENTS AF	RE FOR
				BIDDING, CONSTRUC AND PERMIT PURPO	TION,

### THER EROSION AND SEDIMENT CONTROLS.

	NANCE: ALL EROSION AND SEDIMENT CONTROLS WILL BE MAINTAINED IN GOOD WORKING ORDER.
	IF A REPAIR IS NECESSARY, IT WILL BE DONE AT THE EARLIEST DATE POSSIBLE, BUT NO LATER
	THAN 7 CALENDAR DAYS AFTER THE SURROUNDING EXPOSED GROUND HAS DRIED SUFFICIENTLY
	TO PREVENT FURTHER DAMAGE FROM HEAVY EQUIPMENT. THE AREAS ADJACENT TO CREEKS
	AND DRAINAGE WAYS SHALL HAVE PRIORITY FOLLOWED BY DEVICES PROTECTING STORM SEWER INLET
NCDE	TION: AN INSPECTION WILL BE PERFORMED BY A CONTRACTOR APPROVED INSPECTOR IN ACCORDANG
NSPE!	WITH THE MOST RECENT NPDES REQUIREMENTS. AN INSPECTION AND MAINTENANCE REPORT WILL BE MADE PER EACH INSPECTION. BASED ON THE INSPECTION RESULTS, THE CONTROLS
	SHALL BE REVISED PER THE INSPECTION REPORT. CITY SHALL REVIEW ALL CONTRACTOR INSPECTION
	CITY INSPECTOR TO INSPECT EROSION CONTROL DEVICES EVERY 14 CALENDAR DAYS.
VASTE	MATERIALS: ALL WASTE MATERIALS WILL BE COLLECTED AND STORED IN A SECURELY LIDDED METAL DUMPSTER. THE DUMPSTER WILL MEET ALL STATE AND LOCAL CITY SOLID WASTE MANAGEMENT
	REGULATIONS. ALL TRASH AND CONSTRUCTION DEBRIS FROM THE SITE WILL BE DEPOSITED IN
	THE DUMPSTER. THE DUMPSTER WILL BE EMPTIED AS NECESSARY OR AS REQUIRED BY
	LOCAL REGULATION, AND THE TRASH WILL BE HAULED TO A LOCAL LANDFILL.
	NO CONSTRUCTION WASTE MATERIAL WILL BE BURIED ON SITE.
IAZAR	DOUS WASTE (INCLUDING SPILL REPORTING): AT A MINIMUM, ANY PRODUCTS IN THE FOLLOWING CATEGORIES ARE CONSIDERED TO BE HAZARDOUS: PAINTS, ACIDS FOR CLEANING MASONRY
	CATEGORIES ARE CONSIDERED TO BE HAZARDOUS: PAINTS, ACIDS FOR CLEANING MASONRY
	SURFACES, CLEANING SOLVENTS, ASPHALT PRODUCTS, CHEMICAL ADDITIVES FOR SOIL STABILIZATION,
	OR CONCRETE CURING COMPOUNDS AND ADDITIVES. IN THE EVENT OF A SPILL WHICH MAY BE
	HAZARDOUS, THE CONTRACTOR SPILL COORDINATOR SHOULD BE CONTACTED IMMEDIATELY.
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46 OF 68 SHEETS

BY J.W.B. DATE <u>05/04/2010</u>

John Bullett

DATE: /0/3//06

TOWN OF ADDISON, TEXAS

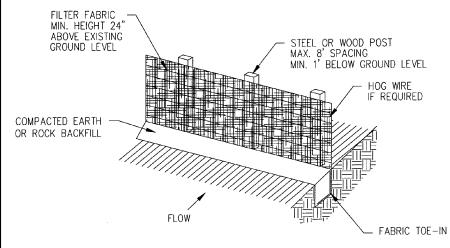
ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I STORM WATER POLLUTION PREVENTION PLAN DETAILS

BIRKHOFF, HENDRICKS & CONWAY L. L. P.
CONSULTING ENGINEERS
Dallas, Texas

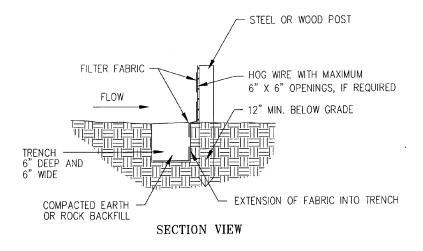
DATE: SEPTEMBER 2006

PROJECT: 2002 102

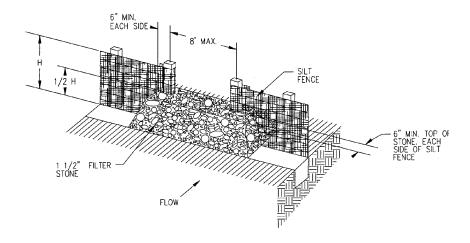




### ISOMETRIC VIEW



# SILT FENCE



STONE OVERFLOW STRUCTURE

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BY J.W.B. DATE 05/04/2010

#### SILT FENCE 5.5

- **DESCRIPTION** A temporary sediment barrier consisting of filter fabric stretched between and attached to metal or wooden posts, with the bottom of the fabric firmly embedded in the soil. At installations draining larger areas the filter fabric will be attached to a hog wire support that is attached to the fence posts.
- PURPOSE To slow the flow of sediment laden water from small disturbed areas to allow sedimentation to occur and to filter out larger sediment particles as the water flows through the filter fabric.
- APPLICATIONS Silt fence is normally used as a perimeter control immediately downstream of small disturbed areas. It can also be used as a flow diversion for very small drainage areas, but does not function as well as a normal diversion channel and is usually much more expensive.
- LIMITATIONS Do not install silt fences across channels, ditches, streams, pipe outlets, or areas of concentrated water flow. Silt fence locations can limit construction vehicle access so the locations should be well planned to prevent obstructions. Water will pond behind the silt fence resulting in localized flooding during major rain events.
- **DESIGN CRITERIA** Place silt fence along perimeter of site where disturbed area sheet runoff must be controlled. Limit the drainage area to 0.25 acres per 100 lineal feet of fence. Provide hog wire support backing whenever the drainage area exceeds 0.10 acres per 100 lineal feet of fence. Maximum post spacing shall not exceed 8 feet. Stone overflow structures or other outlet device shall be installed at all low points along the fence or every 300 feet if there is no apparent low point.

#### **MATERIAL SPECIFICATIONS -**

THESE DOCUMENTS ARE FOR

BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.

DATE: 10/31/06

- Filter Fabric NCTCOG Specification 2.23.4.
- Washed Stone NCTCOG Specification 2.1.8.(e).
- Hog Wire NCTCOG Specification 2.8.2.(b)(1). C.
- Fence Posts NCTCOG Specification 2.8.2.(b) for wood or steel.
- MAINTENANCE REQUIREMENTS Silt fence should be inspected weekly and after major rain events to ensure that the device is functioning properly. Remove sediment from behind fence when the depth of sediment has built up to one-third the height of the fence above grade. Inspect the base of the fence to ensure that no gaps have developed and re-trench as necessary. Inspect fence posts to ensure that they are properly supporting the fence. Straighten, reset and add posts if necessary. If filter fabric is ripped, damaged or deteriorated, replace it in accordance with the original specifications and details.

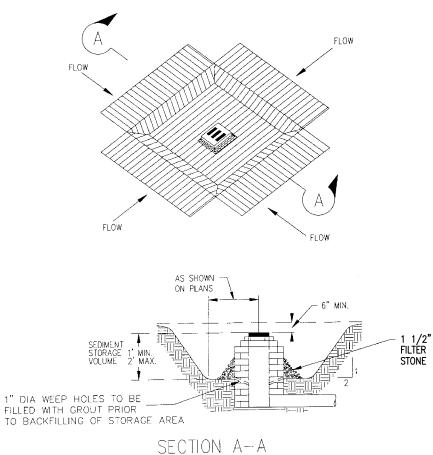
## TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I STORM WATER POLLUTION PREVENTION PLAN DETAILS

BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

JOHN W. BIRKHOF 54137

PROJECT: 2002 102 DATE: SEPTEMBER 2006



# EXCAVATED DROP INLET PROTECTION

#### 5.7 EXCAVATED DROP INLET PROTECTION

- 1. **DESCRIPTION** An excavated area surrounding a storm drain drop inlet.
- PURPOSE To remove sediment from storm runoff before it enters into the storm drain system.
- 3. APPLICATIONS Where storm drain drop inlets are to be used prior to final stabilization of the area draining to the structure. This method is used where the inlet will collect relatively heavy flows and overflow capability is needed. Weep holes or other methods of de-watering the storage area must be provided. This method can also be used in conjunction with other drop inlet protection methods (block and gravel or silt fence barriers) to provide more efficient sediment removal.
- 4. LIMITATIONS Ponding will occur around the inlet with possible localized flooding as the result. Final stabilization and cleanout may be difficult if the finished area around the drop inlet is to be paved. This method is not applicable to use around existing inlets in a paved area.
- 5. DESIGN CRITERIA -
  - A. Drainage Area 5.0 acres or less.
  - B. Depth 1 foot minimum, 2 feet maximum, measured from crest of inlet.
  - C. Volume 1800 cubic feet per acre of drainage area.
  - D. Side Slopes 2H:1V or flatter.
  - E. De-watering Time 48 hours or less. Size and number of holes as needed.

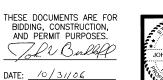
### 6. MATERIAL SPECIFICATIONS -

- A. Filter Stone NCTCOG Specification 2.1.8.(e)
- MAINTENANCE REQUIREMENTS Excavated drop inlet protection should be inspected weekly and after major rain events to ensure that the device is functioning properly. Remove sediment from the excavation when the depth of sediment has built up to one-half of the design depth. If de-watering of storage volume is not occurring, clean or replace filter stone surrounding weep holes. Clean the stone surface the first few times by raking. Repeated sediment build-up will require filter stone replacement.

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BY <u>J.W.B.</u> DATE <u>05/04/2010</u>





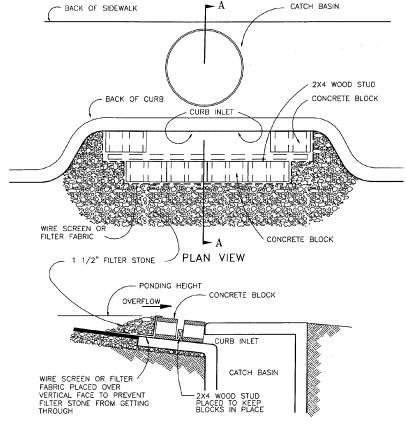
# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
STORM WATER POLLUTION PREVENTION PLAN DETAILS

BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

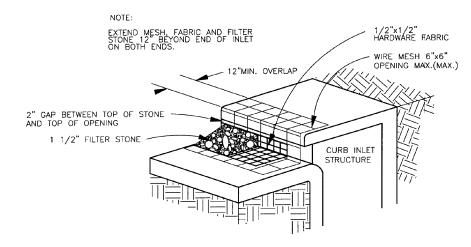
BY: J.W.B. PROJECT: 2002 102 SHEET NO. 48

C: TJH DATE: SEPTEMBER 2006 OF 68 SHEET

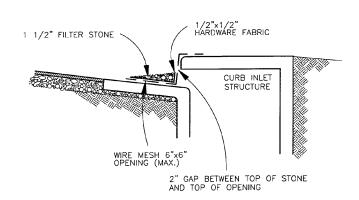


SECTION A - A

## BLOCK AND GRAVEL RECESSED CURB INLET PROTECTION



ISOMETRIC PLAN VIEW



INLET SECTION

# TYPE A CURB INLET PROTECTION

This record drawing is a compilation of the sealed engineering drawing for this project; modified by addenda, change orders and information furnished by the contractor. The information shown on the record drawings that was provided by the contractor or other not associated with the design engineer cannot be verified for accuracy or completeness. This original sealed drawings are on file at the offices of Birkhoff, Hendricks & Carter, L.L.P.

#### 5.10 CURB INLET PROTECTION

- **DESCRIPTION** A temporary sediment control barrier made of filter stone and standard concrete block, welded wire fabric, hardware fabric or 2x4 studs surrounding a storm drain
- PURPOSE To remove sediment from storm runoff before it enters into the storm drain 2. system.
- APPLICATIONS Where storm drain curb inlets are to be used prior to final stabilization of the area draining to the structure. This method is used where the inlet will collect relatively heavy flows and overflow capability is needed. This method is also to be used to protect existing curb inlets located in paved areas.
- LIMITATIONS Ponding will occur around the inlet with possible localized flooding as the result. When used at locations that are open to vehicle traffic, this device will encroach into the traveled way. If the curb inlet is not a recessed type inlet a traffic barricade shall be placed at each end of the inlet protection device.

#### DESIGN CRITERIA -

- Drainage Area 2.0 acres or less.
- Height 6" (Maximum).

#### **MATERIAL SPECIFICATIONS -**

THESE DOCUMENTS ARE FOR

DATE: 10/31/06

- Concrete Block ASTM C 139, Concrete Masonry Unit for Construction.
- Wire Fabric Standard galvanized hardware fabric with ½" by ½" openings.
- Filter Stone NCTCOG Specification 2.1.8.(e).
- Wire Mesh Welded wire fabric conforming to NCTCOG Specification 2.2.7 maximum opening 6"x6".
- MAINTENANCE REQUIREMENTS Curb inlet protection should be inspected weekly and after major rain events to ensure that the device is functioning properly. Remove sediment from the storage area when the depth of sediment has built up to one-half of the storage depth. If de-watering of the storage volume is not occurring, clean or replace the filter stone. Clean the filter stone surface the first few times by raking. Repeated sediment build-up will require filter stone replacement.

# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE STORM WATER POLLUTION PREVENTION PLAN DETAILS

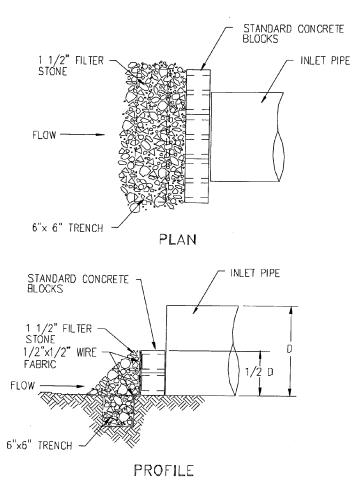
BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

JOHN W. BIRKHOF 54137 DESIGNED BY: J.W.B.

BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. Tola Bullet

PROJECT: 2002 102 DATE: SEPTEMBER 2006

BY J.W.B. DATE 05/04/2010



# CINDER BLOCK PIPE INLET PROTECTION

#### 5.11 PIPE INLET PROTECTION

- **DESCRIPTION** A temporary sediment control barrier made of standard concrete block and filter stone or stone rip-rap and filter stone surrounding the inlet end of a storm drain pipe or inlet pipe headwall.
- PURPOSE To remove sediment from storm runoff before it enters into the storm drain
- APPLICATIONS Where existing or proposed storm drain pipes or culverts are to be used prior to final stabilization of the area draining to the pipe inlet. This method is used where the pipe inlet will collect relatively heavy stormwater flows and overflow capability is
- LIMITATIONS Ponding will occur around the pipe inlet with possible localized flooding as the result. Excavation of a sediment storage area can make final channel stabilization difficult and may create a separate erosion problem if not properly constructed. Do not use Cinder Block Pipe Inlet Protection for pipes larger than 36" in diameter.

#### **DESIGN CRITERIA -**

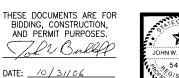
- Volume 1800 cubic feet per acre of drainage area.
- Side Slopes 1.5H:1V or flatter.
- Top of Stone and Sediment Storage ½ of inlet pipe diameter.

#### **MATERIAL SPECIFICATIONS -**

- Concrete Block ASTM C 139, Concrete Masonry Unit for Construction.
- Wire Fabric Standard galvanized hardware fabric with ½" by ½" openings.
- Filter Stone NCTCOG Specification 2.1.8.(e).
- Stone Rip-Rap NCTCOG Specification 2.1.6.(b)(2), Type A.
- MAINTENANCE REQUIREMENTS Pipe inlet protection should be inspected weekly and after major rain events to ensure that the device is functioning properly. Remove sediment from the sediment storage area when the depth of sediment has built up to one-half of the design depth. If de-watering of the storage volume is not occurring, clean or replace the filter stone surrounding the pipe inlet. Clean the stone surface the first few times by raking. Repeated sediment build-up will require filter stone replacement.

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BY J.W.B. DATE 05/04/2010





# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE STORM WATER POLLUTION PREVENTION PLAN DETAILS

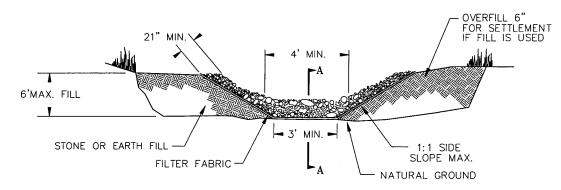
BIRKHOFF, HENDRICKS & CONWAY L. L. P.

CONSULTING ENGINEERS

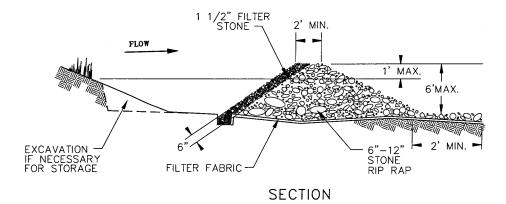
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SEPTEMBER 2006

DESIGNED BY: J.W.B. PROJECT: 2002 102 DATE:



### VIEW LOOKING UPSTREAM



# STONE OUTLET SEDIMENT TRAP

#### 5.13 STONE OUTLET SEDIMENT TRAP

- 1. **DESCRIPTION** A ponding area formed by placing an earth and/or stone embankment across a drainageway or swale. The ponding area may be natural or improved to provide the required storage volume.
- 2. PURPOSE To detain sediment laden runoff long enough to allow the majority of the sediment to settle from the water and to allow diffused runoff from the outlet.
- 3. APPLICATIONS Normally used where the natural topography allows for the construction of the embankment to form the ponding area. Diversions, drainage improvements, and localized grading will allow placement in almost any location that has adequate space for the storage area and will accept the runoff from the disturbed site. The stone outlet sediment trap can be used instead of the standard sediment basin.
- 4. LIMITATIONS Do not place device in a live stream. Avoid placing in areas planned for future improvements such as pavement or buildings. Inlet channels or pipe should be located as far away from the stone outlet as is practicable to allow for maximum sediment settling time in the basin.

#### 5. DESIGN CRITERIA -

- A. Drainage Area 30 acres, maximum.
- B. Storage Volume 1800 cubic feet per acre of disturbed land draining to the device.
- C. Surface Area of Storage Area 1% (0.01) of the area draining to the device.
- D. Embankment Height 6 feet (maximum) to top.
- E. Embankment Slopes 1.5H:1V or flatter.
- F. Embankment Top Width 2 feet (minimum)
- G. Stone Outlet Width 4 feet (minimum)
- H. Outlet Capacity 10-year design storm, C = 0.50, Max. Tc = 15 minutes.
- I. Freeboard @ Design Flow 6 inches.

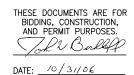
#### 6. MATERIAL SPECIFICATIONS -

- A. Stone Rip-Rap NCTCOG Specification 2.1.6.(b)(2), Type A or re-cycled concrete meeting the same gradation.
- B. Filter Stone NCTCOG Specification 2.1.8.(e).
- C. Filter Fabric NCTCOG Specification 2.23.4.
- MAINTENANCE REQUIREMENTS Stone outlet sediment traps should be inspected weekly and after major rain events to ensure that the device is functioning properly. Remove sediment from the storage area when the depth of sediment has built up to one-half the height of the stone outlet. Inspect the downstream base of the stone outlet and the downstream flow channel to ensure that no excessive erosion or gullies have developed and repair as necessary. The sediment storage area should drain within 48 to 72 hours after a rain event. The filter stone on the upstream face of the stone outlet may require cleaning or replacement if standing water remains for longer periods.

These plans and related specifications were prepared for construction of this specific project only. Reuse of these documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.

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BY <u>J.W.B.</u> DATE <u>05/04/2010</u>

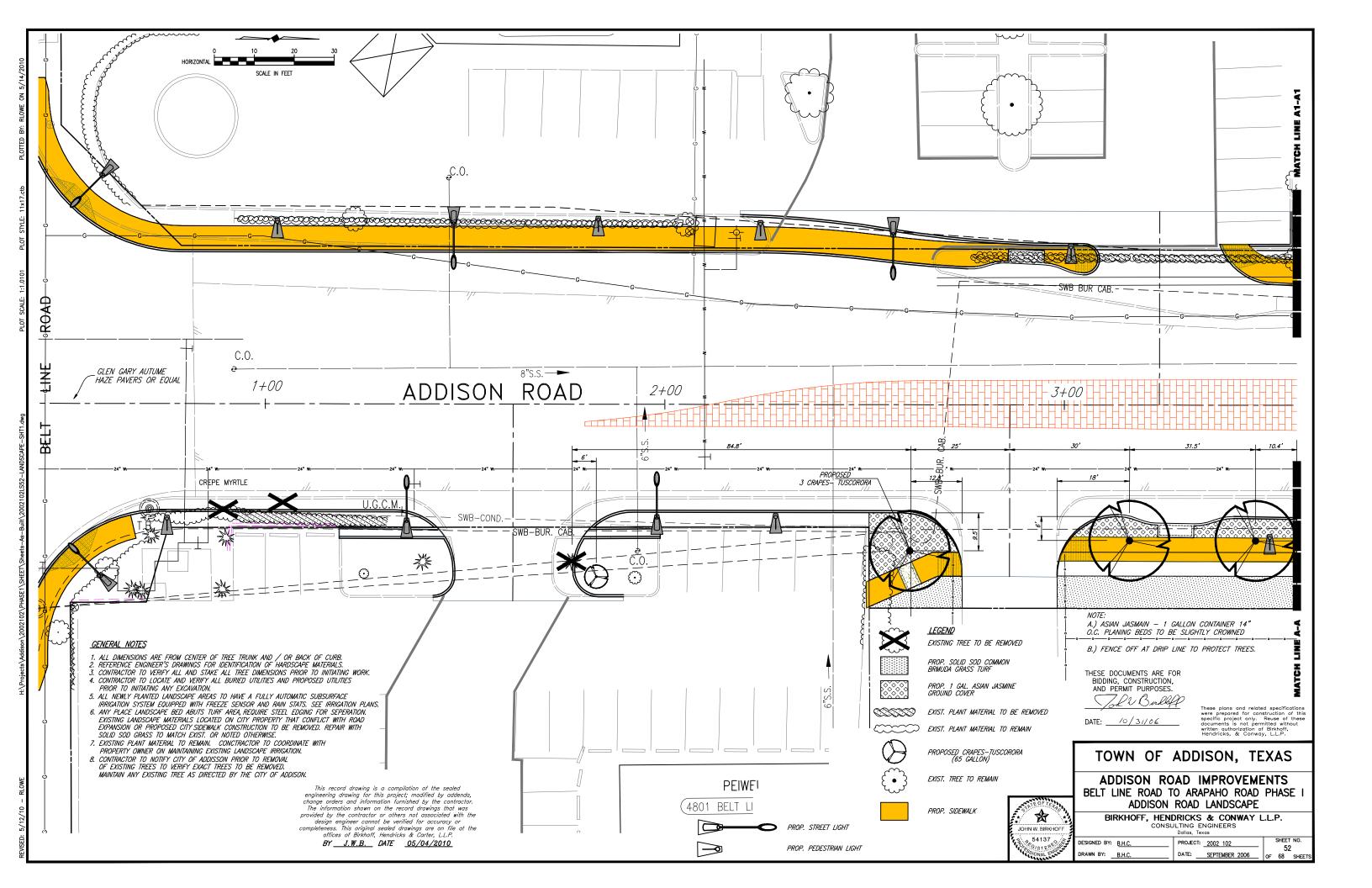


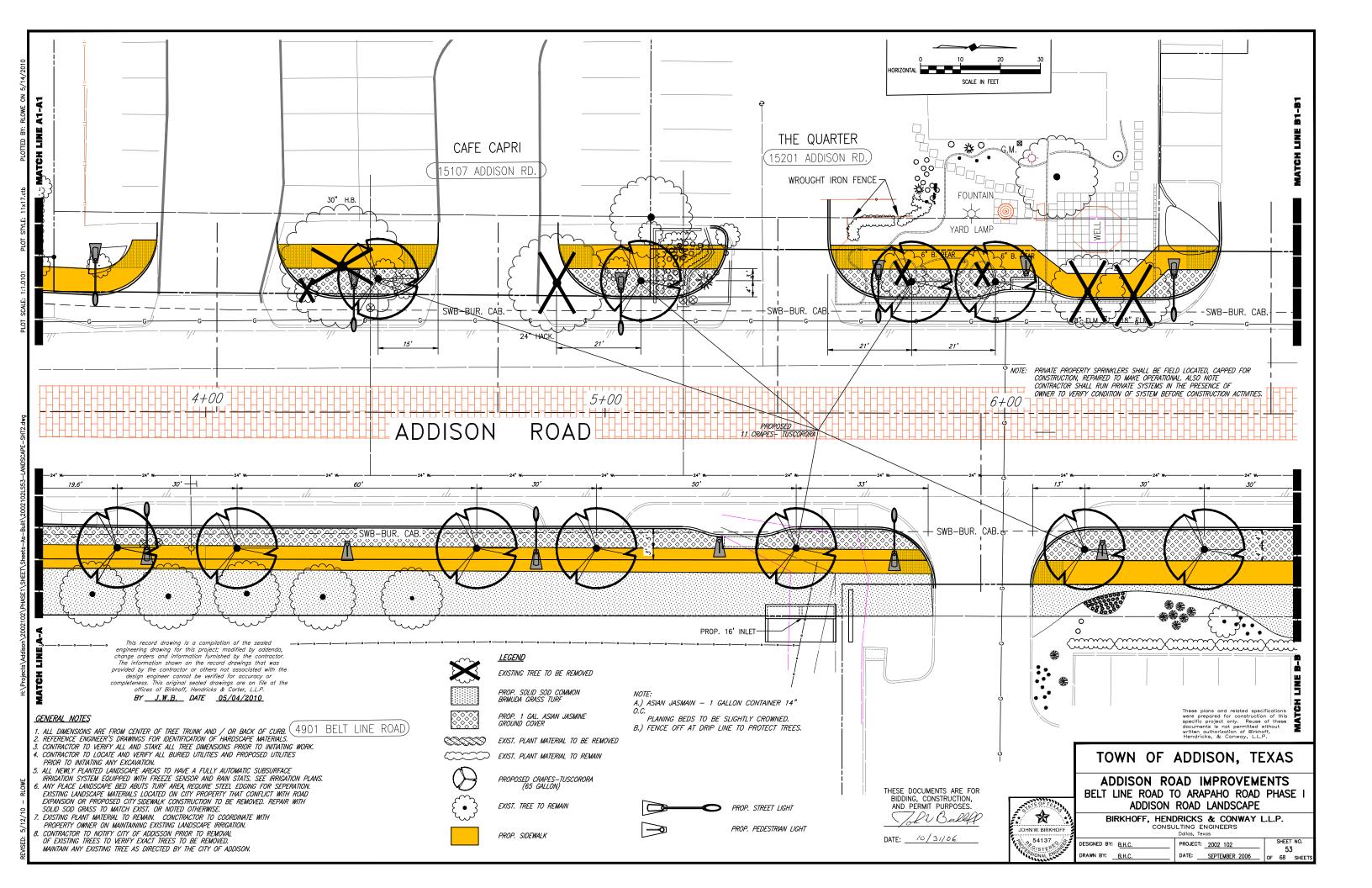
# JOHN W. BIRKHOFF 54137 9/815

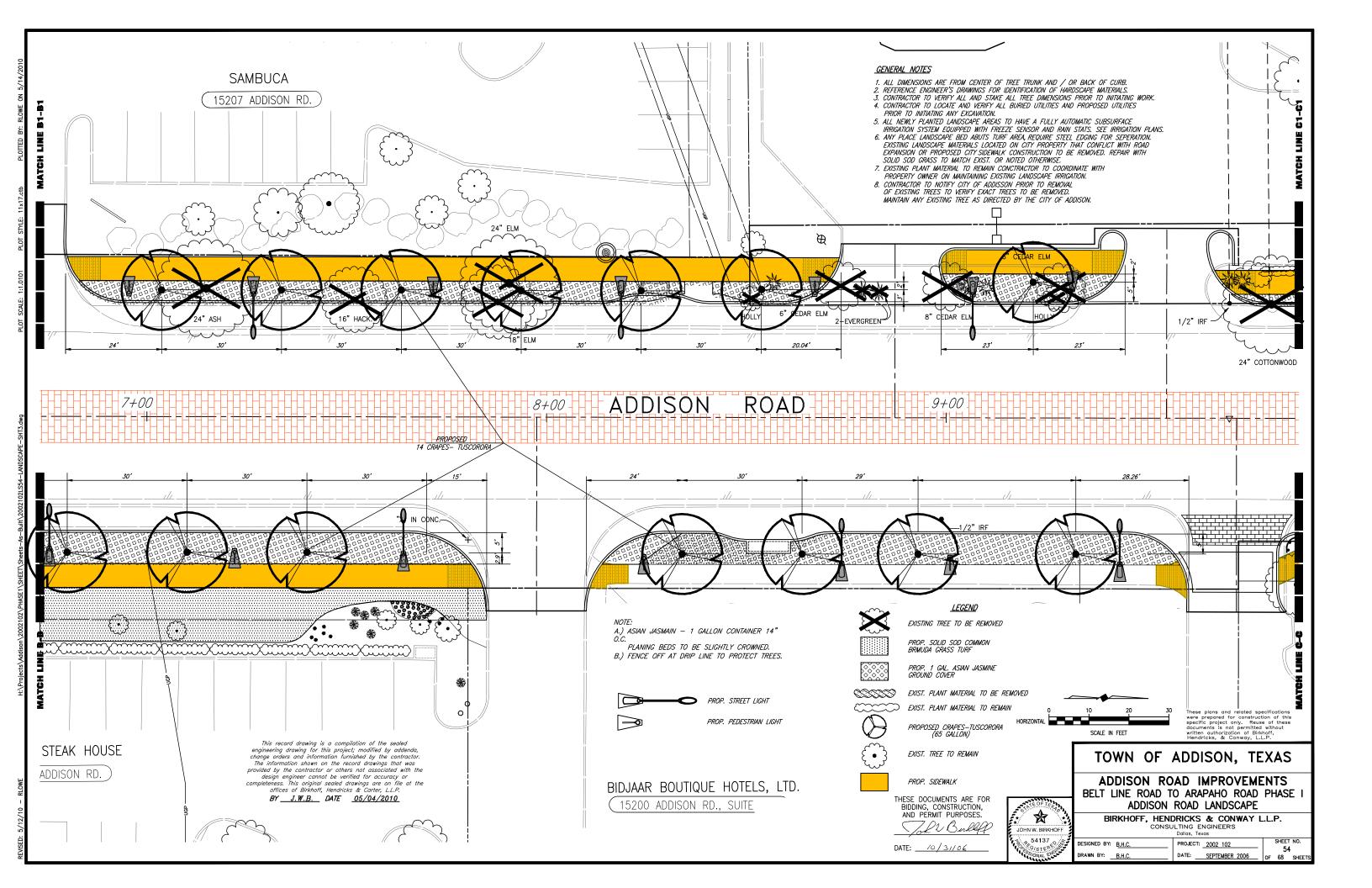
# TOWN OF ADDISON, TEXAS

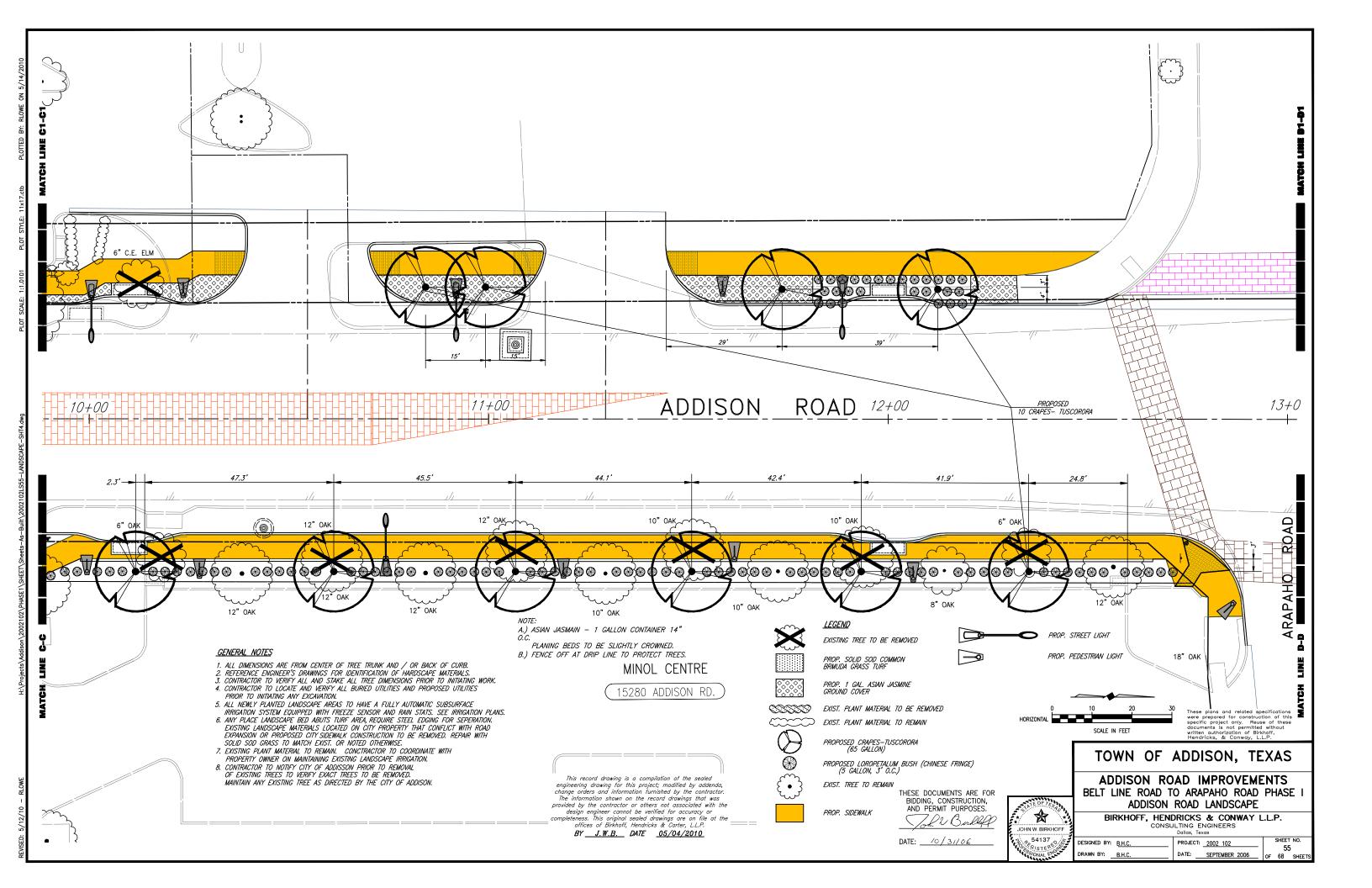
ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
STORM WATER POLLUTION PREVENTION PLAN DETAILS

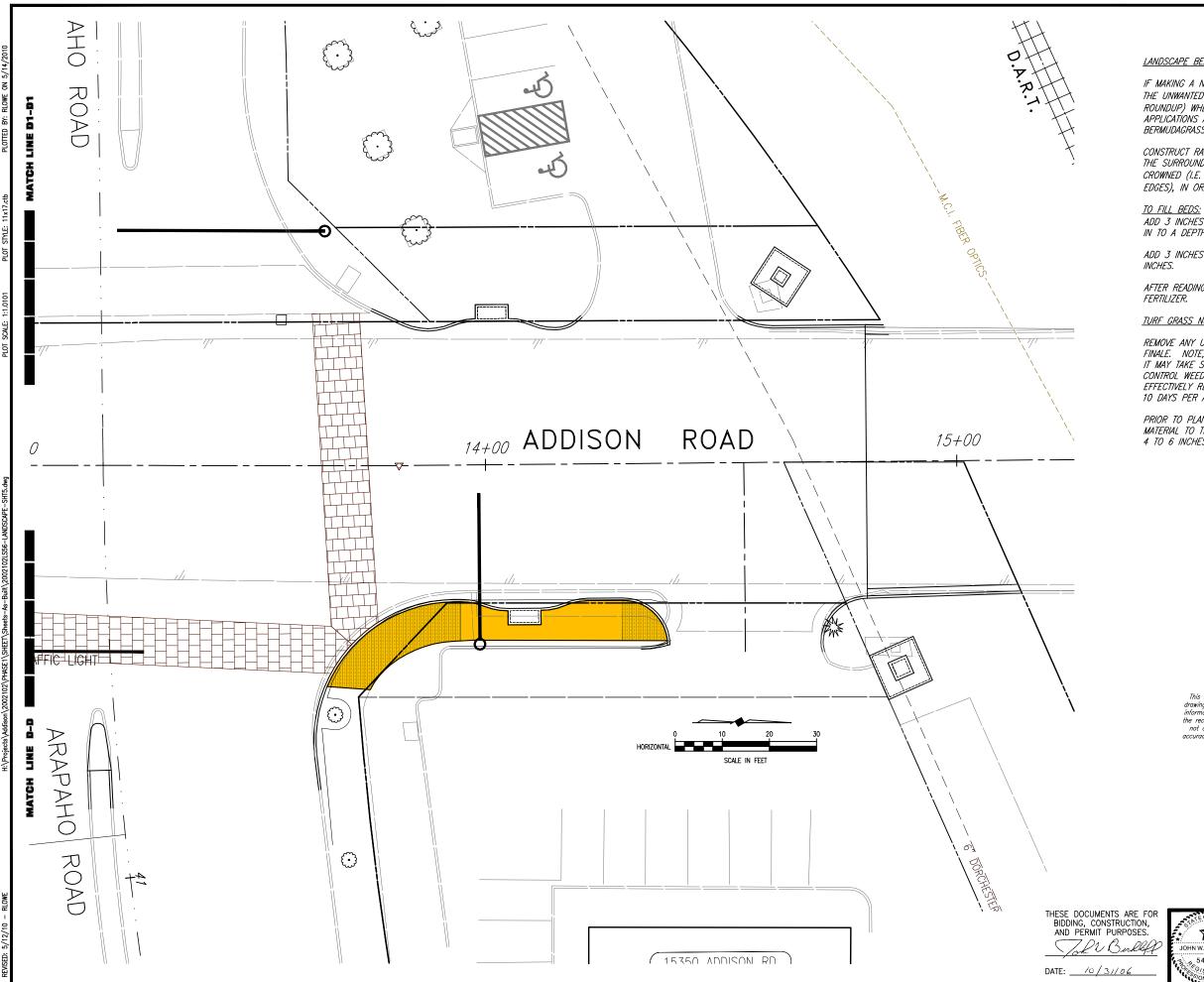
BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS











#### LANDSCAPE BEDS NOTES

IF MAKING A NEW BED IN AN AREA COVERED WITH GRASS AND WEEDS, KILL THE UNWANTED PLANTS BY SPRAYING WITH A GLYPHOSATE PRODUCT (E.G. ROUNDUP) WHEN THE GRASS AND WEEDS ARE ACTIVELY GROWING. TWO APPLICATIONS ARE USUALLY REQUIRED TO KILL DIFFICULT WEEDS SUCH AS BERMUDAGRASS. READ AND FOLLOW ALL LABEL DIRECTIONS.

CONSTRUCT RAISED BEDS, THE CENTERS OF WHICH ARE 3 INCHES ABOVE THE SURROUNDING SOIL. DURING FINAL SHAPING, THE BED SHOULD BE CROWNED (I.E. MADE 3 INCHES HIGHER IN THE CENTER THAN AT THE EDGES), IN ORDER TO HELP DRAIN AWAY EXCESS WATER.

ADD 3 INCHES OF EXPANDED SHALE (E.G. TRUGRO SOIL CONDITIONER), TILL IN TO A DEPTH OF 8-10 INCHES.

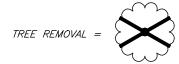
ADD 3 INCHES OF FINISHED COMPOST, TILL IN TO A DEPTH OF 8-10

AFTER READING AND FOLLOWING ALL LABEL DIRECTIONS, ADD APPROPRIATE

#### TURF GRASS NOTES

REMOVE ANY UNWANTED VEGETATION WITH PRODUCTS SUCH AS ROUNDUP OR FINALE. NOTE, FOR DIFFICULT TO CONTROL WEEDS SUCH AS BERMUDAGRASS IT MAY TAKE SEVERAL APPLICATIONS TO EFFECTIVELY REMOVE THESE HARD TO CONTROL WEEDS. KEY IS TO ALLOW ENOUGH TIME PRIOR TO PLANTING TO EFFECTIVELY REMOVE THE WEEDS. FOR ROUNDUP AND FINALE, ALLOW 7 TO 10 DAYS PER APPLICATION OF THE HERBICIDE.

PRIOR TO PLANTING THE NEW LAWN, APPLY A TWO INCH LAYER OF ORGANIC MATERIAL TO THE SOIL SURFACE AND THEN TILL THE MATERIAL INTO THE TOP 4 TO 6 INCHES OF TOPSOIL.



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BY J.W.B. DATE 05/04/2010

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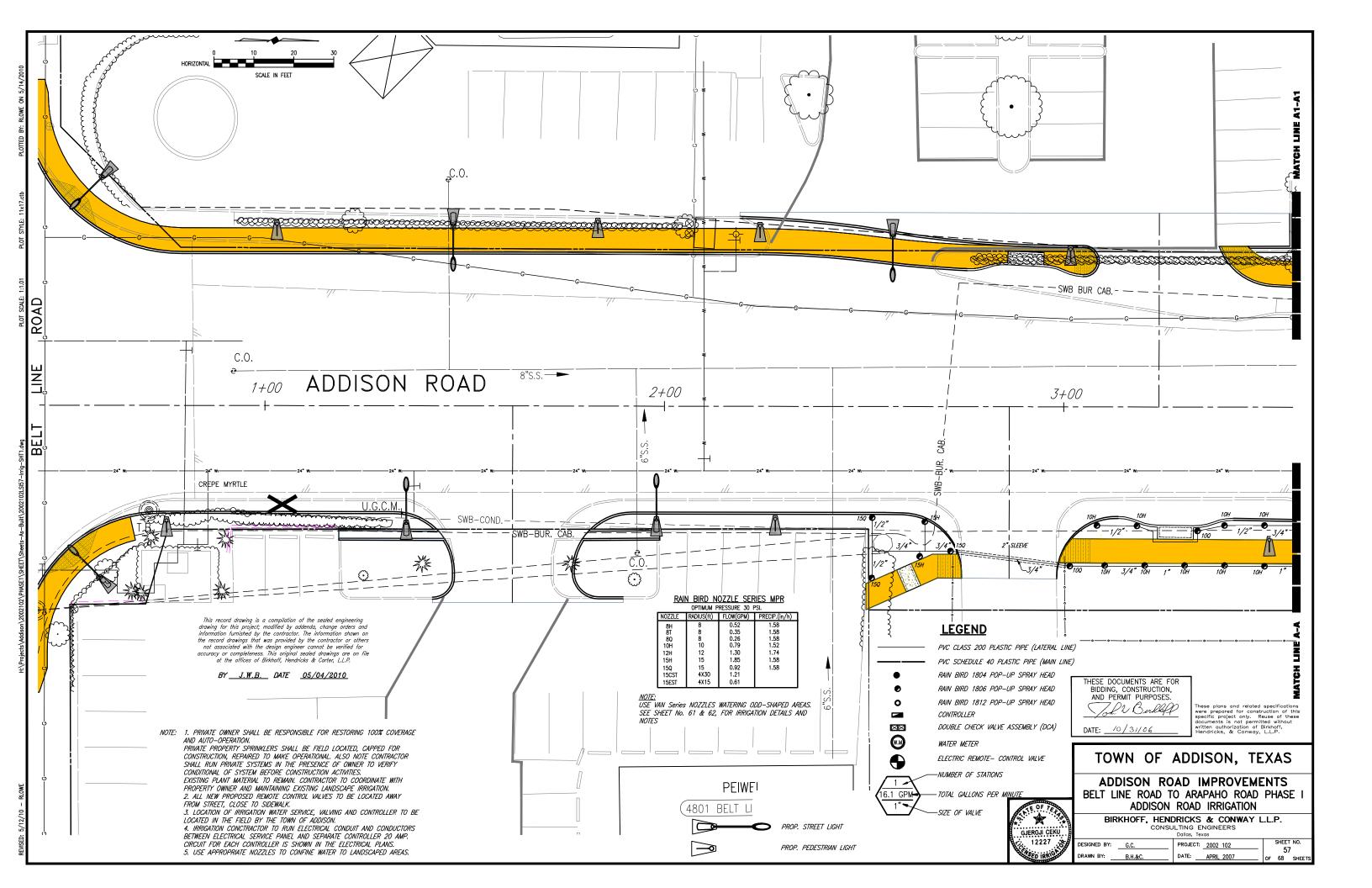
## TOWN OF ADDISON, TEXAS

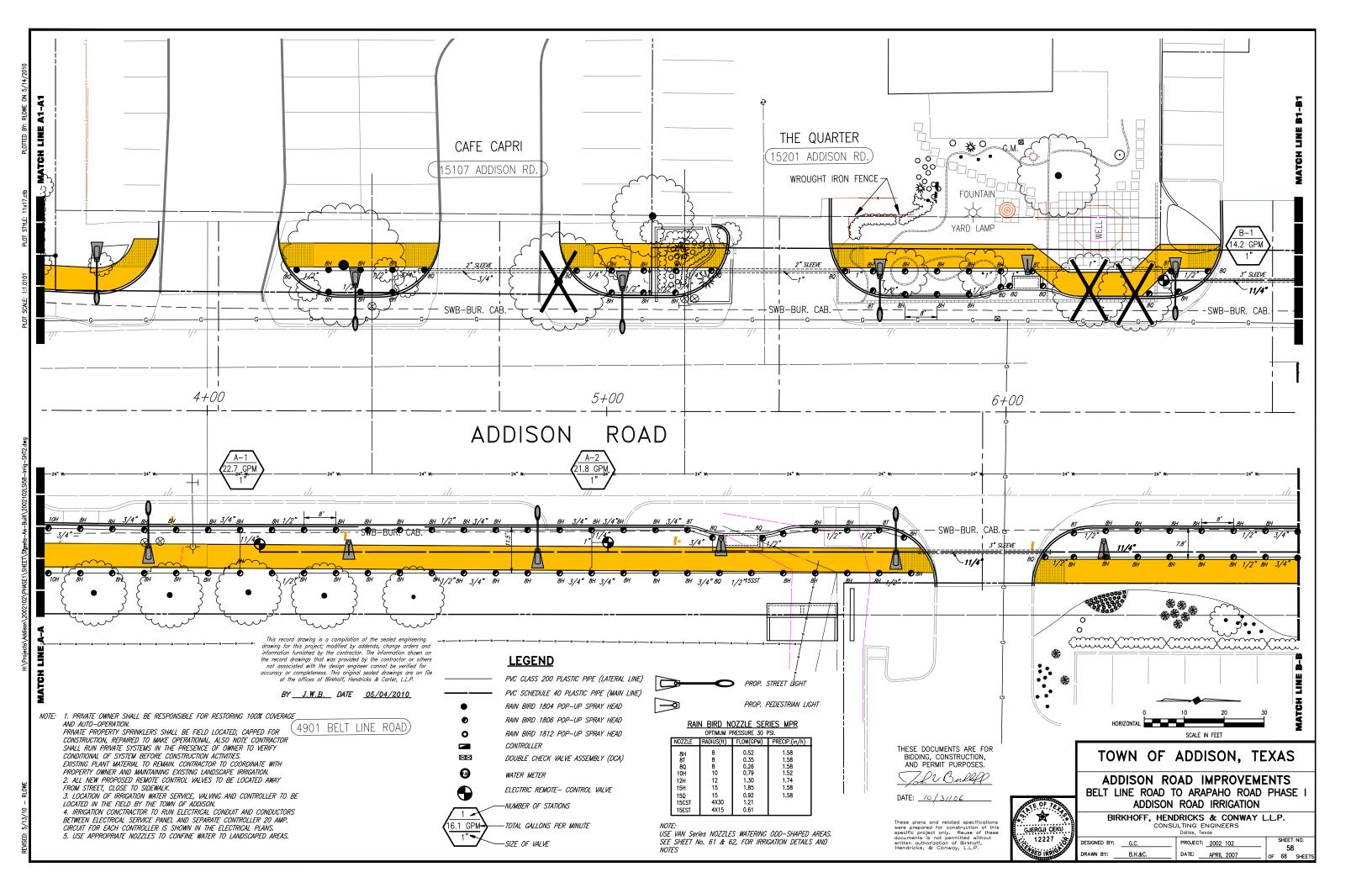
### ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I ADDISON ROAD LANDSCAPE

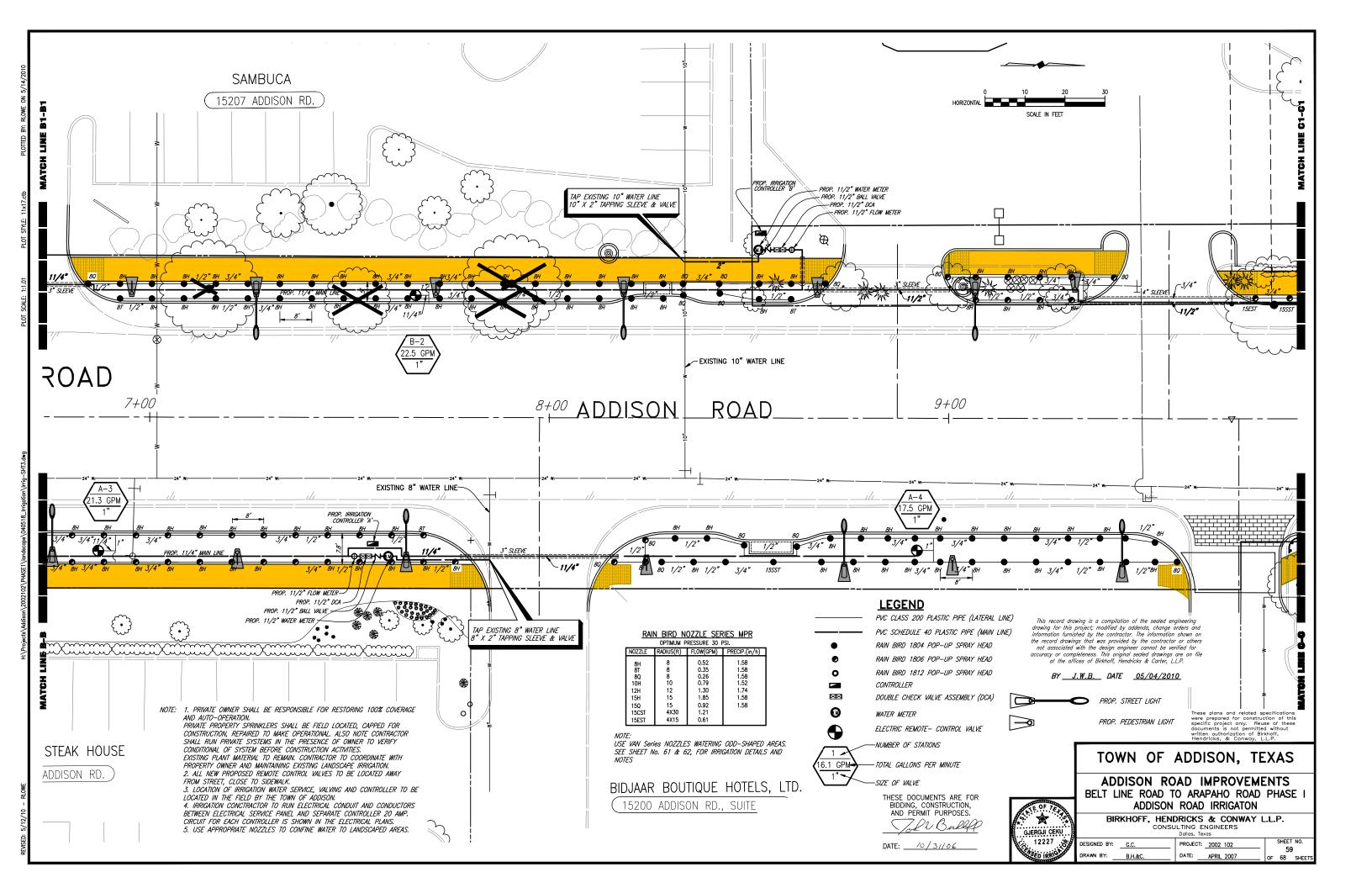
BIRKHOFF, HENDRICKS & CONWAY L.L.P. CONSULTING ENGINEERS

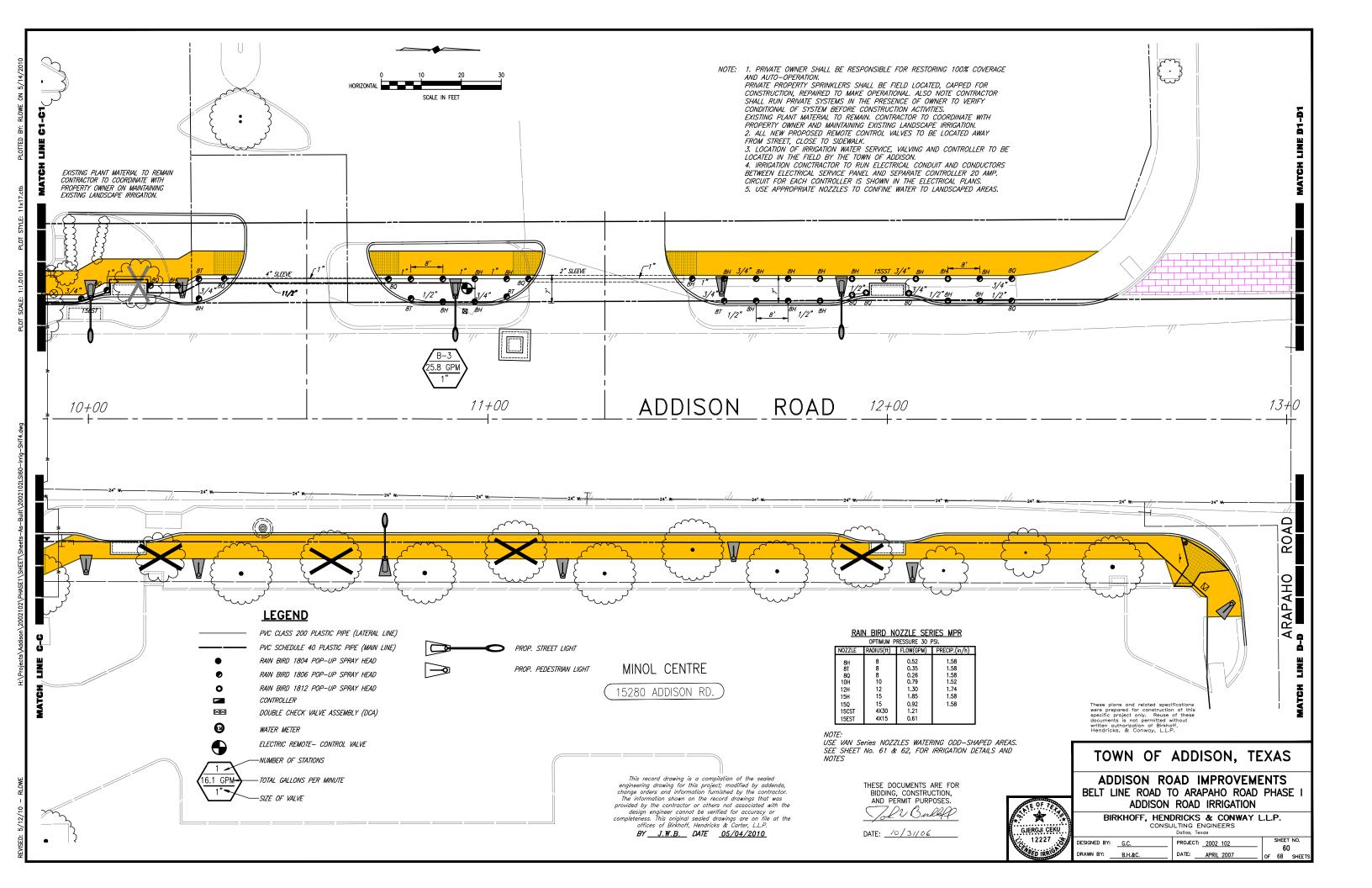
PROJECT: 2002 102 56 DATE: SEPTEMBER 2006

\* JOHN W. BIRKHOFF

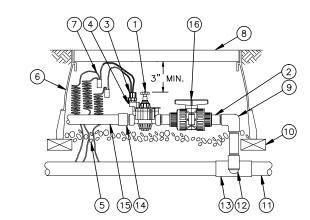








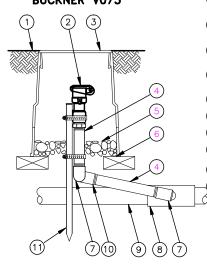
## REMOTE CONTROL VALVE **WEATHERMATIC 11000CR SERIES**



**INSTALLATION DETAIL** NO SCALE

- Weathermatic 11000 FCR SERIES REMOTE CONTROL VALVE
- (2) INSTALL THREADED MALE ADAPTER WITH TELFON TAPE/TYP.
- (3) SOLENOID
- (4) MANUAL BLEED LEVER
- 5 4.0-INCH MINIMUM DEPTH OF 3/4-INCH WASHED GRAVEL
- (6) AMETEK RECTANGULAR PLASTIC BOX
- (7) EXPANSION COIL & WATERPROOF CONNECTOR
- (8) FINISH GRADE/TOP 1/2" ABOVE TOP OF MULCH
- (9) PVC SCH. 40 ELBOW
- (10) BRICK (1 OF 4)
- (11) SCH. 40 MAINLINE PIPE
- (12) PVC SCH. 40 ELL
- (13) PVC SCH. 40 TEE OR ELL
- (14) PVC SCH. 40 MALE ADAPTER
- (15) PVC LATERAL PIPE
- (16) PVC BALL VALVE

## QUICK COUPLER VALVE **BUCKNER V075**



**INSTALLATION DETAIL** 

- 1) FINISH GRADE/TOP 1/2" ABOVE TOP OF MULCH
- QUICK-COUPLING VALVE:
- 3 10" ROUND PLASTIC AMETEK VALVE BOX.
- 4 PVC SCH 80 NIPPLE (LENGTH AS REQUIRED)
- 5 3-INCH MINIMUM DEPTH OF 3/4-INCH WASHED GRAVEL
- (6) BRICK (1 OF 2)
- (7) PVC SCH 40 STREET ELL
- (8) PVC SCH 40 TEE OR ELL
- (9) PVC MAINLINE PIPE
- (10) PVC SCH 40 ELL
- 1/2"Dia X 18" STEEL REBAR WITH STAINLESS STEEL WORM SCREW CLAMP.

## **EVOLUTION DX2 STAINLEES STEEL** PEDESTAL CABINET INSTALLATION

1) FINISH GRADE/TOP 1/2" ABOVE

2 POP-UP SPRAY SPRINKLER: RAIN BIRD 1800

TOP OF MULCH

(3) PVC SCH 40 FITTING

(4) PVC SCH 40 ELL

5 6" GREEN NIPPLES

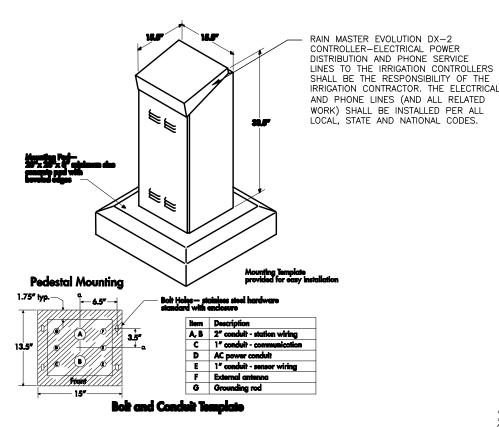
(6) PVC LATERAL PIPE

CUT TO FIT.

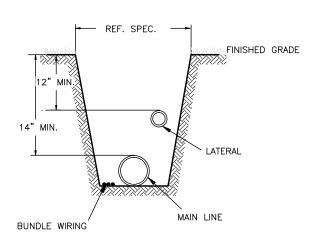
**RAIN BIRD 1800 SERIES** 

\_NXXXXAMXXXXXXXXX

**INSTALLATION DETAIL** 



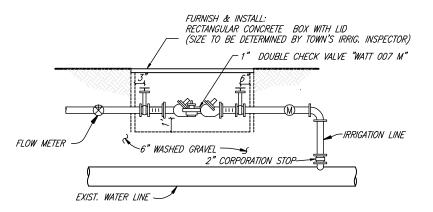
**IINSTALLATION DETAIL** 



TRENCHING DETAIL NO SCALE

BY J.W.B. DATE <u>05/04/2010</u>

#### DOUBLE CHECK VALVE ASSEMBLY



## **INSTALLATION DETAIL**

# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE ADDISON ROAD IRRIGATION NOTES & DETAILS

BIRKHOFF, HENDRICKS & CONWAY, L.L.P.

Dallas, Texas PROJECT: 2002 102 61 B.H. & C. DATE: APRIL 2007

This record drawing is a compilation of the sealed engineering drawing for this project; modified by addenda, change orders and information furnished by the contractor. The information shown on the record drawings that was provided by the contractor or others not associated with the design engineer cannot be verified for accuracy or completeness. This original sealed drawings are on file at the offices of Birkhoff, Hendricks & Carter, L.L.P.

THESE DOCUMENTS ARE FOR

AND PERMIT PURPOSES.

#### **Town of Addison Irrigation Specifications**

#### Revised 04/22/04

These revised specifications supersede any and all others. However, any discrepancies between the approved construction plans and those of the Town <u>MUST</u> be brought to the attention of the Town's designated representative for a final determination. The contractor will present the Town's representative an as-built plan at the final walk-through, along with three Bucker VO75 quick coupling keys with

- ${\bf 1.}$  All work is to be accomplished by or directly supervised at all times by an on-site Irrigator licensed by the State of Texas.
- 2. The contractor shall verify the water pressure before the installation begins. If the static pressure is different than that of the design pressure, contact the designer and Town's representative immediately so changes can be made. Send a fax to the Parks dept. at 972-450-2834 with the current dated and timed static pressure reading. Design head to head with no single head coverage. Use appropriate size nozzles for a given landscape area so as not to spray onto or over payed surfaces or structures. Do not exceed manufacturer's recommendations
- 3. The irrigation installer is responsible for resetting head and/or box height due to settling and after turf, groundcover, shrubs, trees, and mulch is added to the landscape areas. The irrigation contractor must supply a workmanship warranty for (1) year from date of acceptance.
- 4. Plans are diagrammatic and field adjustments are often necessary. For this reason, prior to trenching, valve locations and head layout with flags needs to be done and approved by the Town's irrigation inspector. Not doing so may result in the relocation of heads and/or valves at the irrigation contractor's expense.
- **5. Water Taps:** Water taps will be 2" in size. All parts must conform to the Town of Addison Water Department specifications and are the responsibility of the irritation contractor to provide. Inspection of taps by the Water Department Representative must occur. Excavation and tap permits are required. Contact the Town of Addison Department at (972) 450-2871.
- 6. Water Meters: Only Master or Hersey meters with two (2) brass flanges are acceptable. Meter lay lengths must be in accordance with the Town of Addison Water Department's specifications, housed in appropriate size (to be determined by the Town's Irrigation Inspector) concrete box with full. New stainless steel botts and nuts must be used in the installation along with new neoprene gaskets. The box should be least with the final grade users accepted as were the concrete descriptors. level with the final grade using concrete pavers to support and prevent sinking. Backfill inside the box, 3" below meter base with at least 6" of fine (1/2") pea gravel. Connection to main must be approved and inspected by the Town's Water
  Department Inspector and all tap materials are to be purchased at the expense of the contractor and must comply with the Town of Addison's specifications.
- 7. Backflow Devices: Only Watts 007 M series inline check valve assemblies with the stainless steel ball valve handles and nuts are to be used. Irrigation contractor shall provide and install brass plugs for the test cocks. Connect to the flange using tefion taped copper nipple of sufficient length to center the DCA within its housing.
  - B. Rotors Only Hunter I-20 Series are permitted, unless noted otherwise. Install ¾" above finished grade.
- 14. Risers: Use Sch 80 PVC with Weathermatic LXS Series shrub head adapters with a  $\frac{1}{2}$ %6° green poly cut-off nipple screwed into the threaded fitting in the ground and a threaded coupling between the two. The irrigation inspector reserves the right to determine placement of risers versus pop-ups.
- 15. Wiring: All wires will be 14 gauge UF. Station wires will be red. Common wires will be white. Master valve wire will be blue. Anytime the wiring changes direction, such as at an elbow or a tee, allow a loop of at least 12 inches alongside the fitting at that location. Only continuous wire runs are permissible. Wire should follow the main line where possible and lay along a single side not crossing over lateral lines. Wire is to be placed under mainline with 2" of dirt between wire and pipe.
- 16. Wire Connectors: <u>Use only DBY connectors</u> for all field wire splices other than at the valves themselves. Allow at least 36 inches of pigtailed wire at each splice. <u>Use King One Step</u> tan colored connectors for all valve splices. All valve box splices are to be housed in standard (large) Ametek rectangular plastic valve boxes. All field splices are to be in 10 inch round Ametek plastic valve boxes or standard, large rectangular Ametek plastic valve boxes at the discretion of the Town's
- 17. Backfilling: Prior to any backfilling of trenches, an inspection by the Town's irrigation representative must take place and any necessary changes implemented otherwise manual excavation to enable proper inspection will be necessary. Use clean and approved topsoil to backfill all pipe to depth. All heads and boxes are to be backfilled to grade with clean topsoil. No rocks greater than 1 inch are allowed. Compact trenches to alleviate settling. Minimal depth of coverage is 12 inches.
- 18. Valve sequencing must be performed by the contractor and in an order approved by the Town Irrigation Inspector. At least 12 inches of extra station wiring within the bottom of the pedestal is necessary for each zone and must be of neat and orderly
- **19.** Any deficiencies in coverage noted by the Town's irrigation inspector will be rectified at the cost of the contractor.
- 20.Controller: A Town irrigation representative will determine the type of controlle to be used. All controllers shall have a concrete pad of 36"x36"x6". Pad will be set at 3" above final grade. Install the controller after the concrete pad is completely cured (two days). Use only appropriately sized stainless steel bolts, washers and nuts to secure the controller to the concrete pad. All wiring is to enter the ped appropriately sized PVC <u>sweep</u> elbows extending at least 1" thru and 6" out from under the pad. Control/master valve wiring, flow meter wiring and 120-V service wiring are to be separated with each having its own access elbow. An additional spare %" sweep elbow for phone service is to be installed as well. All national and local codes must be followed during the installation.
  - A. A/C controller Only Irritrol MC Plus controllers will be acceptable. Both Mini-click rain and freeze sensors will be installed and placed where they can operate properly. All non-Rain Master controllers must be permanently wired for quick attachment to a Rain Master remote control unit.

The device will be housed in an appropriate size, (to be determined by the Town's Irrigation Inspector) rectangular concrete box with lid using concrete pavers for proper stability and use extensions to adjust helpht. The irrigation contractor shall be responsible for the DCA testing in accordance with State of Texas law, using a Licensed Backflow Assembly Tester registered with the Town of Addison Water

- 8. Sleeves: All paving must have Town approved sleeve sizes and quantities present. It is the responsibility of the irrigation contractor to notify the Town's Irrigation Inspector of any area where sleeves should be present but are not and Irrigation Inspector of any area where sleeves should be present but are not and provide such materials at his cost. Any paving installed without sleeves will necessitate a bore and subsequent materials at the irrigation contractor's expense. All sleeves 2" and smaller will be Schedule 40 PVC with size and location noted on the plan. Larger sizes will be Class 200. All piping underneath paving, including sidewalks, must be sleeved. All sleeves are to be belied end PVC pipe. A minimum length of 12 inches of sleeve material must extend beyond the pavement.
- 9. Glue and Primer: Use Turftite brand glue on laterals and IPS Grey Heavy Body on main lines and a good quality purple primer on all . Avoid excessive use and wipe excess glue off of all joints and fittings with a clean rag.
- 10. Pipe: <u>All</u> main line pipe 2 inches and smaller is to be Schedule 40 belled PVC; larger sizes are to be Class 200 belled PVC with a minimum depth of 14" and a maximum depth of 16". Put not more than two (2) pipes in any one trench and separate the main line from the lateral line with at least two (2) inches of cover. Class 200 belled PVC lateral piping is to be used with a minimum depth of 12" and a
- 11. Fittings: No crosses are permitted. Separate tees, 45's, elbows and other fittings by at least 12 inches. Reduction tees are preferred over use of single reducer bushings. Multiple reducer bushings will not be accepted. Only Spears and/or Lasco fittings are permitted. Allow 18 inches outside of sleeve before the first fitting. No 45 degree elbows on 1 inch and larger pipe are allowed.

- A. Master Valves: Every point of connection to the water supply system shall have a Weathermatic 11000 FCR series valve as the Master Valve, housed in a standard (large) Ametek rectangular plastic valve box with 4 to 6 inches of small (1/2") pea gravel placed underneath the valve in such a manner as to prevent soil infiltration into the box. Use concrete pavers or bricks placed under edges of valve box for stability. Note: Valve box must not rest on pipe. Blue wire shall be used as the station wire for the Master Valve.
- B. Station Valves: Only Weathermatic 11000 FCR series valves are permitted. A Ball Valve will be installed before every station or zone valve. They are to be located within a standard (large) Ametek rectangular plastic valve boxes with 4 to 6 inches of (1/2") pea gravel placed underneath the valve in such a manner as to prevent soil infiltration into the box. The pea and a manner as to prevent soll infiltration into the box. The pea gravel should be 2 inches from the bottom of the valve body. A minimum of 3" inches of valve box must extend that. inches of valve box must extend below bottom of valve. If necessary use
- C. Ball Valves: Female threaded plastic Spears or Lasco ball valves with positive T-handle cut off must be installed on every 200 feet of mainline for
- B. Battery and/or Solar Operated Controllers Only LEIT controllers will be acceptable. Install rain or freeze sensors on these controllers with SKIT8821-4 installation kit. Install on galvanized thick wall poles and set controller panel to height above finished grade to be determined by Town's
- C. Rain Master: Only an approved size Rain Master Evolution DX-2 controller with a stainless steel pedestal and heavy duty transient protection is permitted. The controller must include all necessary hardware to ensure reliable communication and operation with the Town's central control located at 16801 Westgrove. Installation must include the following Rain Master hardware, purchased only from a Rain Master supplier: DX-03 sensor board, DX-PH phone communication option, Data Industrial flow meter (same size as the mainline), and shielded EV-CAB-SEN flow meter cable. It is the irrigation contractor's responsibility to entail the cost of and work in conjunction with South Western Bell Telephone to establish a dedicated phone service and install an interface within the pedestal at each controller location via direct burial cable within 1" PVC conduit. The entire installation must conform to burial cable within 1" PVC conduit. The entire Installation must conform to Rain Master specifications and be approved by the Town's inrigation inspector prior to and be inspected during installation. Such specifications will include grounding and pad configurations and distances of separation from water meter to DCA to master valve to flow meter and the first fitting. A functional Mini-clik freeze and rain sensor with a Hunter bypass switch must be installed in an approved location and by an approved method. For part numbers and pricing of any Rain Master equipment, contact Mark Stricker of John Deere Landscapes at 972-881-0205. For technical questions, call John DuBose of RainMaster at 214-632-289.
- 21. Communication is the key. If you are unsure, CALL Ron Lee, the Operations

isolation purposes. A ball valve is also required to be installed before every station valve. Use 10" Ametek valve box with a minimum of 3" extending below bottomof valve and fill to bottom of valve with  $\frac{1}{2}$ " pea gravel. Use bricks or concrete pavers under box for stability

- **D. Quick Coupler Valves**: Use only Buckner V075 single lug %" quick coupling valves with a metal top. They are to be connected to a threaded fitting. Teflon tape and appropriate length of gray schedule 80 nipples and schedule 40 fittings are to be used for the swing joint. Secure to 18 inch by 1/2inch steel rebar with a stainless steel worm screw clamp. House QCV in a 10 inch round plastic Ametek valve box. Install Spears ball valve prior to each QCV. Bricks or pavers need to be installed under edges of valve boxes for stability. Backfill bottom of box with ½" pea gravel half way up body of valve.
- E. Flowmeters: Purchase from a Rain Master supplier and install E. Flowmeters: Purchase from a Rain Master supplier and install appropriately sized Data Industrial flowmeter. Foliow all installation instructions as approved by Rain Master. The irrigation contractor must also purchase from Rain Master and install shielded Rain Master EV-Cab-Sen flow meter cable and install within continuous %° or larger gray PVC conduit with 6 inch or larger J-boxes placed every 200 feet or where 360 degrees of fittings are installed; only sweep fittings are permitted. Only a continuous run of cable is allowed; mg splices will be allowed except at the point of connection to the flow meter. Connections at the flow meter must first be soldered and then water proofed within a 3-M DBY connector. Note: certain Rain Master requirements must also be not required in certain them control difference of requirements must also be met regarding installation order and distances of separation between DCA, flow meter, master valve and the first fitting. It is the responsibility of the irrigation contractor to adhere to these requirements At final walk through, proper operation of the flow meter at the Rain Master controller must be demonstrated by the irrigation contractor.
- 13. Heads: All heads will be installed using polyethylene green nipples (¾"x6" for rotors and ½"x6" for pop-ups) screwed into threaded fittings unless noted otherwise. No swing joints on 4" pop-ups or rotors will be allowed.
  - A. Pop-ups Only Rainbird 1800 series are permitted. Install ¾ inch above the finished grade.
    - a. 4 inch pop-ups: turf, tree bubblers within turf areas (use Hunter bler nozzles on spray heads).
    - **b. 6 Inch pop-ups** with no side inlet: very low ground cover (less than 6 inches at mature height).
    - c.12 inch pop-ups with side inlet: Ground cover and low growing shrubs. The ground cover and shrubs should not be more than 12" at maturity. The Town Inspector reserves the right to determine if and when side inlets installed using funny pipe verses the bottom inlet will be allowed. When authorized, use Hardie Blue Line Pipe with Toro barb
    - d. Use 1/2" Sch 80 risers with schrub adapter and Hunter PCN 10 bubbler nozzles for all tree wells with tree grates. Risers shall be a minimum of 2" below bottom of tree grates with nozzle 2" above
- NOTE. -The irrigation system has been designed to operate with minimum static water pressure of 50 p.s.i. and total demand of 26 gpm. Notify Town's representative if any dicrepancy arises.
  - -Contractor shall verify nozzle sizes in field for MPR and head to head coverage. Contractor to mimimize overspray. No overspray onto pavement shall be accepted.

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BY J.W.B. DATE 05/04/2010

were prepared for construction of this specific project only. Reuse of these documents is not permitted without

# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE ADDISON ROAD IRRIGATION NOTES & DETAILS

BIRKHOFF, HENDRICKS & CONWAY, L.L.P.

CONSULTING ENGINEERS

G.C. BH & C

DESIGNED BY: DRAWN BY:

\*

GJERGJI CEKU

Dallas, Texas PROJECT: 2002 102 62 DATE: APRIL 2007 OF 68 SHEETS

THESE DOCUMENTS ARE FOR BIDDING. CONSTRUCTION. AND PERMIT PURPOSES. Tola Bullett

DATE: 10/31/06

#### MODIFICATIONS TO ELECTRICAL SHEETS

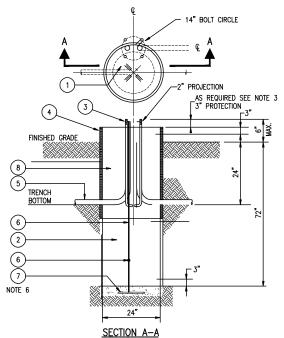
Contractor shall implement these modifications to the Electrical Sheets and include such cost in the appropriate bid items.

- A. Sheet No. 63 (Drawing E-1): Roadway Lighting Part "A"
- 1. Add an additional Utility Service/Detail 2/E-5 for a total of two (2)
  Utility Services. One service will serve Roadway Lighting exclusively
  and the other service will serve Pedestrian and Irrigation Systems.
- 2. Currently a single feeder containing 2 #10, #10G, ¾" C, is shown from the single service to each pullbox/light fixture. Provide an additional feeder of 2 #10, #10G, 2" C, to also loop through each pull box. One circuit will serve Roadway Lighting exclusively while the other feeder will serve Pedestrian and Irrigation Systems.
- 3. Change the conduit size to 2-inch.
- B. Sheet No. 64 (DrawingE-2): Roadway Lighting Part "B"
- 1. Currently a single feeder containing 2#10, #10G, ¾" C, is shown from the single service to each pullbox/light fixture. Provide an additional feeder of 2 #10, #10G, 2" C, to also loop through each pull box. One circuit will serve Roadway Lighting exclusively while the other feeder will serve Pedestrian and Irrigation Systems.
- 2. Change the conduit size to 2-inch.
- C. Sheet No. 65 (Drawing E-3): Roadway Lighting Part "C"
- 1. Currently a single feeder containing 2 #10, #10G, ¾" C, is shown from the single service to each pullbox/light fixture. Provide an additional feeder of 2 #10, #10G, 2" C, to also loop through each pull box. One circuit will serve Roadway Lighting exclusively while the other feeder will serve Pedestrian and Irrigation Systems.
- 2. Change the conduit size to 2-inch.
- D. Sheet No. 66 (Drawing E-4): Roadway Lighting Part "D"
- 1. Add an additional Utility Service/Detail 2/E-5 for a total of two (2)
  Utility Services. One service will serve Roadway Lighting exclusively
  and the other service will serve Pedestrian and Irrigation Systems.
- 2. Currently a single feeder containing 2 #10, #10G, ¾" C, is shown from the single service to each pullbox/light fixture. Provide an additional feeder of 2 #10, #10G, 2" C, to also loop through each pull box. One circuit will serve Roadway Lighting exclusively while the other feeder will serve Pedestrian and Irrigation Systems.
- 3. Change the conduit size to 2-inch.

- E. Sheet No. 66 (Drawing E-5): Details.
  - 1. Modify detail 3/E-5 height currently indicated for the pole as 38'-0" to 17'-0".
  - 2. Detail (3/E-5) Fixture SAA add the following: "For combination roadway/pedestrian type SA fixture, the light fixture and obstruction light will both be served from the pedestrian lighting circuit and be controlled by the obstruction light photo cell."
- F. Sheet No. 67 (Drawing E-6):
- 1. The roadway light fixture SA will be served from the roadway lighting circuit and be controlled by the photo cell mounted on top of the roadway fixture.
- 2. Add the description "photo cell" to the leader line on the detail for fixture SB to control the fixture.
- G. Sheet No. 68 (Drawing E-7): Schedules
  - 1. Modify Light Fixture Schedule note 4. Note should read "Similar to "SA" except 17-feet maximum height pole".
  - 2. Add the following:
  - "The poles shall be steel and the color of the top coats shall be selected by the Town of Addison. The manufacturer of the fixtures is Lumec-Schneder of Canada, local representative, 214-658-9000."
- H. Locations of light standards shown on Electrical Sheets are shown schematically. Place light standards to locations as shown on coordinate layout plans, make field adjustment as required and approved by the Town.

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BY J.W.B. DATE 05/04/2010



ITEM	DESCIPTION
1	#3 REBAR, 18 IN. DIA. HOOP, 3 IN. OVERLAP
3	ANCHOR BOLT, GALV. 1" W/25" & 30" POLES & 1/4" W40' POLES
4	CONCRETE FOUNDATION TUBE, 24 IN. DIA.
5	CONDUIT, PVC OR PEC (NOT IN FOUNDATION AUN)
6	WIRE, #5 COPPER, SOLID (SEE NOTE 6)
7	GROUND, POLE BUTT WIRE COIL (SEE NOTE 6)
8	CONCRETE (SEE NOTE 1)

#### STREET LIGHT BASE

NOTE:

1. CONCRETE TO BE MIN. 3000 PSI AT 28 DAYS. (5 SACK) MAX. TOP OF FOUNDATION TO BE TROWELED TO A FLAT AND LEVEL SURFACE. AVOID EXCESSIVE TROWELING. CONCRETE TO SET A MIN. OF 7 DAYS BEFORE POIF INSTALL ATION.

- 2. REBAR HOOPS ARE TIED BEGINNING 3' BELOW TOP OF CONCRETE FORM AND ARE REPEATED AT APPROXIMATE 1' INTERVALS TO POTTEM OF FOLINATION.
- 3. 1" ANCHOR BOLTS TO BE USED WITH 25' & 30' SQUARE AND ROUND POLES.
- 4. CONCRETE FORM OF SONOTUBE TO EXTEND TO BOTTOM OF TRENCH OR AS NEEDED.
- 5. PROVIDE MIN. 24" GROUND WIRE PIGTAIL OUT OF CONCRETE FOR CONNECTION TO POLE.
- 6. A MIN. OF 12' OF BARE #5 SD CU WIRE TO BE PLACED IN BOTTOM OF HOLE AND COVERED WITH 2' OF DIRT.
- 7. IF SOIL HAS BEEN DISTURBED, EXTEND FOUNDATION BY DEPTH OF DISTURBED SOIL.
- 8. NOT APPLICABLE FOR POLE HEIGHT GRATER THAN 40'.

These plans and related specification were prepared for construction of the specific project only. Reuse of their documents is not permitted without written authorization of Birkhoff,

# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
ELECTRICAL MODIFICATION NOTES

BIRKHOFF, HENDRICKS & CONWAY, L.L.P.

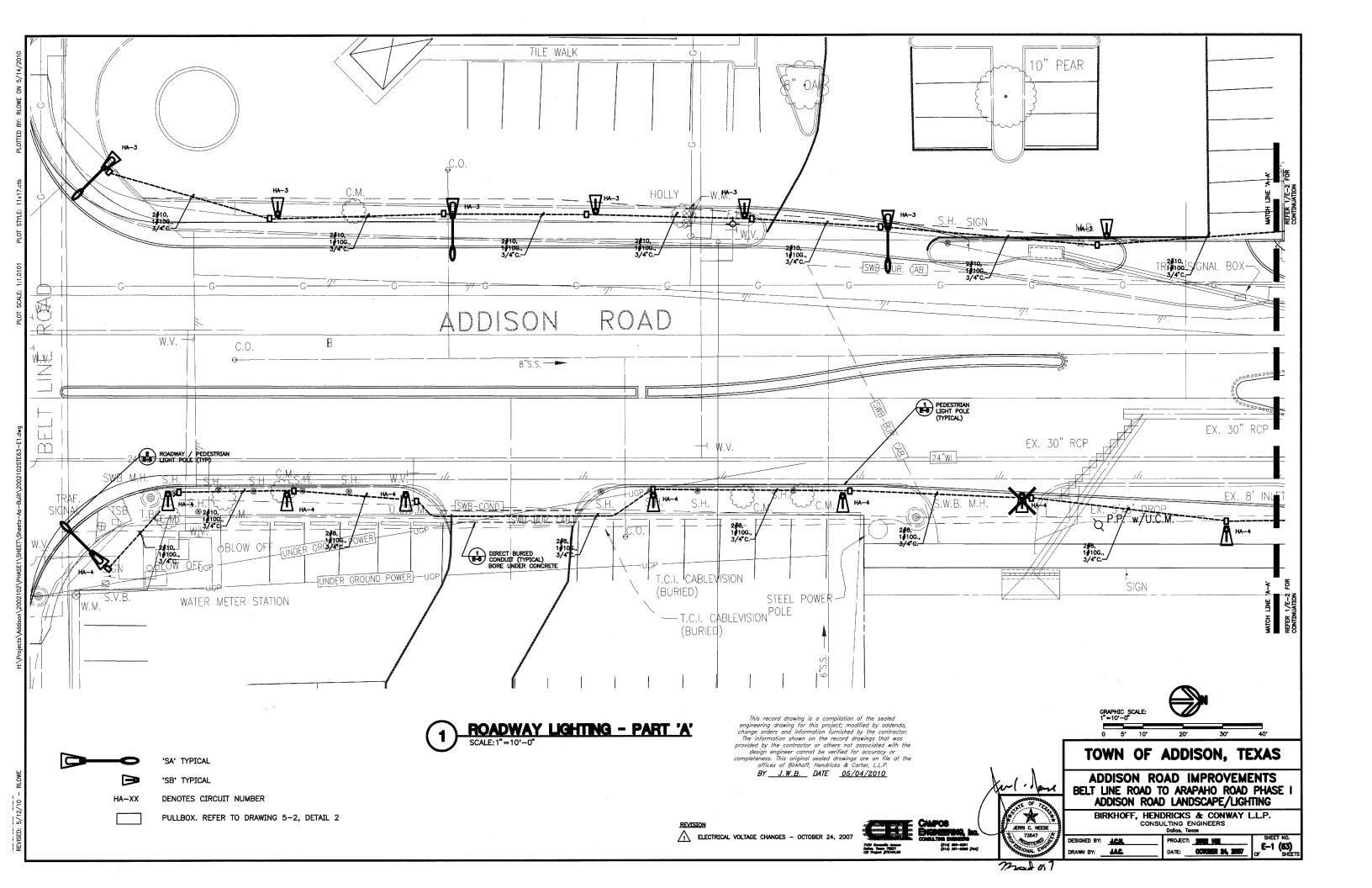
NSULTING ENGINEER

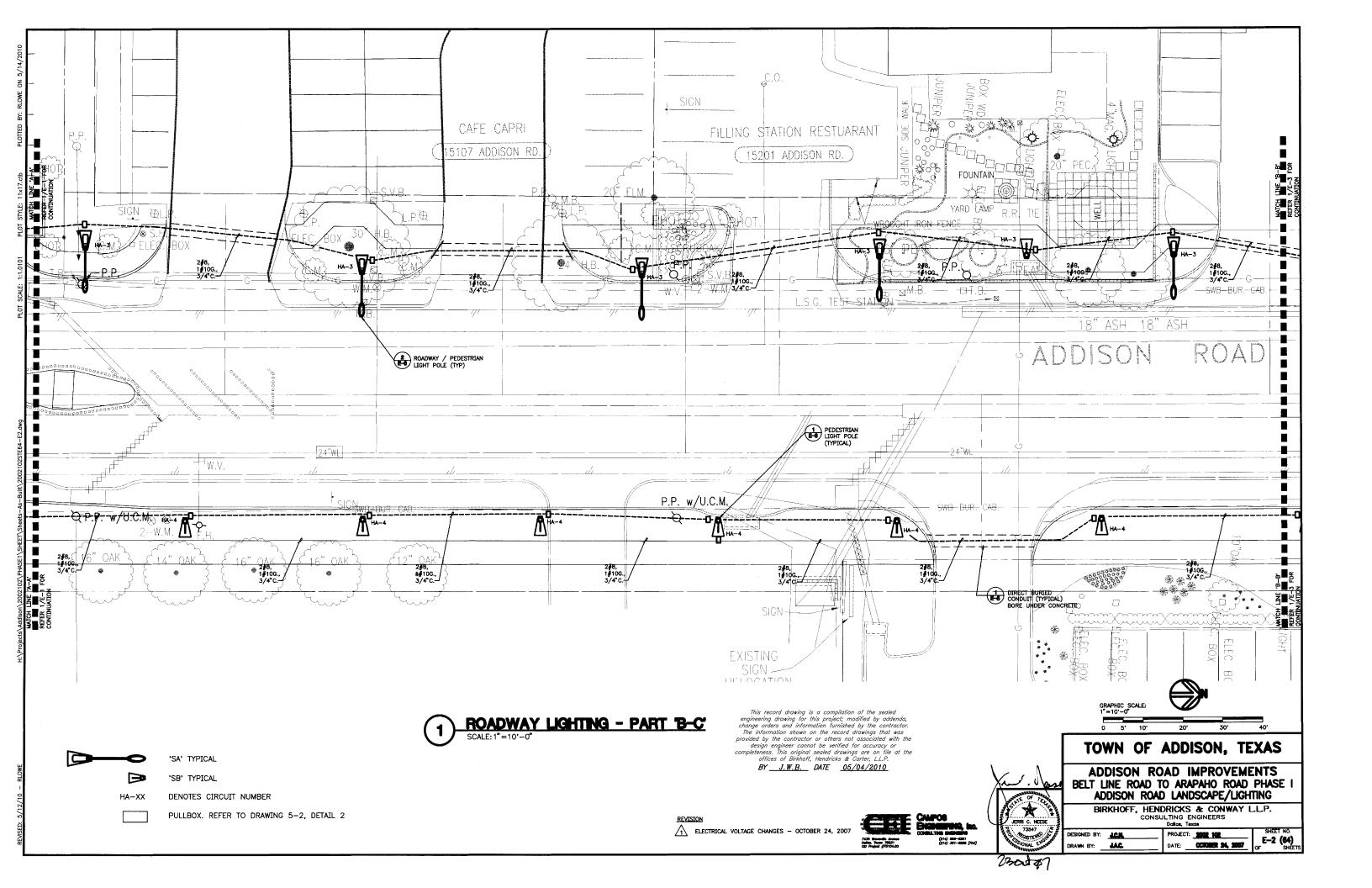
DESIGNED BY: G.C. PROJECT: 2002 102 SHEET NO.

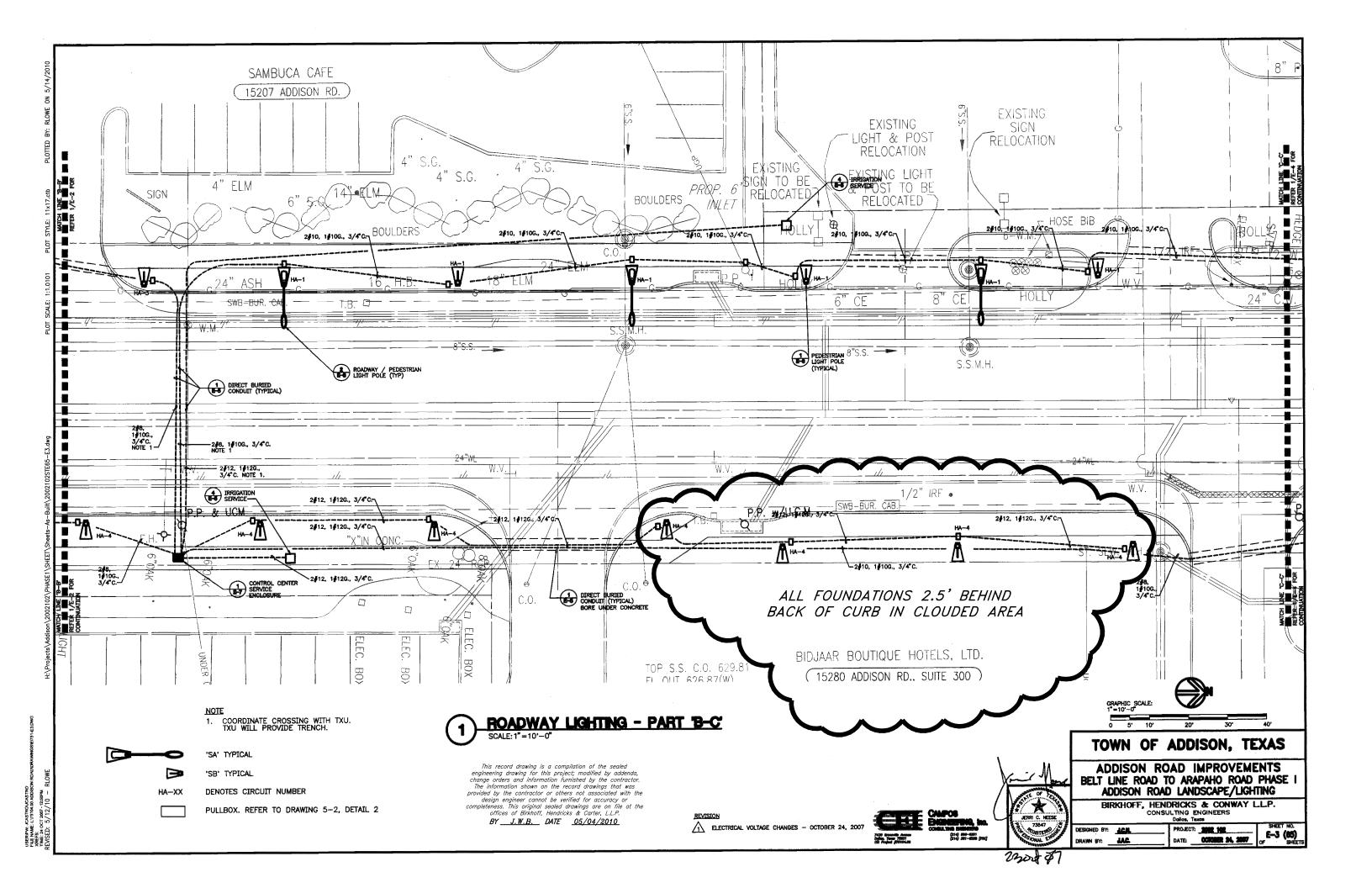
DRAWN BY: B.H. & C. DATE: SEPTEMBER 2006 OF 68 SH.

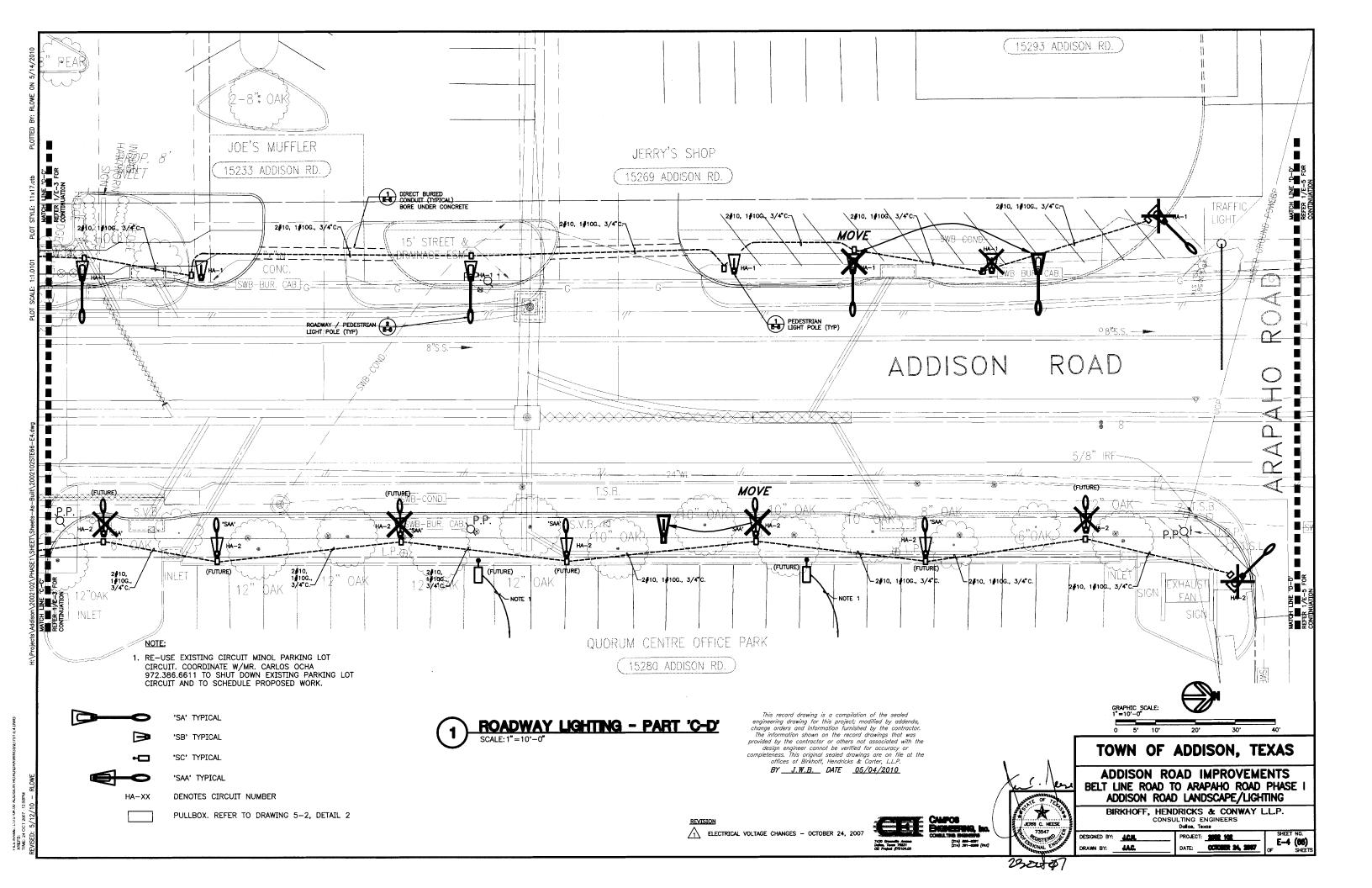
THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.

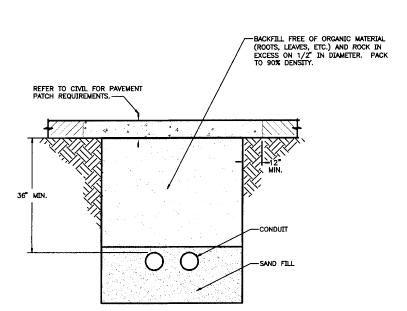
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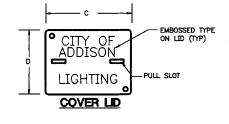










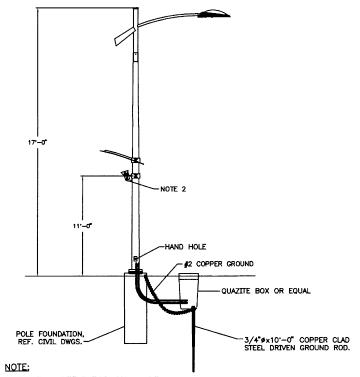


COVERS							
PULLBOX TYPE	DIMENSIONS (INCI						
	A	B	THK.				
A	23¼"	1374"	2"				

B	OXES					
PULLBOX TYPE	DIMENSIONS (INCHES					
	С	۵	E			
A	231/4"	13¾"	18"			

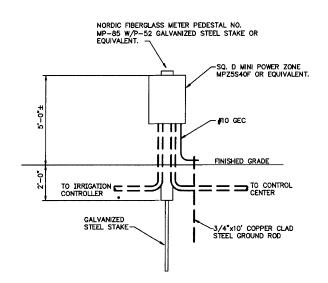
-INSTALL 1-2" CONDUIT FOR FUTURE USE STUBBED AND CAPPED 24" PAST THE EDGE OF THE FOUNDATION.

- 1. SEE STANDARD SPECIFICATIONS FOR MATERIALS AND CONSTRUCTION.
- 2. COVER LETTING SHALL BE 1" LETTERS CAST—IN PLACE WITH THE LID. THE COVERS OR LIDS SHALL BE MARKED "CITY OF ALLEN LIGHTING" WITH CLEAN, EVEN STROKE LETTERING.
- 3. ALL DIMENSIONS ARE NOMINAL.



1. FIXTURE & REFLECTOR SHALL BE CAPABLE OF ORIENTATION TOWARDS THE ROADWAY OR 180° FROM THE ROADWAY.



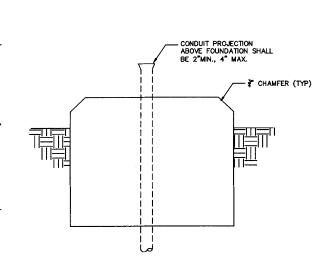


IRRIGATION SERVICE



- GROUT BOTTOM OF PULLBOX 1" THICK WITH 2" DRAIN HOLE

- EMBEDDED THREADED STAINLESS STEEL INSERTS (2 OPPOSITE CORNERS)



# ROADWAY/PEDISTRIAN FIXTURE 'SAA'

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BY J.W.B. DATE 05/04/2010



4" SLEEVE FOR SERVICE CONDUIT

REFER TO DRAWINGS FOR

1" SLEEVE (FOR GROUND ROD) SHALL BE INSERTED WHEN FOUNDATION IS

POURED. INSTALL A \$"X10" BONDED COPPER GROUND ROD WITH CLAMP.

GALVANIZED 1"X12"X4"
COMPLETE WITH NUTS AND
WASHERS. ANCHOR BOLT
PROJECTION ABOVE
FOUNDATION SHALL BE

2"MIN., 21"MAX. -

SIZES. (TYP) -

# CONTROL CENTER FOUNDATION DETAIL

ELECTRICAL VOLTAGE CHANGES - OCTOBER 24, 2007



# GRAPHIC SCALE:

# TOWN OF ADDISON, TEXAS

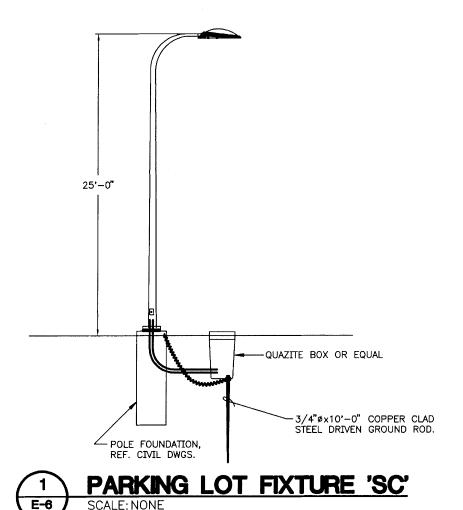
ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I ADDISON ROAD LANDSCAPE/LIGHTING

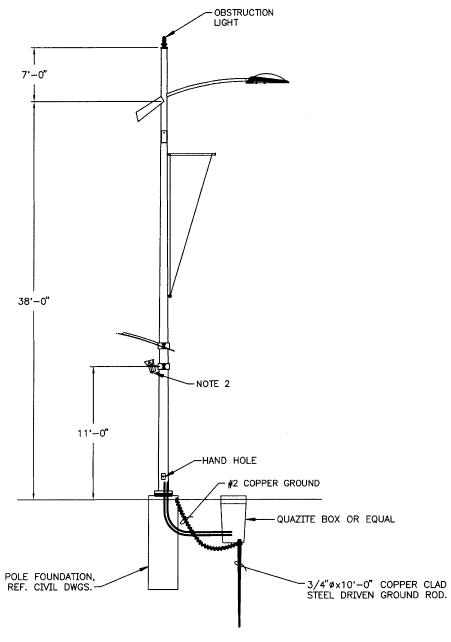
BIRKHOFF, HENDRICKS & CONWAY LLP.
CONSULTING ENGINEERS

PROJECT: 2002 102 DESIGNED BY: JCM DATE: OCTOBER 34, 2007 DRAWN BY: JAC

E-5 (66) F SHEE

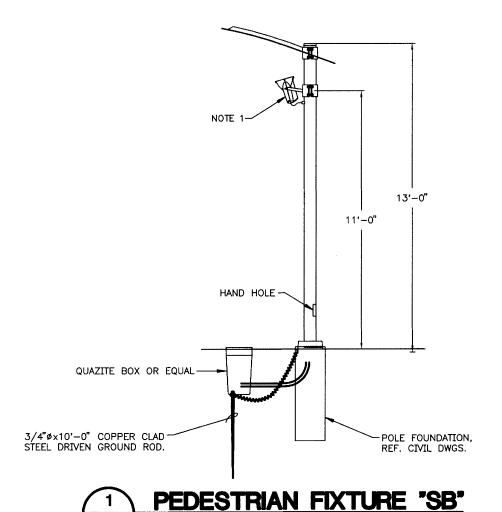
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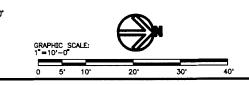


NOTES: 1. PROVIDE 277V/1ø PRIMARY/120V SECONDARY, XFMR 1KVA TO SERVE OBSTRUCTION LIGHT.

> 2. FIXTURE & REFLECTOR SHALL BE CAPABLE OF ORIENTATION TOWARDS THE ROADWAY OR 180 FROM THE ROADWAY.



NOTES: 1. FIXTURE & REFLECTOR SHALL BE CAPABLE OF ORIENTATION TOWARDS THE ROADWAY OR 180' FROM THE ROADWAY.



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ROADWAY/PEDESTRIAN FIXTURE 'SA'

£ ELECTRICAL VOLTAGE CHANGES - OCTOBER 24, 2007

# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I ADDISON ROAD LANDSCAPE/LIGHTING

BIRKHOFF, HENDRICKS & CONWAY LLP.

CONSULTING ENGINEERS
Dallas, Texas DESIGNED BY: 1CH PROJECT: 2002 102 DATE: OCTUBER 24, 2007



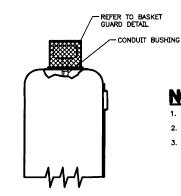
SCALE: NONE

E-6 (67) OF SHEETS

			FIXTURE DATA								
TYPE:	MANUFACTURER:	VOLTS:	CATALOG NO.:	LAMPS:	DESCRIPTION:	LAMP/FIXT	BALLAST/ FIXT	VA/FIXT	NO. FIX	TOTAL KV	
SA	LUMEC-SCHREDER	277	HSX-400MHBT28-SCTX- LTS884A	1-MS400/HOR I-MP70P/U/MED	ROADWAY/PEDESTRIAN FIXTURE NOTE 1,5 & 6	2	2	580	17	9.86	
SB	LUMEC-SOHREDER	277	FOCAL-70MHT8-NA RROW -SCTX-LTS884A	1-MP70P/U/MED	PEDESTRIAN FIXTURE NOTE 2,5 & 6	1	1	95	32	3.04	
sc	WMEC-SCHREDER	-	HBM400MHMN3CWA RDS-1AFS2PH8	1-MS400/HOR	PARKING LOT FIXTURE 400W MH, TYPE III DIST	1	1	458	2	0.92	
SAA	WMEC-SCHREDER	277	HSX-400MHBT28-SCTX- LTS884A	1-MS400/HOR 1-MP70P/U/MED	ROADWAY/PEDESTRIAN FIXTURE NOTE 4, 5 & 6	2	2	580	3	1.74	
SAA		277				2	2	580	3		
2	POLE SHALL BE AT POLE SHALL BE LT POLE SHALL BE HE	S885A-1				BUDGET:	DE	SCRIPTIC	N:	ACTUA	

	FEED	ER PER DRAWINGS						PANE	L <u>НФ</u> .	•		LOCA	TED IN	FRON	IT OF OUTBACK STEAK	HOUSI	E
ст	LOAD	DESCRIPTION	Ċ	EGC	N	W	СВ	LOAD	LOAD	СВ	W	N	EGC	С	DESCRIPTION	LOAD	CI
1	3265	NORTHWEST FIXTURES	•		•	•	20	6345	******	20	•	•	•	*	(FUTURE) FIXTURES	3080	2
3	3650	SOUTHWEST FIXTURES	•	•	•	•	20	354 141	5935	20	•	•	•	•	EAST FIXTURES	2285	4
5	0	SPARE	-	-		-	20	0	11111111	20	-	-	- 1		SPARE	0	6
7	0	SPARE		-	**	-	20		0	20		-			SPARE	0	8
9	0	SPACE	-	-		T-	-	0		-		-	-		SPACE	0	10
11	0	SPACE	***	-		-	-	****	0			-			SPACE	0	12
			4	180Y/27	V A	BUS		3	PHASE MLO		4 X	WIRE GROL	IND BA	R	PEDESTAL MOUNTED NEMA 3R	-	
				60	Α			X	MCB			ISO G	ROUNE	BAR	MINIMUM 10,000 AIC		
OTE	S														PANEL TOTALS:	KVA	AMI
•=	REFE	R TO DRAWINGS FOR W	RING	AND C	ONDL	IT SIZE	S.								PHASE A:	6.3	13.
															PHASE B:	5.9	12
															TOTAL:	12.3	25.

DEMAND LOAD CALCS:	CONTINUOUS KVA:	X	1.25	Ξ	15.
	RECEPTACLE KVA:	х	1.00	=	0.0
KITCHEN EQU	IPMENT LOADS (NONE):	х	1.00	=	0.0
	OTHER LOADS::	x	1.00	=	0.0
	TOT	TAL E	OV KVA:	=	15.
	TOTAL DI	V AN	PERES:	=	32.0



SIMILAR TO "SA" EXCEPT 20' MAXIMUM HEIGHT POLE

5. QUANTITIES INDICATED ARE FOR CALCULATION ONLY, CONTRACTOR SHALL REVIEW DRAWINGS FOR TAKE-OF

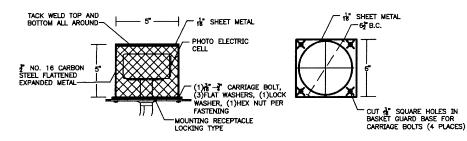
1. ORIENT PHOTO ELECTRIC CELL TO FACE NORTH.

TOTAL HEATED SQ FT. INTERIOR VAIPER SQ FT EXTERIOR LIGHTING KVA

12.90

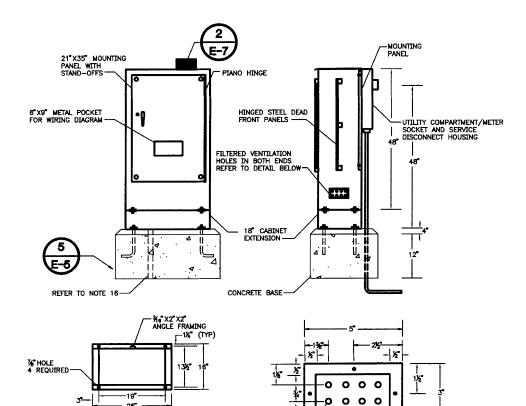
- 2. ALL CONDUIT CONNECTIONS SHALL BE WATERTIGHT
- 3. REFER TO PHOTO ELECTRIC CELL SPECIFICATIONS.

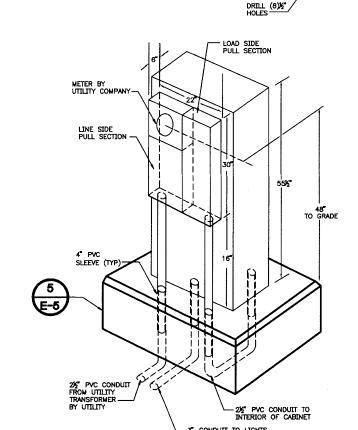
# MOUNTING DETAIL



# BASKET GUARD DETAIL







CONTROL CENTER SERVICE ENCLOSURE

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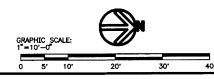
BRASS MACHINE SCREWS

(4)32X%" CUT FLUSH TO OUTSIDE OF CABINET

BY J.W.B. DATE <u>05/04/2010</u>

## **NOTES**

- . Install free standing and self-supporting control center cabinet, per the drawings, nema 3r construction. Use aluminum ALLOY, THICKNESS 0.125", FOR CABINET HOUSING AND DOORS.
- USE HELIARC WELDING METHOD FOR ALL EXTERNAL AND INTERNAL CONTINUOUS WELDS. MAKE WELDS FREE OF CRACKS, BLOW HOLES AND OTHER IRREGULARITIES. MAKE INSIDE AND OUTSIDE EDGES OF CABINET FREE OF BURRS AND PROVIDE SMOOTH, UNIFORM AND NATURAL ALUMINUM FINISH.
- MAKE FULL HEIGHT DOORS WHICH USE STAINLESS HINGES AND HAVE DOOR STOPS AND LOCKS.
- USE TAMPER-PROOF CABINETS SO AS TO PREVENT INSERTION OR WIRES OR OBJECTS INSIDE THE CABINET BY UNAUTHORIZED PERSONS.
- 5. PROVIDE BOLT-ON CIRCUIT BREAKERS.
- PROVIDE A SLOPED SURFACE ON THE TOP OF THE CABINET SO AS TO PREVENT WATER ACCUMULATION.
- 7. PROVIDE DOUBLE FLANGED DOOR OPENINGS ON ALL (4) FOUR SIDES TO INCREASE STRENGTH AROUND THE OPENINGS AND ALSO TO KEEP AWAY ANY LIQUID OR DIRT FROM ENTERING THE ENCLOSURE WHEN THE DOOR IS OPENIND.
- 8. USE DOOR RESTRAINTS TO PREVENT DOOR MOVEMENT WHEN OPENED IN WINDY CONDITIONS.
- FURNISH GASKETING FOR DOORS WHICH SATISFY PHYSICAL PROPERTIES AS FOUND IN UL 508 TABLE 21.1. MAKE WEATHER—TIGHT SEALS BETWEEN THE CABINET AND THE DOOR.
- 10. USE 1/4-20 300 SERIES STAINLESS STEEL BOLTS AND 300 SERIES STAINLESS STEEL NY-LOCK NUTS FOR BOLTING HINGES TO THE CABINET
- 11. FURNISH HINGES MADE OF 075-14 GAUGES 300 SERIES STAINLESS STEEL WELD THE TOP AND BOTTOM OF THE HINGE PIN TO RENDER IT TAMPER PROOF.
- 12. FURNISH GASKETING FOR BOLT HOLES WHICH MEET OR EXCEED THE REQUIREMENT OF A NEMA 3R RATING.
- 13. REFER TO SPECIFICATIONS FOR ADDITIONAL CONSTRUCTION REQUIREMENTS.
- 14. THE DOOR SHALL BE RAINTIGHT AND DUSTPROOF
- 15. INSTALL 5/8" DIA. BY 10' COPPER CLAD GROUND ROD WITH CLAMP.
- 18. IN UNPAVED AREAS A RAISED PCC PAD 36"X4"XWIDTH OF FOUNDATION, SHALL BE PLACED IN FRONT OF THE CABINET, PAD SHALL BE SET 2" BELOW THE FOUNDATION ELEVATION. SLOPE PAD AWAY FROM CABINET.
- 17. USE AN APPROVED SILICON SEALER RTV TYPE GREY IN COLOR OR CLEAR, BETWEEN CABINET AND FOUNDATION.
- 18. UTILITY COMPANY SHALL PROVIDE AND INSTALL THE TRANSFORMER, TRANSFORMER PAD, CONDUIT, AND WIRING TO METER. THE UTILITY WILL PROVIDE THE METER BASE TO BE INSTALLED BY CONTRACTOR.
- 19. COORDINATE THE CONTROL CENTER LOCATIONS AND EQUIPMENT SPACING WITH THE UTILITY COMPANY. PROVIDE A SHOP DRAWING WITH DIMENSIONS FOR EACH UTILITY SERVICE LOCATIONS. SHOW THE PROPOSED UTILITY EQUIPMENT AND CONTROL CENTER LOCATIONS. SUBMIT TO THE ENGINEER FOR WRITTEN APPROVAL PRIOR TO INSTALLING EQUIPMENT.

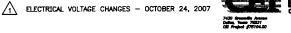


# TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS BELT LINE ROAD TO ARAPAHO ROAD PHASE I ADDISON ROAD LANDSCAPE/LIGHTING

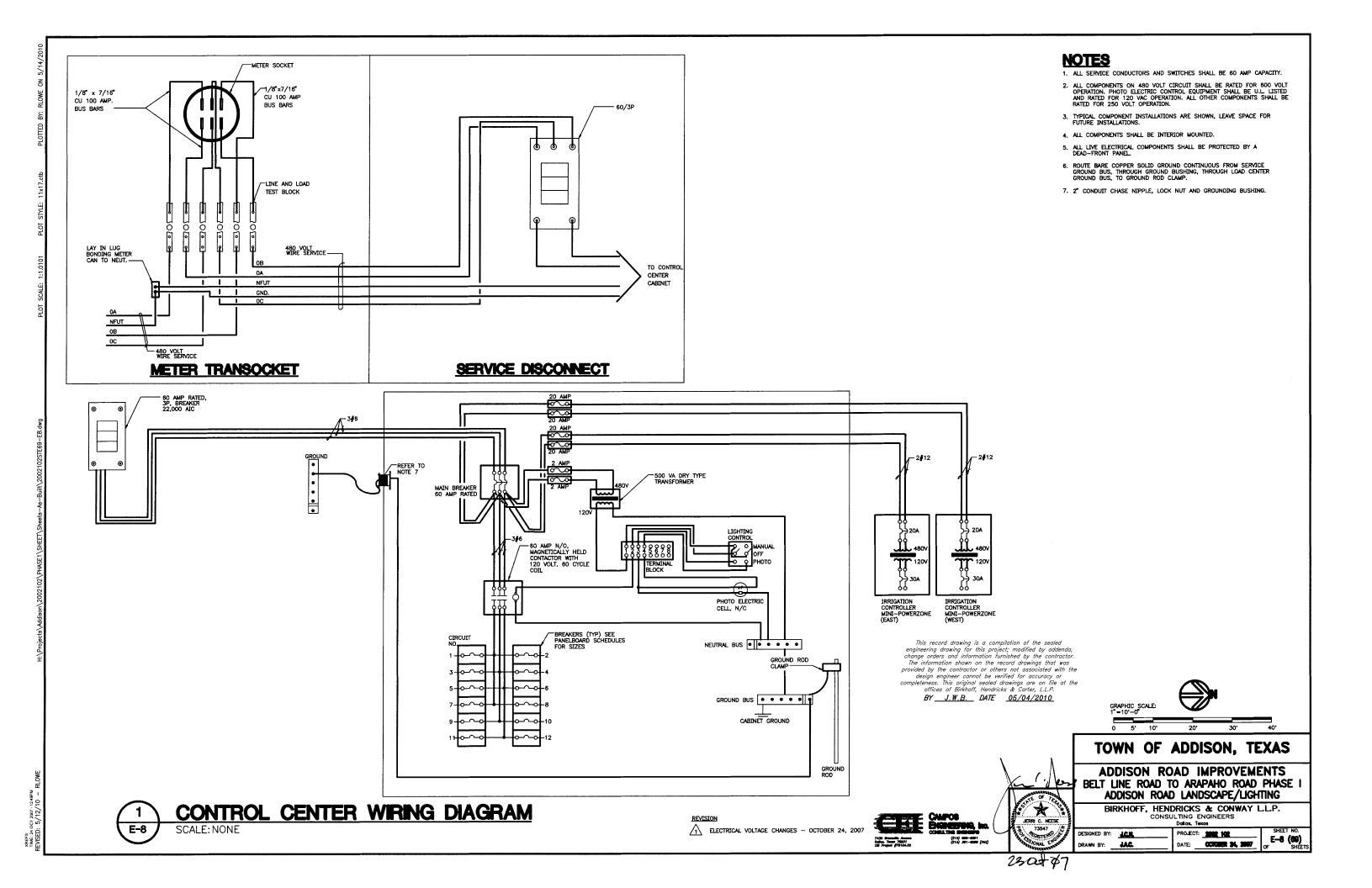
BIRKHOFF, HENDRICKS & CONWAY L.L.P. CONSULTING ENGINEERS

DESIGNED BY: 101 RAWN BY: JAC PROJECT: 2002 102 E-7 (66) DATE: **OCTOBER 34, 3957** 



CAMPOS ENGINEER

(214) 800-6201 (214) 301-6200 [FW]



II. GROUND ROOS

#### A. MATERIALS

- 1. All ground rods installed at electrical services, including supplemental lightning protection ground rods specified by the plans in other locations such as pole bases, shall be capper coaled steel and listed by a NRT. Rods shall be a minimum diameter of 5/8 inch. The length shall be a minimum of 8 feet, Larger diameter or longer length rods may be called for in some specific locations, see the individual pions sheets.
- 2. Ground rod clamps shall be listed to be in direct contact with the soil. Where concrete encasement is required, the clamp shall be listed for concrete encasement.

#### B. CONSTRUCTION METHODS

- 1. Ground rads installed in locations such as pole bases, to provide supplemental lightning protection need not be locally in contact with the soil. Where called for by the plans, rods may be encased in soil or concrete or any combination of soil and concrete. When concrete encosed, the connection of the conductor to the rad shall be readily accessible for inspection or repairs. When driven into the soil the upper end shall be between 2 to 4 inches below finished grade. Ground rads shall not be placed in the same drilled hale as a timber pale.
- 2. Growind rods shall be installed such that the end imprinted with the rod's part number is installed as being the upper end.
- 3. Non-conductive coolings such as concrete splatter shall be removed from the rod at the clamp location. 4. Routing of lightning protection ground rod wires shall be run as short and straight as possible. Where bends are required
- they shall have a minimum radius of four inches. 5. Unless specifically called for by the plans, conduits used for ground rad wires shall be non-metallic. Where matal conduits are specified, a grounding bushing and properly sized bonding isosper shall be provided and properly installed
- 6. Where racky soil or a solid rack battern is encountered when driving a ground rad and the horizontal trench placement method is the any wable solution, written authorization from the Town must be aquired.

#### m. GROUND BOX

#### A. MATERIALS

- 1. Ground boxes 15x30x24 inches (WxLxD) or smaller shall be polymer concrete of the type required by the descriptive code shown elsewhere Larger ground baxes shall be as shown elsewhere in the plane.
- 2. All around boxes and covers shall be permanently marked either by impress or by permanent ink, with manufacturer's model number and manufacturer's name or logo.
- 3. Covers shall be boiled down, and boilt holes in the box shall be arranged to drain dirt.
- 4. Ground box Types A. B. C. D & E shall meet the following requirements:
- a. Ground boxes and covers will be manufactured from polymer concrete reinforced with continuous strongs of woven or stitched borosilicate fiberglass cloth. The polymer concrete shall be made from catalyzed polyester resin, sand and aggregate, and shall have a minimum compressive strength of 11,000 psi. Palymer concrete containing chapped liberalass or liberaloss reinforced plastic is not acceptable.
- b. Minimum inside dimensions shall be as follows (width a length a depth):

Type A shall be 11.5 inches x 21 inches x 10 inches. (122311)

Type B shall be 11.5 inches x 21 inches x 20 inches, (122522)

Type C shall be 15.25 inches x 28.25 inches x 10 inches, (1629)1

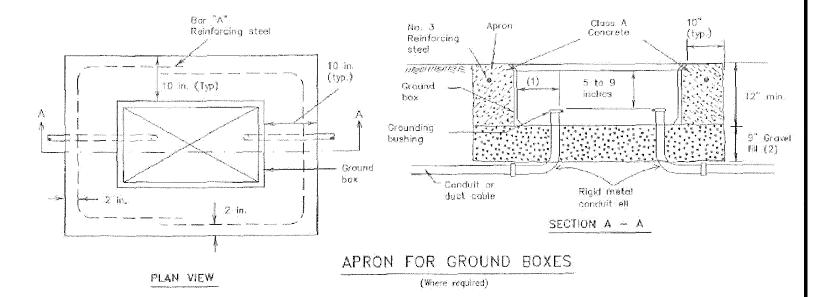
Type O shall be 15.25 inches x 28.25 inches x 20 inches, (162922) Type E shall be 11.5 inches x 21 inches x 16 inches, (122317)

- c. Bottom edge of box or extension shall be tooled with a minimum 1 1/4 inch florige
- d. Ground boxes shall withstand 600 lbs. per sq. ft. applied over the entire sidewall with less than 1/4 inch deflection per foot length of box. Ground boxes and covers shall withstend a test loading of 20,000 lbs, over a 10 inch by 10 inch area centered on the cover with less than 1/2 inch deflection. Cround boxes and povers shall meet Western Underground Standards 3.6. Manufacturer shall supply certification by an independent laboratory or scaled by a Taxos-Licensed
- e. Covers shall be 2 inch (nominal) thick polymer concrete. All hardware shall be stalless steel. Cover shall be secured with two 1/2 inch stainless steel bolts. Balts shall be self-retaining and shall withstand a minimum of 70 ft-lbs. torque and shall have a minimum 750 lbs. straight pull out strength. Nots shall be floating and shall provide a minimum of 1/2 inch movement from the center of the nut. Covers shall be skid resistant, minimum 0.5 coefficient of friction. Covers shall be interchangeable between manufacturers and shall conform to the dimensions shown herein. Unless otherwise approved by the Engineer, cover shall be legibly imprinted with the following words in minimum 1 inch
  - Ground Boxes containing wiring for traffic signals shall be lobeled. Danger High Voltage Traffic Signals. Ground boxes containing wiring for illumination systems shall be lobeled. Danger High Voltage Illumination. Ground boxes containing wiring for traffic management systems shall be labeled. Danger High Voltage Traffic Management,
  - Ground boxes containing wiring for sign Illumination systems shall be labeled, Danger High Vallage Sign - Numination.
  - Cround poxes containing wiring for traffic signals that also contain flumination, powered by the signal electrical service, shall be liabeled, Danger High Valtage Traffic Signal.

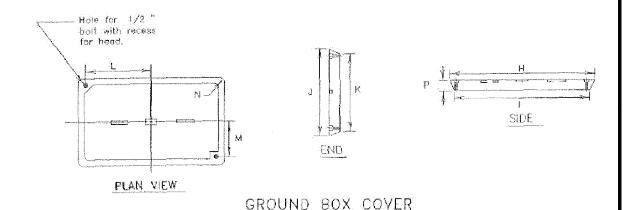
#### 8. CONSTRUCTION METHODS

- 1, Ground boxes shall be set on a 9 inch (minimum) bed of coarse No. 1 aggregate as defined by Item 421. Gravel shall be in place prior to esting box and conduits shall be capped. Any gravet or dirt in conduit shall be removed.
- 2. When required by Item descriptive code, construction of an opron encasing a ground box including concrete and reinforcing steel shall not be paid for directly but shall be subsidiary to the ground box. Reinforcing steel may be field bent. Concrete for oprons shall be considered miscellaneous concrete for testing purposes. Aprens shall be cost in place.
- 3. Conduit holes may be cut in the walls of type B & D baxes at least 18 inches beneath the cover.
- 4. If, within the limits of this project, the Contractor must utilize an existing ground box equipped with a metal cover, the Contractor shell bond the cover to the grounding conductor with a 3 foot long flexible standed jumper the same size as the grounding conductor. Connection of bonding jumper to metal ground cover shall not be paid for directly but shall be subsidiary to virious bid items. The box(es) must be clearly shown on the plans with plan notes fully describing the
- 5. If there are other ground boxes with metal Covers within the project limits but not involved in the contract, the Engineer may direct the Contractor to ground the covers, designating and identifying the specific boxes in writing. This work will be gold for separately.
- 5. Termination to metal ground box covers shall be made using a tank ground type lug.





- (1) Final position of end of conduit shall not exceed one-half the distance to the side of box apposite the conduit entry.
  (2) Place gravel "under" the box, not "in" the box. Gravel should not encroach on the interior volume of the box.
- (3) install pushing on the upper end of all elis-
- (4) Where a ground rad is present in the ground box, connect it to any and all equipment grounding conductors using a listed connector.
- (5) Maintain sufficient space between all conduits so as to allow for proper installation of bushings.
- (6) All conduits shall be installed in a neat and workmanlike manner.



80X DIMENSIONS (INCHES)								
SIZE	H   1		J	K	L	М	N	þ
A, B & E	23 1/4	23	13 3/4	13 1/2	9 7/8	5 1/8	1 3/8	2
C & 8	30 1/2	-30 1/4	17 1/2	17 1/4	13 1/4	6 3/4	1 3/8	2

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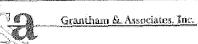
THE STAL ARPEXANNS ON THIS COCCUENT WAS AUTHORIZED BY THE LIAM J. HATCHELL ON 7 - 71 - 04 ALTERATION OF A SEALED DOCUMENT WISHOUT PROPER HORPICATION TO THE RESTOURNESS ENGINEER IS AN OFFEISE LOOSE THE TEXAS ENGINEERING PRACTICE ACT.

THIS DETAIL SHEET WAS OBTAINED FROM TXDOT

GAA DESIGN: BRG REVIEWED: BRG DWG: 320DETAILS-ELEC DRAWN: ARAPAHO ROAD PHASE III STANDARD CONSTRUCTION DETAILS traffic signal electrical - sheet

# TOWN OF ADDISON

MAY 2004 SCALE: NOT TO SCALE JOB NO.:



ng sisaroh road, sutte 3/2, l.s. s

TS-6972) 984-7333 (FEU (972) 884-2394 (FA ))

SHT.

