GENERAL NOTES:

A. FOUNDATIONS:

FOUNDATIONS FOR THE TOWER AND BASE BUILDINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH RECOMMENDATIONS OF GEOTECHNICAL REPORT NO. 0761-1044 OF AUGUST 1996 BY FUGRO-McCLELLAND INC., DALLAS, TEXAS.

- B. WIND LOADS: 70 MPH, EXPOSURE C, 1994 UNIFORM BUILDING CODE.
- C. EARTHQUAKE LOADS: SEISMIC ZONE 0.

D. GENERAL:

- ALL EXISTING FIELD CONDITIONS AND STRUCTURAL DIMENSIONS CONTROLLED BY OR RELATED TO MECHANICAL OR ELECTRICAL EQUIPMENT SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. THE RESIDENT ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES FOUND.
- MECHANICAL AND ELECTRICAL EQUIPMENT SUPPORTS, ANCHORAGES, OPENINGS, RECESSES AND REVEALS NOT SHOWN ON THE STRUCTURAL DRAWINGS BUT REQUIRED BY OTHER CONTRACT DRAWINGS SHALL BE PROVIDED PRIOR TO CASTING CONCRETE.
- TOWER HAS BEEN DESIGNED FOR OPERATIONAL LOADS ON COMPLETED STRUCTURE. DURING CONSTRUCTION STRUCTURE SHALL BE PROTECTED BY BRACING AND BALANCING WHEREVER EXCESSIVE CONSTRUCTION LOADS MAY OCCUR.
- 4. WHERE THE WEIGHT OF EQUIPMENT OR MATERIALS BEING TRANSPORTED TO LOCATION. OR TEMPORARILY STORED. EXCEEDS THE DESIGN LIVE LOAD, PLANKING SHALL BE PROVIDED ON THE FLOOR SLAB AND SHORING PROVIDED BENEATH THE FLOOR.
- 5. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SIZE AND LOCATION OF WALL, ROOF AND FLOOR OPENINGS, SLEEVES AND CONCRETE PADS UNDER EQUIPMENT. THE CONTRACTOR SHALL VERIFY EXACT SIZE AND LOCATION WITH EQUIPMENT FURNISHED.

PRECAST CONCRETE PANELS:

- REINFORCED CONCRETE FOR PRECAST PANELS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF (f'c) 6000 PSI AT 28 DAYS (NORMAL WEIGHT).
- 2. REINFORCING STEEL SHALL BE NEW BILLET STEEL, DEFORMED BARS, CONFORMING TO ASTM SPECIFICATION A 615. GRADE 60 WITH A MINIMUM YIELD STRENGTH OF (fy) 60,000 PSI.
- 3. ALL REINFORCING SHALL HAVE A MINIMUM COVER OF 2" UNLESS SHOWN OTHERWISE ON DRAWINGS.
- 4. THE PRECAST MANUFACTURER SHALL BE RESPONSIBLE FOR PRECAST PANEL REINFORCING DESIGN FOR MANUFACTURING AND HANDLING LOADS AND LOADS AFTER THE PANELS ARE INSTALLED AT THEIR FINAL POSITION. THE DESIGN WIND PRESSURE SHALL BE IN AC-CORDANCE WITH UBC-94.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR BRACING EACH PANEL IN POSITION BEFORE RELEASING IT FROM THE LIFTING CRANE. THESE BRACES SHALL REMAIN IN PLACE UNTIL ALL CONNECTIONS ARE COMPLETED.
- REFER TO ARCHITECTURAL DRAWINGS FOR ALL CHAMFERS AND REVEAL STRIP LOCATIONS.
- 7. REFER TO ARCHITECTURAL AND STRUCTURAL DETAILS FOR TYPES AND LOCATIONS OF ALL EMBEDDED PLATES AND INSERTS WHICH SHALL BE INSTALLED ON EACH INDIVIDUAL PANEL.
- 8. WHERE WELDS OF PANEL CONNECTIONS ARE GREATER THAN 3/16", USE MULTIPLE PASSES ALLOWING TIME TO DISSIPATE HEAT IN ORDER TO PREVENT CRACKING OF CONCRETE PANELS.
- NON-SHRINK MORTAR BETWEEN PANEL JOINTS SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF (f'c) 5000 PSI (NORMAL WEIGHT).
- 10. MISCELLANEOUS METAL FOR PRECAST WALL PANEL CONNECTIONS (ANGLES AND PLATES) SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123-890. PREPARED AND FIELD PAINTED AS SPECIFIED.
- II. THE PRECAST MANUFACTURER SHALL BE RESPONSIBLE FOR PRECAST PANEL CONNECTIONS TO THE STRUCTURE. CONNECTIONS SHOWN ON DRAWINGS ARE FOR DESIGN INTENT ONLY.

F. STRUCTURAL STEEL:

- I. ALL STRUCTURAL STEEL SHALL BE ASTM A36 UNLESS NOTED OTHERWISE.
- 2. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST AISC "SPECIFICATIONS FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- 3. ALL FIELD CONNECTIONS SHALL BE MADE WITH 3/4" DIAMETER ASTM A325 BOLTS (SLIP-CRITICAL CONNECTION). UNLESS NOTED OTHERWISE. AND TIGHTENED BY THE "TURN OF NUT" METHOD (TWO BOLTS PER CON-NECTION MINIMUM).

- 4. STRUCTURAL TUBING SHALL CONFORM TO ASTM A 500-90, GRADE "B" WITH A MINIMUM YIELD STRENGTH OF 46,000 PSI.
- 5. ALL ANCHOR BOLTS SHALL BE ASTM A325 UNLESS NOTED OTHERWISE.
- 6. ALL WELDING SHALL BE DONE WITH E70 ELECTRODES IN CONFORM-ANCE WITH THE LATEST AMERICAN WELDING SOCIETY (AWS) "STRUCTURAL WELDING CODE", AWS DI.I. TYPICALLY PROVIDE BACKING BARS AS REQUIRED FOR ALL COMPLETE PENETRATION GROOVE WELDS.
- 7. ALL BEAM CONNECTIONS NOT SPECIFICALLY INDICATED SHALL BE FIELD BOLTED WHERE POSSIBLE AND DESIGNED BY THE CONTRACTOR. THE CONNECTION SHALL BE DESIGNED TO A MINIMUM OF 1/2 THE TOTAL UNIFORM LOAD CAPACITY AS TABULATED IN PART 2 "ALLOWABLE UNIFORM LOADS FOR BEAMS LATERALLY SUPPORTED" OF THE A.I.S.C. MANUAL OF STEEL CONSTRUCTION (NINTH EDITION) FOR THE GIVEN SHAPE, SPAN AND GRADE OF STEEL BEAM IN QUESTION. DOUBLE ANGLE SHEAR CONNECTIONS SHALL BE USED WHERE POSSIBLE.
- COLUMN BASE PLATES, CAP PLATES, AND STIFFENER PLATES SHALL BE WELDED ALL AROUND.
- 9. STAIR FRAMING AND CONNECTIONS SHALL BE DESIGNED BY STAIR MANUFACTURER.
- 10. LIGHT GAGE METAL FRAMING SHALL CONFORM TO THE ASTM DESIG-NATION AS PER THE AISI SPECIFICATIONS.

G. METAL DECK:

- METAL DECK SHALL CONFORM TO ASTM A 446, GRADE A. GAL-VANIZED WITH A MINIMUM COATING DESIGNATION OF G90.
- 2. METAL DECK SHALL BE DESIGNED FOR A BASIC ALLOWABLE STRESS OF 20,000 PSI (USE ONLY WITH AN INTERLOCKING SIDE LAP).
- ROOF DECK SHALL BE 1.5", WIDE-RIB, 20 GAGE, METAL DECK (UNLESS NOTED OTHERWISE) IN CONFORMANCE WITH THE STEEL DECK INSTITUTE. ROOF DECK SHALL CONFORM TO THE FOLLOWING MINIMUM SECTION PROPERTIES: "I" = 0.212" TO THE FOURTH POWER PER FOOT. "Sp" = 0.234" & "Sn" = 0.247" TO THE THIRD POWER PER FOOT.
- 4. FLOOR DECK SHALL BE 2", WIDE RIB, 18 GAGE, COMPOSITE METAL DECK (UNLESS NOTED OTHERWISE) IN CONFORMANCE WITH THE STEEL DECK INSTITUTE WITH 2.5" MINIMUM NORMAL WEIGHT CONCRETE FILL REINFORCED WITH 4X4-W2.IXW2.I. WWF FLOOR DECK SHALL CONFORM TO THE FOLLOWING MINIMUM SECTION PROPERTIES: "I" = .569" TO THE FOURTH POWER PER FOOT, "Sp" = 0.523" & "Sn" = 0.522" TO THE THIRD POWER PER FOOT.
- DIAPHRAGM ACTION SHALL BE PROVIDED FOR ALL AREAS WITH WELDING PATTERN IN ACCORDANCE WITH SPECIFIED MANUFACTURER'S RECOMMENDATIONS TO PROVIDE THE FOLLOWING SHEAR CAPACITIES.
- 6. CONTRACTOR SHALL BE RESPONSIBLE FOR THE SUPPORT OF METAL DECK FOR THE FOLLOWING OPENINGS: ROOF DRAINS. MECHANICAL PIPE OPENINGS. AND OTHER OPENINGS SMALLER THAN 12X12 INCH
- 7. REINFORCING IN ADDITION TO THE WELDED WIRE FABRIC SHALL BE PLACED IN THE BOTTOM OF THE FLUTES AT LOCATIONS INDICATED ON THE DRAWINGS.

H. ALUMINUM ANCHORS, SHAPES AND CONDUITS:

- WHERE ALUMINUM ANCHORS, ALUMINUM SHAPES, OR ALUMINUM ELECTRICAL CONDUITS ARE EMBEDDED IN CONCRETE. ALL CONTACT SURFACES SHALL BE PAINTED WITH ZINC-CHROMATE PRIMER. THE PAINT SHALL BE ALLOWED TO DRY THOROUGHLY BEFORE THE ALU-MINUM IS PLACED IN CONTACT WITH THE CONCRETE.
- 2. ALUMINUM SURFACES TO BE PLACED IN CONTACT WITH CONCRETE, WOOD OR MASONRY CONSTRUCTION. EXCEPT WHERE THE ALUMINUM IS TO BE EMBEDDED IN CONCRETE, SHALL BE GIVEN A HEAVY COAT OF AN ALKALI-RESISTANT BITUMINOUS PAINT BEFORE INSTALLATION. THE PAINT SHALL BE APPLIED AS IT IS RECEIVED FROM THE MANU-FACTURER WITHOUT THE ADDITION OF ANY THINNER.

I. LIVE LOADS:

TOME

IOWER:	
CAB FLOOR:	150 PSF
MECHANICAL AND ELECTRICAL:	250 PSF
STAIRWAYS, CORRIDORS, LOBBIES	\$ 100 PSF
CAB ROOF:	20 PSF
ALL OTHER FLOORS:	150 PSF

BASE-EG BUILDING:

100 PSF
100 PSF
150 PSF
20 PSF
250 PSF

J. FIRE PROOFING:

SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.

K. REINFORCED CONCRETE:

I. APPLICABLE CODE:

CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF ACI 301 AND ACI 318.

2. REINFORCING STEEL DETAILS:

ALL DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS. UNLESS NOTED OTHERWISE. SHALL BE IN ACCORDANCE WITH THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315), LATEST EDITION.

3. MATERIALS:

A. ALL CAST-IN-PLACE STRUCTURAL CONCRETE SHALL BE IN ACCORDANCE WITH ACI AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF (f'c) 4000 PSI AT 28 DAYS, NORMAL WEIGHT.

B. REINFORCEMENT:

ALL REINFORCEMENT SHALL BE NEW BILLET STEEL, DEFORMED BARS. CONFORMING TO ASTM A615. GRADE 60 WITH A MINIMUM YIELD STRENGTH OF (fy) 60000 PSI. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185-900.

4. CONCRETE COVER:

CONCRETE CLEAR COVER FOR REINFORCING BARS SHALL BE AS FOLLOWS.

- A. CONCRETE CAST AGAINST GROUND 3".
- CONCRETE IN CONTACT WITH GROUND OR EXPOSED TO WEATHER.
 - (1) BARS GREATER THAN #5 2"
 - (2) BARS #5 OR LESS 1 1/2"
- CONCRETE NOT PERMANENTLY EXPOSED TO GROUND OR WEATHER.
- (I) BEAMS AND COLUMNS I I/2" TO SPIRALS, TIES, OR STIRRUPS
- (2) SLABS AND WALLS 3/4"

5. CONSTRUCTION JOINTS:

ADDITIONAL CONSTRUCTION JOINTS SHALL HAVE PRIOR APPROVAL OF CONTRACTING OFFICER.

6. PENETRATIONS:

PENETRATIONS OTHER THAN SHOWN SHALL NOT BE ALLOWED WITHOUT CONTRACTING OFFICER'S APPROVAL.

7. BAR LAP SPLICE LOCATIONS FOR GRAVITY LOADS:

ALL BOTTOM BARS MAY BE SPLICED AT SUPPORT ONLY. TOP BARS MAY BE SPLICED AT CENTER OF SPAN ONLY.

8. RESTRICTED BAR ANCHORAGE:

IN CASES WHERE REINFORCING BARS CANNOT BE EXTENDED AS FAR AS REQUIRED DUE TO THE LIMITED EXTENT OF THE ADJACENT CONCRETE STRUCTURE, THE BARS SHALL EXTEND AS FAR AS POSSIBLE AND END IN STANDARD HOOKS.

9. STANDARD HOOKS:

BARS ENDING IN RIGHT ANGLE BENDS OR HOOKS SHALL CONFORM TO THE REQUIREMENTS OF PAR 7.1. ACI 315.

10. CHAMFERS:

EXCEPT AS OTHERWISE REQUIRED. EXPOSED CONCRETE CORNERS AND EDGES SHALL HAVE 3/4" CHAMFERS, UON.

II. SLABS-ON-GRADE:

SLABS-ON-GRADE, UNLESS NOTED OTHERWISE, SHALL BE 6 INCHES THICK, REINFORCED WITH #3 @ 12" C/C. EW, TOP AND BOTTOM, OVER VAPOR BARRIER.

12. WATER STOP:

WATER STOP SHALL BE GREENSTREAK TYPE-R THERMOPLASTIC ELASTOMERIC RUBBER WATERSTOP. 4" WIDE. DUMBBELL TYPE OR APPROVED EQUAL.

BAR SPLICES:

ALL DOWEL AND LAP LENGTHS SHALL BE AS SHOWN IN TABLE, UNLESS NOTED OTHERWISE.

TENSION LAP SPLICE LENGTHS (INCHES) FOR f'c = 4000 PSI AND fy = 60,000 PSI

BAR	CLASS	TOP BARS				OTHER BARS							
SIZE		CATEGORY				CATEGORY							
		ı	2	3	4	5	6	I	2	3	4	5	6
#3	A	16	16	16	16	16	16	13	13	13	13	3	13
	B	21	21	21	21	21	21	16	16	16	16	6	16
#4	A	23	22	22	22	22	22	18	17	17	17	17	17
	B	30	28	28	28	28	28	23	22	22	22	22	22
#5	A	36	29	27	27	27	27	27	22	21	21	21	21
	B	46	37	35	35	35	35	36	29	27	27	27	27
#6	A	50	40	35	32	32	32	39	31	27	25	25	25
	B	65	52	46	42	42	42	50	40	35	32	32	32
#7	A	69	55	48	39	38	38	53	42	37	30	29	29
	B	89	71	63	50	49	49	69	55	48	39	38	38
#8	A	90	72	63	51	45	43	70	56	49	39	35	33
	B	117	94	82	66	49	56	90	72	63	51	45	43
#9	A	114	91	80	64	57	48	88	70	62	49	44	37
	B	148	119	104	83	74	63	114	91	80	64	57	48
#10	A	145	116	102	81	73	58	112	89	78	63	56	45
	B	188	151	132	106	94	76	145	116	102	81	73	58
#	A	178	142	125	100	89	71	137	110	96	77	69	55
	B	231	185	162	130	116	93	178	142	125	100	89	71

WHERE d_b = NOMINAL DIAMETER OF A BAR

- A. CLASS A = EMBEDMENT OR STAGGERED LAP LENGTH; CLASS B = TYPICAL LAP LENGTH
- LAP SPLICES MAY BE CLASS A ONLY IF NOT MORE THAN 50% OF BARS ARE LAP SPLICED WITHIN ONE LAP LENGTH.
- C. TOP BARS ARE HORIZONTAL REINFORCEMENT PLACED SO THAT MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE BARS.
- OTHER BARS ARE ALL BARS OTHER THAN TOP BARS.
- E. VALUES OF INFOR BARS IN BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT MEETING MINIMUM REQUIREMENTS FOR STIRRUPS IN ACI II.5.4 AND 11.5.5.3, OR MEETING TIE REQUIREMENTS IN ACI 7.10.5: AND ARE BASED ON MINIMUM COVER SPECIFIED IN ACI 7.7.1.
- #11 AND SMALLER EDGE BARS WITH C/C SPACING NOT LESS THAN 6db ARE ASSUMED TO HAVE A SIDE COVER NOT LESS THAN 2.5db . OTHERWISE, CATEGORY 5 APPLIES RATHER THAN CATEGORY 6.

CATEGORY, ACCORDING TO CENTER-TO-CENTER BAR SPACING **STRUCTURAL** CONCRETE ELEMENT COVER ≥6d^p ≤3d_h $\langle 4d_h \rangle$ <6d^b BEAMS, COLUMNS, AND INNER LAYER OF WALLS OR SLABS 4 OTHER $\rightarrow d_h < 2d_h$

DEPARTMENT OF TRANSPORTATION FEDERAL AVIATION ADMINISTRATION SOUTHWEST REGION FORT WORTH, TEXAS LOW ACTIVITY LEVEL AIRPORT TRAFFIC CONTROL TOWER GENERAL NOTES ATCT/BASE-EG BUILDING

ADDISON (ADDISON AIRPORT TEXAS dann lay SISTEMS ENGINEER, ANI-640 MANAGER TERMINAL PLATFORM, ANI-640 ESIGNEDI A. RAB/N.P. DATE: 06-22-01 ISSUED BY REVIEWED: N. PAREKH AIRWAY FACILITIES

DIVISION

DATE

WHEOF TEL

NIKHIL B. PAREKH

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REV.

PARSONS DALLAS, TX

FACILITY:

ORIG. DFT.: S. RAJPREEJA

DFTG. CHECKED

REF. DWG.:

DESCRIPTION

ADS-ATCT- SOI

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