

GENERAL NOTES

- STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE PROVISIONS OF THE 2009 INTERNATIONAL BUILDING CODE.
- THE BUILDING STRUCTURE HAS BEEN DESIGNED TO RESIST THE FOLLOWING CODE PRESCRIBED LOADS:

LIVE LOADS

GARAGE.....	40 PSF
STAIRS.....	100 PSF
MECHANICAL.....	150 PSF

SNOW LOADS

GROUND SNOW LOAD, P _g	5 PSF
SNOW IMPORTANCE FACTOR, I _s	1.0
SNOW EXPOSURE FACTOR, C _e	0.9

△ THERMAL FACTOR, C _t	1.2
--	-----

WIND LOADS

BASIC WIND SPEED (THREE SECOND GUST), V _{3s}	90 MPH
WIND IMPORTANCE FACTOR, I _w	1.0
EXPOSURE CATEGORY.....	B

SEISMIC LOADS

OCCUPANCY CATEGORY.....	II
SEISMIC IMPORTANCE FACTOR, I _e	1.0
SPECTRAL RESPONSE COEFFICIENT, S _s	1.2 2 _g
SPECTRAL RESPONSE COEFFICIENT, S ₁	5.1 _g
SITE CLASS.....	C
SEISMIC DESIGN CATEGORY.....	A

- THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.
- THE STRUCTURAL DRAWINGS SHALL NOT BE SCALED FOR DETERMINATION OF QUANTITY, LENGTH OR FIT OF MATERIALS.
- PRINCIPAL OPENINGS ARE INDICATED ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND ELECTRICAL DRAWINGS FOR ALL OPENINGS. PROVIDE BLOCKOUTS, INSERTS, CURBS, OPENINGS AND SLAB DEPRESSIONS NOT SHOWN.
- CONTRACTOR SHALL COMPARE STRUCTURAL AND ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCY TO THE ARCHITECT PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS.
- CONTRACTOR SHALL INSURE THAT CONSTRUCTION MATERIALS WHOSE WEIGHT EXCEEDS THE DESIGN LIVE LOADS INDICATED ON THE STRUCTURAL DRAWINGS ARE NOT STORED ON STRUCTURALLY SUPPORTED FLOOR OR ROOF FRAMING.
- THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL WORK AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY CONDITION WHICH, IN HIS OR HER OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.
- LOADINGS FOR MECHANICAL EQUIPMENT ARE BASED ON THE UNIT(S) SHOWN ON THE STRUCTURAL DRAWINGS. ANY CHANGES IN TYPE, SIZE, WEIGHT OR NUMBER OF UNIT(S) SHALL BE REPORTED TO THE ARCHITECT PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS OR MECHANICAL EQUIPMENT.
- REPRODUCTION OF THE STRUCTURAL DRAWINGS, EITHER IN PART OR IN WHOLE, FOR SUBMITTALS OR SHOP DRAWINGS SIGNIFIES ACCEPTANCE OF INFORMATION SHOWN AS CORRECT AND OBLIGES THE USER TO ANY EXPENSE, REAL OR IMPLIED, ARISING FROM THEIR USE.
- CONTRACTOR SHALL SCHEDULE SITE OBSERVATION VISITS WITH THE ENGINEER OF RECORD AND/OR TESTING LABORATORY A MINIMUM OF FORTY-EIGHT HOURS PRIOR TO THE REQUIRED TIME OF THE VISIT.
- CONTRACTOR SHALL ALLOW TEN (10) WORKING DAYS FOR THE ENGINEER TO REVIEW EACH STRUCTURAL SUBMITTAL OR SHOP DRAWING.

FOUNDATION NOTES

- THE FOUNDATION DESIGN IS BASED ON THE PROJECT GEOTECHNICAL REPORT PREPARED BY REED ENGINEERING GROUP, INC. (PROJECT NO. 13998) DATED DECEMBER 15, 2009.
- THE FOUNDATION DESIGN IS BASED ON A POTENTIAL VERTICAL MOVEMENT, PVM, ON THE ORDER OF ONE (1) INCH OR LESS. IF THIS VALUE IS NOT ACCEPTABLE TO THE OWNER OR TENANTS, THE FOUNDATION DESIGN MUST BE REVISED.
- THE FOUNDATION SHALL CONSIST OF AUGER-EXCAVATED, STRAIGHT SHAFT REINFORCED CONCRETE PIERS. REFER TO TYPICAL PIER DETAIL FOR BEARING STRATA. PIERS HAVE BEEN PROPORTIONED FOR THE FOLLOWING:

END BEARING.....	80,000 PSF
SKIN FRICTION (COMPRESSION).....	20,000 PSF
SKIN FRICTION (TENSION).....	13,000 PSF
- ALL GRADE BEAM SIDES SHALL BE HARD FORMED, EARTH-FORMING IS NOT ACCEPTABLE.
- CORRUGATED PAPER FORMS, AS MANUFACTURED BY SUREVOID PRODUCTS INC., SHALL BE INSTALLED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER TO PROVIDE A NOMINAL SIX (6) INCH VOID BENEATH ALL GRADE BEAMS. SIX INCH THICK BY TWELVE (12) INCH HIGH PLASTIC BACKFILL RETAINER BOARDS, AS MANUFACTURED BY SUREVOID PRODUCTS, INC., SHALL BE INSTALLED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER CONTINUOUSLY ALONG EACH SIDE OF ALL GRADE BEAMS.
- THE GARAGE SLAB ON GRADE SHALL BE PLACED OVER A FOUR (4) INCH THICK LAYER OF MOIST COARSE SAND OVER UNDISTURBED NATURAL SOIL OR COMPACTED AND TESTED SELECT FILL REQUIRED TO ACHIEVE FINAL GRADES. THE BUILDING AREA SHALL BE STRIPPED OF ALL VEGETATION, TOPSOIL AND ANY OTHER DELETERIOUS MATERIALS. THE EXPOSED SUBGRADE SOILS SHALL BE PROOF-ROLLED WITH A HEAVY VEHICLE TO EVIDENCE WEAK AREAS, SOFT SPOTS IN THE SUBGRADE SHALL BE EXCAVATED TO FIRM SOIL AND REPLACED WITH SELECT FILL IN ACCORDANCE WITH THE PROJECT GEOTECHNICAL REPORT.
- INFORMATION ABOVE IS PRESENTED ONLY AS A SUMMARY OF THE PROJECT GEOTECHNICAL REPORT. THE CONTRACTOR IS RESPONSIBLE FOR REVIEWING AND COMPLYING WITH THE RECOMMENDATIONS CONTAINED IN THE PROJECT GEOTECHNICAL REPORT. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.
- BECAUSE OF THE ELAPSED TIME, THE CURRENT SOIL CONDITIONS MAY DIFFER SIGNIFICANTLY FROM THE SAMPLES THAT WERE USED IN THE DEVELOPMENT OF THE PROJECT GEOTECHNICAL REPORT REFERENCED ABOVE. THEREFORE, IT IS RECOMMENDED THAT THE BUILDING OWNER CONSULT WITH THE PROJECT GEOTECHNICAL ENGINEER TO DETERMINE IF THE FOUNDATION DESIGN PARAMETERS ARE CONSISTENT WITH THE CURRENT SOIL CONDITIONS.

STRUCTURAL CONCRETE NOTES

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 AND ACI 318. ALL CONCRETE SHALL BE LABORATORY DESIGNED AND CONTROLLED.
- CONCRETE IN THE FOLLOWING AREAS SHALL HAVE SAND AND GRAVEL OR CRUSHED STONE COARSE AGGREGATES AND CORRESPONDING TWENTY-EIGHT (28) DAY COMPRESSIVE STRENGTH AS FOLLOWS:

PIERS AND GRADE BEAMS.....	3,500 PSI
SLAB-ON-GRADE.....	3,500 PSI
- CONCRETE PROTECTION FOR STEEL REINFORCEMENT SHALL BE AS FOLLOWS (SEE ACI 318, SECTION 7.7 FOR CONDITIONS NOT INDICATED):

ALL CONCRETE PLACED AGAINST SOIL.....	3"
SLABS ON GRADE.....	2" AT MID-DEPTH
FORMED GRADE BEAMS.....	2" BOTTOM, 2" SIDES, 1 1/2" TOP
CONCRETE ON STEEL DECK.....	AT MID-DEPTH OF CONCRETE
BEAMS, JOISTS AND COLUMNS.....	1 1/2"
- LOCATE JOINTS TO LEAST IMPAIR STRENGTH AND APPEARANCE OF STRUCTURE. LOCATE HORIZONTAL JOINTS IN CONCRETE ONLY WHERE THEY NORMALLY OCCUR OR WHERE INDICATED ON PLAN. LOCATE VERTICAL JOINTS IN THE MIDDLE THIRD OF SPAN.
- ROUGHEN SURFACE OF HORIZONTAL OR NEARLY HORIZONTAL CONSTRUCTION JOINTS SO THAT AGGREGATE SHALL BE EXPOSED UNIFORMLY, LEAVING NO LAITANCE, LOOSENED PARTICLES OR DAMAGED CONCRETE.
- THE PLACEMENT OF SLEEVES OR OPENINGS THRU CONCRETE MEMBERS IS PROHIBITED UNLESS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS OR APPROVED IN WRITING BY THE ENGINEER OF RECORD.
- PROVIDE CHAMFERS AND REVEALS AS INDICATED IN THE ARCHITECTURAL DRAWINGS.

REINFORCING STEEL NOTES

- ALL DETAILING OF STEEL REINFORCEMENT AND ACCESSORIES SHALL CONFORM TO ACI COMMITTEE 315 PUBLICATION SP-86, "ACI DETAILING MANUAL."
- DEFORMED BAR REINFORCEMENT SHALL BE DOMESTIC NEW BILLET STEEL IN CONFORMANCE WITH ASTM A615, GRADE 60.
- WELDED WIRE FABRIC SHALL BE ELECTRICALLY WELDED, COLD-DRAWN WIRE IN CONFORMANCE WITH ASTM A185, GRADE 65. WELDED WIRE FABRIC SHALL BE PLACED IN FLAT SHEETS ONLY.
- LAP WELDED WIRE FABRIC AT LEAST 1 1/2 SQUARES PLUS WIRE END EXTENSIONS BUT NOT LESS THAN TWELVE (12) INCHES, UNLESS NOTED OTHERWISE. EXTEND MESH ACROSS SUPPORTING BEAMS AND WALLS.

ADHESIVE ANCHOR AND DOWEL NOTES

- WHERE NOTED IN THE PLANS AND DETAILS, ADHESIVE ANCHORS AND DOWELS SHALL BE INSTALLED WITH SIMPSON STRONG-TIE SET HIGH STRENGTH EPOXY IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER.
- ADHESIVE ANCHORS AND/OR DOWELS NOT NOTED IN THE PLANS AND DETAILS ARE NOT ALLOWED WITHOUT PRIOR WRITTEN CONSENT OF THE STRUCTURAL ENGINEER OF RECORD.
- UNLESS NOTED OTHERWISE, THE MINIMUM EMBEDMENT DEPTH OF ADHESIVE ANCHORS AND DOWELS SHALL BE AS FOLLOWS:

ANCHOR/DOWEL	EMBEDMENT
3/8" DIA. OR #3 BAR.....	4 1/2"
1/2" DIA. OR #4 BAR.....	7"
5/8" DIA. OR #5 BAR.....	9 5/8"
3/4" DIA. OR #6 BAR.....	11 1/4"
7/8" DIA. OR #7 BAR.....	13 1/8"
1" DIA. OR #8 BAR.....	15"

STRUCTURAL PRECAST CONCRETE NOTES

- ALL PRECAST MEMBERS, CONNECTIONS AND EMBEDS SHALL BE DESIGNED BY THE PRECAST MANUFACTURER FOR ALL DEAD, LIVE, WIND AND SEISMIC LOADS IN ACCORDANCE WITH THE BUILDING CODE NOTED ABOVE AND THE REQUIREMENTS EXPRESSED IN THE CONTRACT DOCUMENTS.
- THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS AND DESIGN CALCULATIONS (INCLUDING ALL PRECAST MEMBERS AND CONNECTIONS) PERFORMED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF TEXAS. SHOP DRAWINGS SHALL INCLUDE FRAMING PLANS SHOWING ALL PREFABRICATED MEMBERS WITH MARK NUMBER FOR EACH MEMBER TYPE.
- DETAILS FOR THE CONNECTION OF PRECAST MEMBERS TO CAST-IN-PLACE CONCRETE SHALL BE FURNISHED BY THE PRECAST MANUFACTURER. THE CONTRACTOR SHALL FURNISH AND COORDINATE THE LOCATION OF EMBEDS IN CAST-IN-PLACE CONCRETE REQUIRED FOR PRECAST CONNECTIONS.
- PROVIDE 1/2 INCH CHAMFER AT EXPOSED CONCRETE EDGES, UNLESS NOTED OTHERWISE.
- HOT-DIP GALVANIZE STRUCTURAL STEEL MEMBERS AND EMBEDS EXPOSED TO ELEMENTS AND WHERE INDICATED ON DRAWINGS IN ACCORDANCE WITH ASTM A123.
- TOUCH-UP FIELD WELDS ON GALVANIZED ITEMS WITH PAINT CONFORMING TO TT-P-641.

REMOVAL OF FORMWORK AND RESHORING NOTES:

- REMOVE FORMWORK AND RESHORE IN ACCORDANCE WITH ACI 301 AND RECOMMENDATIONS OF ACI 347 TO ENSURE COMPLETE SAFETY OF FORMWORK AND STRUCTURE.
- FORMWORK FOR COLUMNS, WALLS, SIDES OF BEAM AND OTHER PARTS NOT SUPPORTING WEIGHT OF CONCRETE MAY BE REMOVED AFTER CONCRETE HAS SUFFICIENTLY CURED TO RESIST DAMAGE FROM FORMWORK REMOVAL OPERATIONS.
- RETAIN FORMS AND SHORING FOR POST-TENSIONED BEAMS AND SLABS UNTIL MEMBER HAS BEEN FULLY TENSIONED AND UNTIL ALL COLUMNS ABOVE HAVE BEEN CAST AND HARDENED.
- LOCATE SHORES OR RESHORES ONLY ABOVE WHERE SHORES OR RESHORES ARE LOCATED AT LEVEL BELOW.
- NO CONSTRUCTION LOADS OR LIVE LOADS ARE PERMITTED ON FLOOR DURING STRIPPING AND UNTIL RESHORING IS COMPLETE.
- BEGIN RESHORING IMMEDIATELY AFTER FORMS AND SHORES HAVE BEEN REMOVED AND IN NO CASE LATER THAN END OF WORKING DAY ON WHICH STRIPPING OF FORMS OCCURS.
- PLACE RESHORES SNUG AGAINST SOFFIT OF FLOOR ABOVE. DO NOT TIGHTEN RESHORES AS TO PRELOAD FLOOR BELOW.
- EXTEND RESHORING OVER A SUFFICIENT NUMBER OF FLOORS TO DISTRIBUTE THE WEIGHT OF NEWLY PLACED CONCRETE AND FORMWORK AS WELL AS CONSTRUCTION LIVE LOADS SO THAT THE DESIGN LIVE LOAD OF FLOORS BELOW IS NOT EXCEEDED. AS A MINIMUM, THREE FLOORS SHALL BE UTILIZED TO SUPPORT THE LEVEL BEING PLACED.
- DO NOT REMOVE RESHORES SUPPORTING THE UPPERMOST LEVEL UNTIL CONCRETE HAS ACHIEVED ONE HUNDRED PERCENT OF THE SPECIFIED 28-DAY COMPRESSIVE STRENGTH.

STRUCTURAL STEEL NOTES

- ALL STRUCTURAL STEEL DETAILING, FABRICATION AND INSTALLATION SHALL CONFORM TO THE STANDARDS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC).
- PROVIDE NEW DOMESTIC STRUCTURAL STEEL IN ACCORDANCE WITH THE FOLLOWING:

WIDE FLANGE SHAPES.....	ASTM A992
CHANNELS, PLATES AND ANGLES.....	ASTM A36
STEEL TUBE.....	ASTM A500, GRADE B
STEEL PIPE.....	ASTM A53 (TYPES E OR S), GRADE B
- THE DETAILER SHALL DESIGN ALL CONNECTIONS TO RESIST FIFTY (50) PERCENT OF THE ALLOWABLE SHEAR CAPACITY OF THE BEAM, UNLESS NOTED OTHERWISE. AS A MINIMUM, PROVIDE THE NUMBER OF BOLTS SHOWN BELOW FOR EACH BEAM SIZE:

BEAM SIZE	NUMBER OF BOLTS
W8 & W10.....	2 MINIMUM
W12, W14, W16.....	3 MINIMUM
W18 & W21.....	4 MINIMUM
W24 & W27.....	5 MINIMUM
W30 & W36.....	6 MINIMUM
W36 & W40.....	7 MINIMUM
- CONNECTION BOLTS FOR STRUCTURAL STEEL MEMBERS SHALL BE 3/4 INCH DIAMETER ASTM A325-N BOLTS, UNLESS NOTED OTHERWISE.
- ANCHOR BOLTS SHALL BE UNFINISHED THREADED FASTENERS THAT CONFORM TO ASTM F1554, GRADE 36 BOLTS AND NUTS WITH HEXAGONAL HEADS.
- SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED EXCEPT AS SPECIFICALLY INDICATED IN STRUCTURAL DRAWINGS.
- ERECT ALL STEEL BEAMS WITH NATURAL OR SPECIFIED CAMBER UP.
- UNLESS NOTED OTHERWISE, HOT DIP GALVANIZE ALL STRUCTURAL STEEL MEMBERS AND EMBEDS EXPOSED TO WEATHER OR SOIL AND WHERE INDICATED ON DRAWINGS. GALVANIZING SHALL CONFORM TO ASTM A123.
- TOUCH UP FIELD WELDS ON GALVANIZED ITEMS WITH PAINT CONFORMING TO TT-P-641.
- ALL STAIRS, LANDINGS AND SUPPORTS SHALL BE DESIGNED BY THE STAIR MANUFACTURER. THE MINIMUM DESIGN LIVE LOAD FOR STAIRS AND ACCESSORIES SHALL BE ONE HUNDRED (100) POUNDS PER SQUARE FOOT. CONTRACTOR SHALL SUBMIT COMPLETE DESIGN CALCULATIONS AND SHOP DRAWINGS. SUBMITTALS SHALL BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF TEXAS.
- DO NOT ATTACH EXTERIOR WALL ELEMENTS TO STEEL FRAMING UNTIL ALL DECKING HAS BEEN ATTACHED TO FRAME AND STRUCTURAL BRACING IS IN PLACE (OR ADEQUATE TEMPORARY BRACING HAS BEEN INSTALLED). EXTERIOR WALL ELEMENTS ATTACHING TO STEEL FRAMING SHALL HAVE CONNECTIONS WHICH ALLOW FOR BOTH HORIZONTAL AND VERTICAL ADJUSTMENT TO COMPENSATE FOR MEMBER ROTATION AND DEFLECTION.

WELDING NOTES

- WELDING OF STRUCTURAL STEEL SHALL CONFORM TO AWS D1.1. USE E70XX ELECTRODES FOR FIELD AND SHOP WELDS. USE ONLY LOW-HYDROGEN ELECTRODES ON ASTM A242, A514, A572 AND A588 STEEL.
- WELDS NOT INDICATED IN DRAWINGS SHALL BE MINIMUM SIZE CONTINUOUS FILLET WELD IN ACCORDANCE WITH AWS D1.1. FILLET WELDS SHALL BE CONTINUOUS, UNLESS NOTED OTHERWISE.
- PROVIDE FILLET WELD AT ALL CONTACT JOINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE CAPACITY OF THE SMALLER MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
- ALL GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.
- AUTOMATICALLY END WELD HEADED STUDS AND DEFORMED BARS WHERE INDICATED ON DRAWINGS. STUDS SHALL CONFORM TO ASTM A108.

REINFORCED CONCRETE MASONRY NOTES

- REINFORCED CONCRETE MASONRY WALL CONSTRUCTION HAS BEEN DESIGNED FOR A MINIMUM COMPRESSION STRENGTH (f_m) OF 1,500 PSI. THIS VALUE SHALL BE VERIFIED IN ACCORDANCE WITH NCMR TR 759. "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY."
- CONCRETE BLOCK SHALL BE ASTM C90, GRADE N, TYPE 1, LIGHT-WEIGHT UNITS OF EIGHT (8) INCH NOMINAL THICKNESS WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,900 PSI ON THE NET AREA OF THE BLOCK.
- MORTAR SHALL BE TYPE "M" OR "S" IN ACCORDANCE WITH ASTM C270 AND SHALL HAVE A TWENTY-EIGHT (28) DAY COMPRESSIVE STRENGTH OF 2,500 PSI OR 1,800 PSI, RESPECTIVELY. AGGREGATES FOR MORTAR SHALL CONFORM TO ASTM C144.
- GROUT SHALL CONFORM TO ASTM C478 WITH A MAXIMUM AGGREGATE SIZE OF 3/8 INCH AND A 28-DAY COMPRESSIVE STRENGTH OF 2,000 PSI. AGGREGATES FOR GROUT SHALL CONFORM TO ASTM C404.
- LAP SPLICE LENGTH FOR CONTINUOUS DEFORMED BAR REINFORCEMENT IN CONCRETE MASONRY CONSTRUCTION SHALL BE AS FOLLOWS:

#3 BARS.....	19 INCHES MINIMUM
#4 BARS.....	25 INCHES MINIMUM
#5 BARS.....	31 INCHES MINIMUM
#6 BARS.....	57 INCHES MINIMUM
- ALL CELLS CONTAINING REINFORCING BARS, BOLTS OR OTHER METAL FABRICATIONS SHALL BE GROUTED SOLID. ANY CELLS AT OR BELOW FINISHED GRADE SHALL BE GROUTED SOLID.
- REINFORCED CONCRETE MASONRY CONSTRUCTION SHALL BE RUNNING BOND, UNLESS NOTED OTHERWISE.

SPECIAL INSPECTION

- PARKIN-PERKINS-OLSEN CONSULTING ENGINEERING, INC. (PPO) IS NOT THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE ACTING AS THE OWNER'S AGENT. SPECIAL INSPECTION IS NOT PART OF PPO'S CONTRACT, BUT THE FOLLOWING IS PRESENTED HERE FOR THE BENEFIT OF THE CONTRACTOR AND THE BUILDING OFFICIAL.
- THE OWNER OR REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE ACTING AS THE OWNER'S AGENT SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTION DURING CONSTRUCTION IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
- THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN SECTION 109 OF THE INTERNATIONAL BUILDING CODE.
- SPECIAL INSPECTORS SHALL MAINTAIN AND SUBMIT REPORTS IN ACCORDANCE WITH SECTION 1704.1.2 OF THE INTERNATIONAL BUILDING CODE.
- INSPECTIONS REQUIRED:

INSPECTION TASKS PER 2009 IBC	INSPECTION FREQUENCY	
	CONTINUOUS	PERIODIC
STEEL CONSTRUCTION (SECTION 1704.3 AND TABLE 1704.3)		
STEEL FABRICATION PROCESS PER 1704.2		X
MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS		X
INSPECTION OF HIGH-STRENGTH BOLTING (REFER TO SECTION 1704.3.3 FOR INSPECTION TYPE)	X	X
MATERIAL VERIFICATION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH SECTION 1708.4	-	-
MATERIAL VERIFICATION OF WELD FILLER MATERIALS SHALL BE IN ACCORDANCE WITH AISC 360, SECTION A3.5	-	-
WELDING (REFER TO 1704.3 FOR EXCEPTIONS TO CONTINUOUS INSPECTION)	X	X
STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS		X
CONCRETE CONSTRUCTION (SECTION 1704.4 AND TABLE 1704.4)		
REINFORCING STEEL PLACEMENT		X
REINFORCING STEEL WELDING	X	
BOLTS INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE	X	
VERIFICATION OF USE OF REQUIRED MIX DESIGN		X
TESTING OF FRESH CONCRETE SLUMP, AIR CONTENT AND TEMPERATURE	X	
CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	
MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X
ERECTION OF PRECAST CONCRETE MEMBERS		X
VERIFICATION OF CONCRETE STRENGTH PRIOR TO STRESSING OF TENDONS AND PRIOR TO SHORE AND FORM REMOVAL		X
FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		X
MASONRY CONSTRUCTION (SECTION 1704.5 AND TABLE 1704.5.1)		
SITE-PREPARED MORTAR		X
CONSTRUCTION OF MORTAR JOINTS		X
LOCATION OF REINFORCEMENT AND CONNECTORS		X
VERIFY SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X
VERIFY TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION		X
VERIFY SIZE, GRADE AND TYPE OF REINFORCEMENT		X
VERIFY WELDING OF REINFORCING BARS	X	
VERIFY PROTECTION OF MASONRY DURING COLD OR HOT WEATHER		X
VERIFY PRIOR TO GROUTING: GROUT SPACE IS CLEAN, PLACEMENT OF REINFORCEMENT AND CONNECTORS, PROPORTIONS OF SITE-PREPARED GROUT, AND CONSTRUCTION OF MORTAR JOINTS		X
GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENT PROVISIONS	X	
PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS SHALL BE OBSERVED	X	
COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED		X
SOILS (SECTION 1704.7)		
VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY		X
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		X
PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS		X
VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL	X	
PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY		X
PIER FOUNDATIONS (SECTION 1704.9)		
OBSERVE DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH PIER	X	
VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM PIER DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE) AND ADEQUATE END BEARING STRATA CAPACITY	X	
FOR CONCRETE PIERS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1704.4	-	-
FOR MASONRY PIERS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1704.5	-	-

REVISIONS

△ PRECAST SUBMITTAL COORDINATION.....	10/17/11
---------------------------------------	----------

KELLER SPRINGS LOFTS

LOFT APARTMENTS IN ADDISON, TEXAS

CONSTRUCTION ISSUE
10-17-2011



4144 N. Central Expy., Suite 855
Dallas, TX 75204
214.520.8878
bgoarchitects.com

DATE

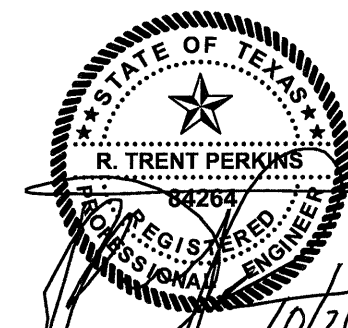
08-05-2011

PROJECT

11129

SHEET NUMBER

SG1.01



THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY
R. TRENT PERKINS, P.E. 84284

THIS DOCUMENT IS THE RENDERING OF A PROFESSIONAL SERVICE, THE ESSENCE OF WHICH IS THE PROVIDING OF ADVICE, JUDGEMENT, OPINION, OR SIMILAR PROFESSIONAL SKILL.



PARKIN - PERKINS - OLSEN
CONSULTING ENGINEERING, INC.
9330 LBJ Freeway Suite 1055
Dallas, Texas 75243
Tel 214.221.2220 Fax 214.221.2252
Project No. 39155
Registration No. F-1479