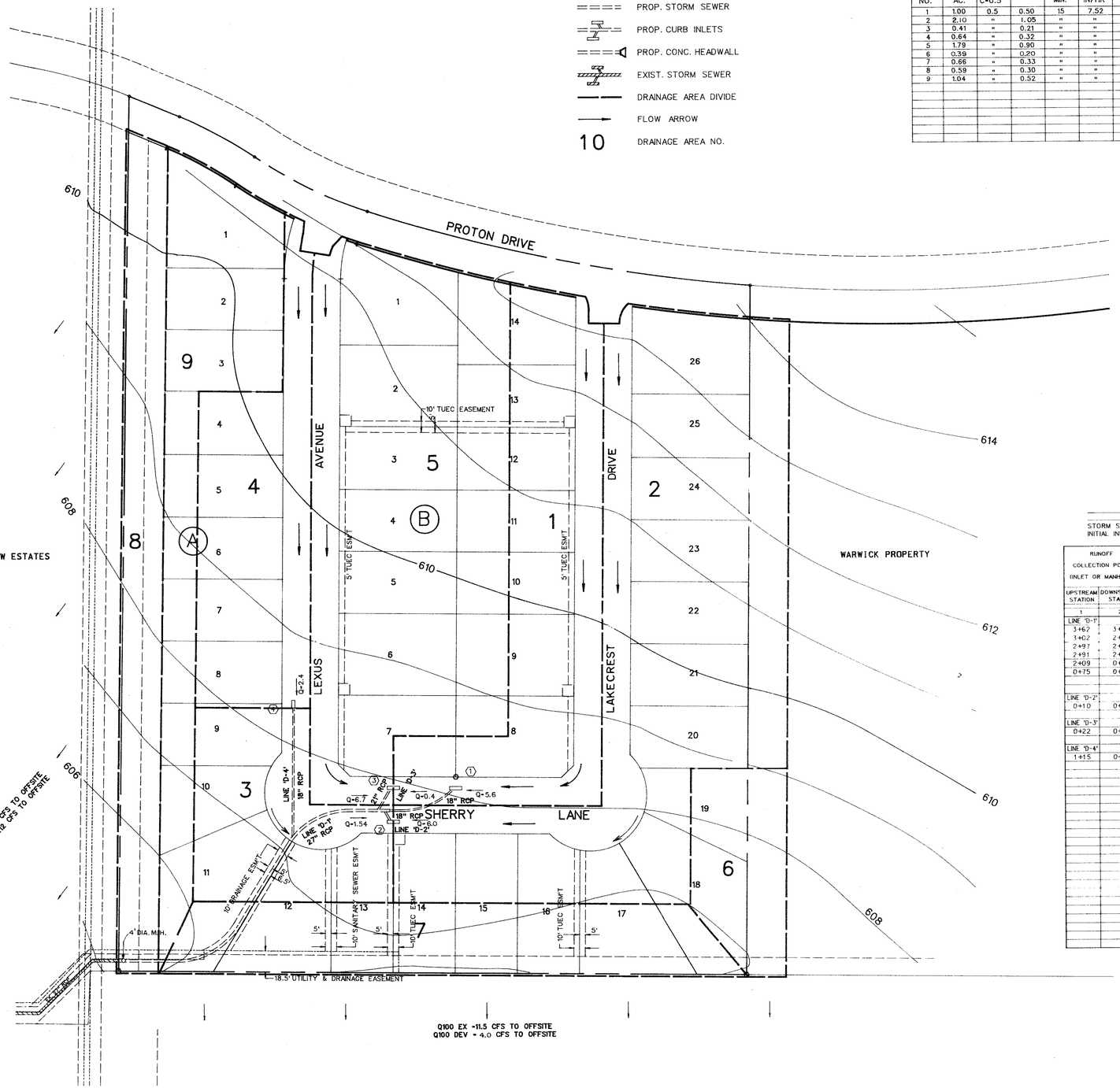
LEGEND

- PROP. STORM SEWER
- PROP. CURB INLETS
- PROP. CONC. HEADWALL
- EXIST. STORM SEWER
- DRAINAGE AREA DIVIDE
- FLOW ARROW
- 10 DRAINAGE AREA NO.



RUNOFF COMPUTATIONS

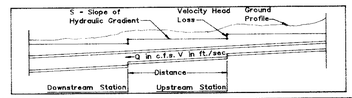
DRAIN. AREA NO.	AC.	RES. C-0.5	TOTAL C.A.	T_C MIN.	I_{100} IN/HR	Q_{100} CFS	REMARKS
		ACRES DRAINED					
1	1.00	0.5	0.50	15	7.52	3.76	TO INLET NO. 1
2	2.10	"	1.05	"	"	7.90	TO INLET NO. 2
3	0.41	"	0.21	"	"	1.54	TO INLET NO. 2
4	0.64	"	0.32	"	"	2.40	TO INLET NO. 4
5	1.79	"	0.90	"	"	6.70	TO INLET NO. 3
6	0.39	"	0.20	"	"	1.47	TO OFFSITE
7	0.66	"	0.33	"	"	2.50	TO OFFSITE
8	0.59	"	0.30	"	"	2.22	TO OFFSITE
9	1.04	"	0.52	"	"	3.91	TO OFFSITE

N

0 25 50 100
SCALE: 1" = 50'

INLET COMPUTATIONS

INLET No	Station	D.A. No	Qa cfs	Carry-Over cfs	Total Qa cfs	z	z/n	s ft/ft	y ft	P ft	a ft	qi cfs	Lr ft	La ft	La/Lr	a/y	Qi/Qa Curb	Qi cfs	Qi-Qa cfs	Carry Over cfs	Gutter Capacity cfs	Remarks
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
1	1+29	1&2	5.6	0.00	5.60	26	1667	0.0075	0.37	9.54	0.33	0.661	8.47	10	1.18	0.91	1.88	6.04	0.00		5.8	
2	0+74	2&3	7.54	0.00	6.94	26			Pmax=	13.00	0.33	2.350	3.21	10	3.12							LOW POINT
3	0+74	1&5	7.10	0.00	7.10	26			Pmax=	13.00	0.33	2.350	3.02	10	3.31							LOW POINT
4	0+86	4	2.40	0.00	2.40	26	1667	0.0120	0.24	6.36	0.33	0.543	4.42	6	1.36	1.36	1.18	2.83	0.00		6.9	



STORM SEWER CALCULATIONS

BY _____
DATE _____

STORM SEWER LINE _____
INITIAL INLET TIME _____ MINUTES

RUNOFF COLLECTION POINT (INLET OR MANHOLE) UPSTREAM STATION	DOWNSTREAM STATION	Distance Between Collection Points	INCREMENTAL DRAINAGE AREA Area No.	Drainage Area (Acres)	Runoff Coeff. "C"	Incremental "CA"	Accumulated "CA"	Time at Upstream Station (minutes)	Design Storm Frequency (yrs)	Intensity "I" (inches/hr.)	Storm Water Runoff "Q" (c.f.s.)	Slope of Hydraulic Gradient "S" (Z.)	Selected Storm Sewer Size	Velocity in Sewer Between Collection Points "V" (f.p.s.)	Head Loss Coeff. Kj	Velocity Head Loss at Upstream Station $Kj\frac{V^2-V^4}{2g}$ (feet)	Flow Time in Sewer Distance V x 60 (minutes)	Time at Downstream Station (minutes)	Remarks
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
LINE 'D-1'																			
3+62	3+02	60	1&2	1.49	0.5	0.74	0.74	15.00	100	7.52	5.6	0.242	18"	4.9	1.25	0.47	0.21	15.21	
3+02	2+97	5	1&2	1.49			0.74	15.21	100	7.52	5.6	0.028	27"	4.9	0.50	0.10	0.02	15.23	
2+97	2+91	6	2&3	2.00	0.5	1.00	1.74	15.23	100	7.52	13.1	0.153	27"	6.1	1.00	0.21	0.02	15.24	
2+91	2+09	82	1,2,5	1.70	0.5	0.95	2.69	15.24	100	7.52	20.2	0.364	27"	6.7	1.00	0.12	0.21	15.45	
2+09	0+75	134	1-5	0.64	0.5	0.32	3.01	15.45	100	7.52	22.6	0.455	27"	6.8	1.00	0.10	0.33	15.78	
0+75	0+00	75	1-5	0.64			3.01	15.78	100	7.52	22.6	0.455	27"	9.6	1.00	0.73	0.13	15.91	
LINE 'D-2'																			
0+10	0+00	10	2&3	1.84	0.5	0.92	0.92	15.00	100	7.52	6.9	0.372	18"	6.0	1.25	0.71	0.03	15.03	
LINE 'D-3'																			
0+22	0+00	22	1&5	1.89	0.5	0.94	0.94	15.00	100	7.52	7.1	0.171	21"	6.1	1.25	0.72	0.06	15.06	
LINE 'D-4'																			
1+15	0+00	115	4	0.64	0.5	0.32	0.32	15.00	100	7.52	2.4	0.045	18"	4.9	1.25	0.47	0.39	15.39	

AS-BUILT (FEB. 1992)

STATE OF TEXAS
JAMES E. HARRINGTON
69239
REGISTERED PROFESSIONAL ENGINEER
James E. Harrington
10-17-91

The seal appearing on this document was authorized by James E. Harrington, P.E. 69239, on October 17, 1991

CORWIN ENGINEERING, INC.
3023 Routh Street Dallas, Texas 75201 (214) 220-9136

DEVELOPMENT PLANS FOR
LES LACS I
ADDISON, TEXAS

DRAINAGE AREA MAP

DRAWN BY	DESIGNED BY	CHECKED BY	SHEET NO.
JEH	CEI	WLC	3 OF 19
JOB NUMBER 9120	DATE OCTOBER 1991	SCALE: 1"= 50'	

FILE: 9120PS2.DGN VI-MAP
REF FILE: 9120AL.DGN
VIEW: WS
DATE: 10-17-91