

GRADING NOTES

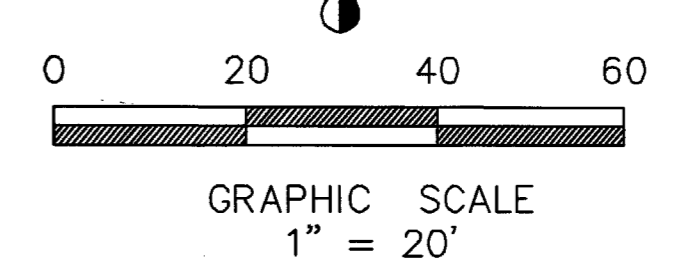
1. Prior to commencing construction activities, the Contractor shall consult with the Engineer and/or the Developer to determine if any portions of the site are to be left undisturbed. The Contractor shall be entirely responsible for the protection of existing structures, trees, vegetation, and other items designated to remain. The Contractor shall comply with all provisions of the tree mitigation plan(s) or local tree preservation requirements, where applicable. The Contractor shall bear the entire cost of restoring, replacing, or otherwise compensating the Developer for any protected facilities or vegetation that are damaged or destroyed during earthwork or clearing operations.
2. All earthwork under buildings or pavement shall be free of organic materials, including stumps, roots, and other vegetation. The Contractor shall be responsible for stripping organic material from the ground surface and disposing of it as specified in Note #9 in the General Notes, Sheet C1.0.
3. Landscaped areas shall receive 6" of topsoil unless otherwise noted elsewhere in the landscape or civil engineering plans. Stripped topsoil shall be stockpiled onsite at a location approved by the Developer. At the conclusion of construction, excess topsoil shall be considered waste material, and the Contractor is responsible for disposing of it as specified in Note #9 in the General Notes, Sheet C1.0.
4. Earthwork shall be inspected and tested on a continuing basis by the Geotechnical Engineer or an independent testing firm. The Contractor shall follow the Geotechnical Engineer's recommendations, and shall obtain approval before placement of fill containing significant numbers of rocks in excess of 4" in diameter.
5. All fill to be placed under structures or pavement shall consist of onsite soils compacted per Note #18, Sheet C1.0 and/or the geotechnical report.
6. All subgrade shall be proof-rolled prior to the placement of paving.
7. Final paving, curb, sidewalk and building pad elevations will be placed with an elevation tolerance of plus or minus 0.03 feet. Grades in landscaped areas will be placed with an elevation tolerance of plus or minus 0.10 feet.
8. Grades in paved areas are top of finished pavement unless noted. Grades along curb lines are to base of curb (gutter) unless denoted with "TC" to signify Top of Curb. All curbs are 0.50' (6") high unless noted otherwise on the plans. Finished floor elevations are to top of floor including thickness of flooring materials; consult architectural and/or structural plans for depth of excavation under building(s).
9. Landscaped areas designated as "Grade to Drain" shall have an absolute minimum slope of 1.5% unless otherwise indicated on the plans, and shall be sloped at 2% or greater whenever possible. Unless otherwise indicated, maximum slope of landscaped areas shall be 3:1 (H:V).
10. Unless otherwise indicated, grading of areas subject to pedestrian access shall comply with the latest revision of the Texas Accessibility Standards (T.A.S.) enacted by the Texas Department of Licensing and Regulation.
11. Ramps in excess of 0.50' (6") vertical rise require handrails per T.A.S. regulations. Handrails are shown on these plans are for schematic reference only. Refer to architectural plans for contractor details and exact locations of handrails on ramps and stairs, where applicable.
12. All areas not covered by buildings, pavement or other erosion-resistant surfaces shall be stabilized in compliance with the SWPPP and/or landscape plan, where applicable.
13. Refer to street paving plans for proposed grades within proposed right-of-way.
14. Units of length are feet unless otherwise marked as inches ("), centimeters (cm), meters (m) or yards (yd).
15. Refer to Sheet C1.0 for other general notes applicable to grading.
16. TREE MITIGATION PLAN FOR TREE REMOVAL & REPLACEMENT REQUIRED.

BENCHMARKS

Temporary Bench Mark: "□" cut on northwest corner of Y inlet at the northeast corner of property Elev = 639.63'
 Bench Mark: "□" cut on inlet sw corner Soujrm & Addison Rd. Elev = 641.95'

LEGEND

- PROPOSED CONCRETE PAVEMENT
- PROPOSED CONCRETE SIDEWALKS
- ▨ EXISTING PAVEMENT TO BE REMOVED
- EXISTING CONCRETE PAVERS
- PROPOSED CONCRETE PAVERS

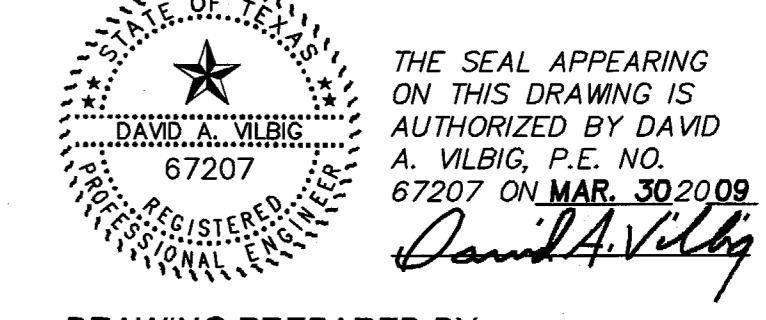


APPROVED FOR CONSTRUCTION
 Town of Addison
 Public Works Department
 APPROVED BY: *LARRY J. JENSEN*
 DATE: 9-30-2009

All responsibility for the adequacy of these plans remains with the Engineer who prepared them. In approving these plans, the Town of Addison makes no representation of adequacy of the work of the Design Engineer.

NOTE TO GENERAL CONTRACTOR:
 Grading in driveway east of proposed building based on 2007 VAI as-built plans. Grading in this area has not been field-verified. G.C. to notify Engineer immediately if grades differ substantially from what is shown on this plan.

DRAWING PREPARED BY:
VILBIG & ASSOCIATES, INC.
 CONSULTING ENGINEERS & SURVEYORS
 10132 MONROE DRIVE DALLAS, TEXAS 75229
 (214) 352-7333 fax (214) 352-0999
 TEXAS ENGINEERING FIRM # F-5614



**TRINITY CHRISTIAN ACADEMY
 NEW PERFORMING ARTS BLDG.
 BUILDING CONSTRUCTION PLANS
 17001 ADDISON RD., ADDISON, TX 75001**

GRADING PLAN

Project: AS-081

C2.0

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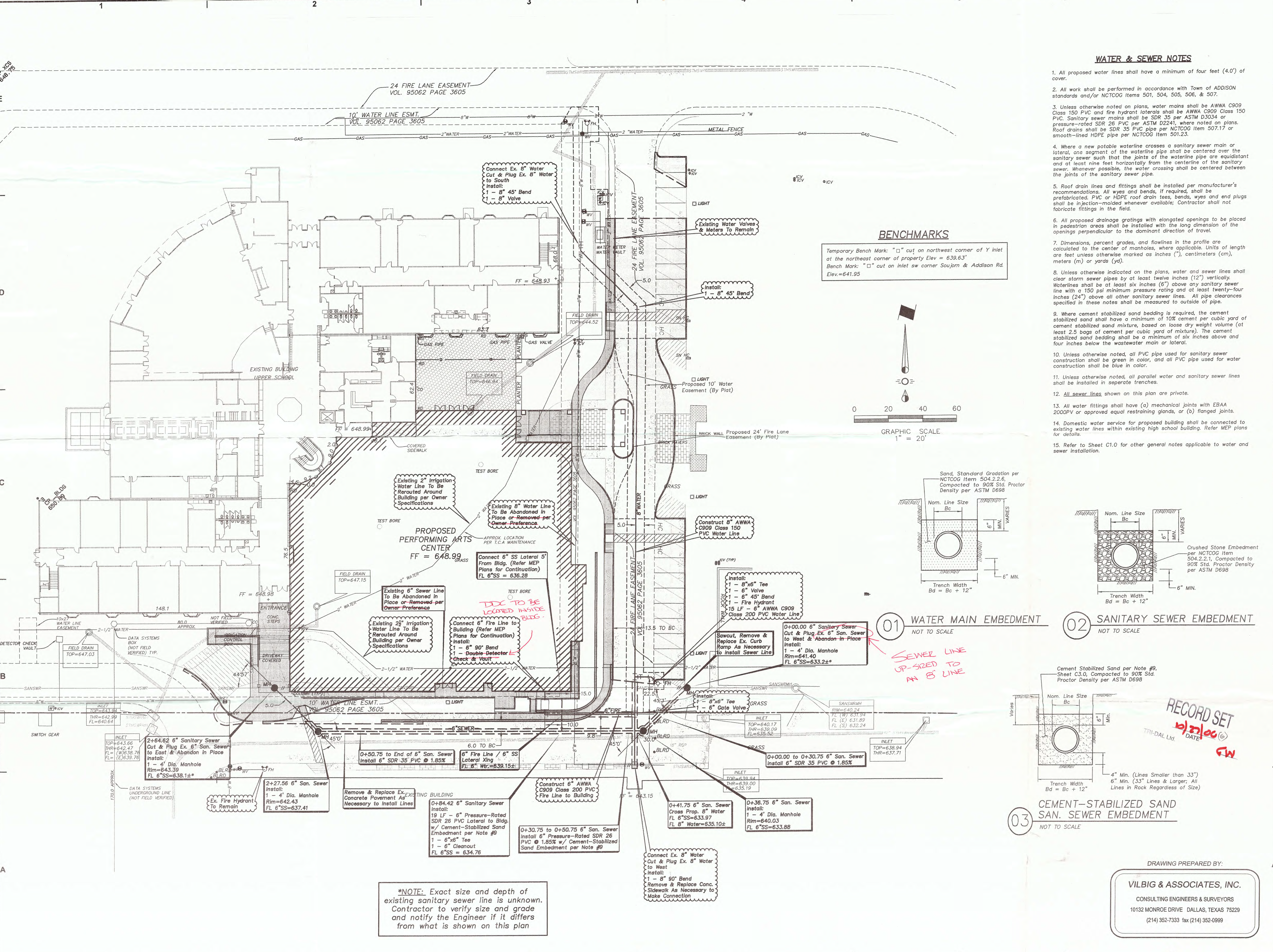
Rev	Date	Description
1	June 19, 2009	Issued for Construction
2	May 18, 2009	Arch'd grading notes added
3	May 24, 2009	Final review
4	Sept. 14, 2009	Rebuilding walk reviewed

Original Issue Date: March 30, 2009
 Engineer: David A. Vilbig, P.E.
 Project Manager: Chris Wilton, P.E.
 Designer: CWV

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 Building Grids & Way
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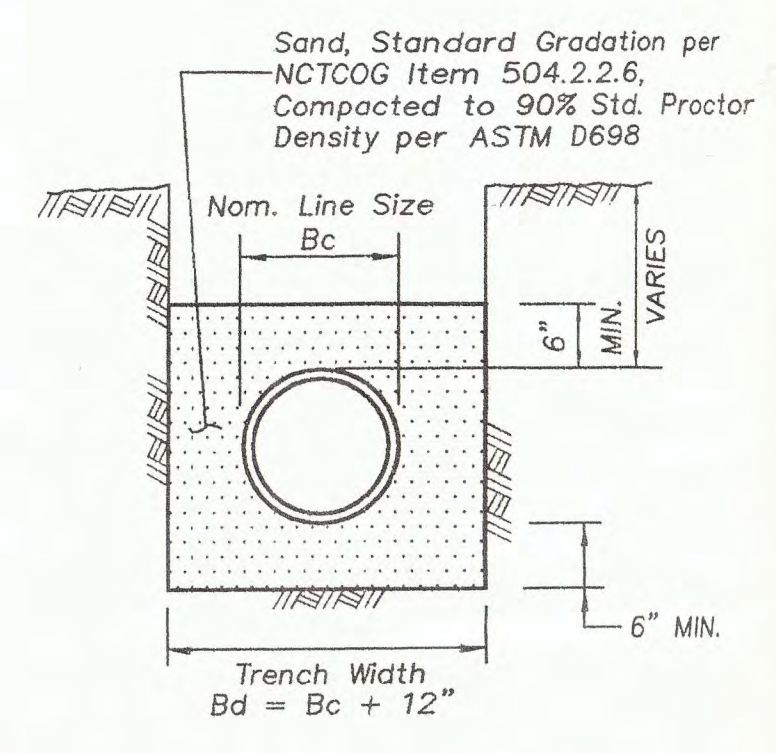
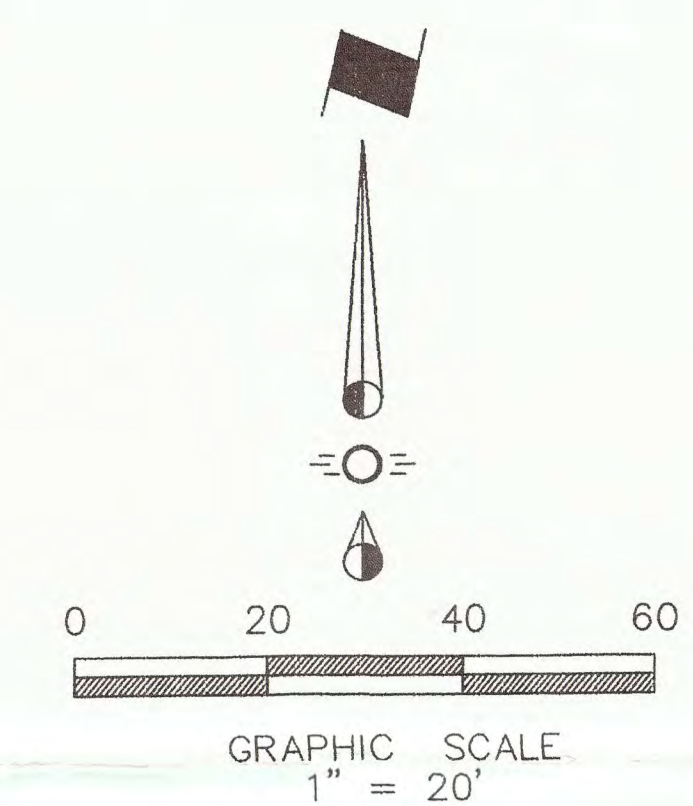


WATER & SEWER NOTES

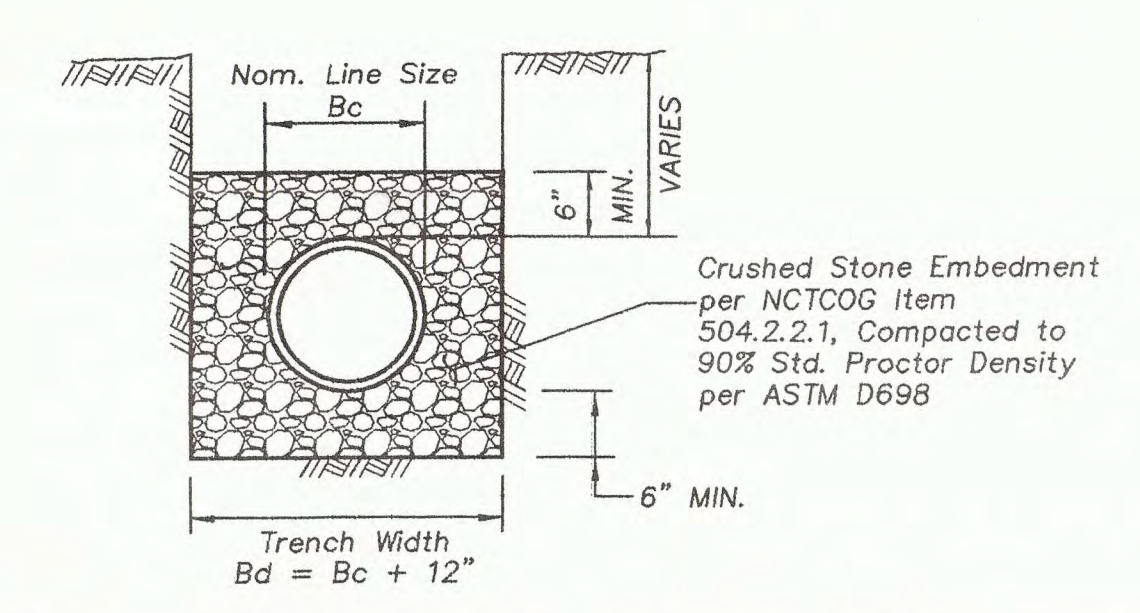
1. All proposed water lines shall have a minimum of four feet (4.0') of cover.
2. All work shall be performed in accordance with Town of ADDISON standards and/or NCTCOG Items 501, 504, 505, 506, & 507.
3. Unless otherwise noted on plans, water mains shall be AWWA C909 Class 150 PVC and fire hydrant laterals shall be AWWA C909 Class 150 PVC. Sanitary sewer mains shall be SDR 35 per ASTM D3034 or pressure-rated SDR 26 PVC per ASTM D2241, where noted on plans. Roof drains shall be SDR 35 PVC pipe per NCTCOG Item 507.17 or smooth-lined HDPE pipe per NCTCOG Item 501.23.
4. Where a new potable waterline crosses a sanitary sewer main or lateral, one segment of the waterline pipe shall be centered over the sanitary sewer such that the joints of the waterline pipe are equidistant and at least nine feet horizontally from the centerline of the sanitary sewer. Whenever possible, the water crossing shall be centered between the joints of the sanitary sewer pipe.
5. Roof drain lines and fittings shall be installed per manufacturer's recommendations. All wyes and bends, if required, shall be prefabricated. PVC or HDPE roof drain tees, bends, wyes and end plugs shall be injection-molded whenever available; Contractor shall not fabricate fittings in the field.
6. All proposed drainage gratings with elongated openings to be placed in pedestrian areas shall be installed with the long dimension of the openings perpendicular to the dominant direction of travel.
7. Dimensions, percent grades, and flowlines in the profile are calculated to the center of manholes, where applicable. Units of length are feet unless otherwise marked as inches ("), centimeters (cm), meters (m) or yards (yd).
8. Unless otherwise indicated on the plans, water and sewer lines shall clear storm sewer pipes by at least twelve inches (12") vertically. Waterlines shall be at least six inches (6") above any sanitary sewer line with a 150 psi minimum pressure rating and at least twenty-four inches (24") above all other sanitary sewer lines. All pipe clearances specified in these notes shall be measured to outside of pipe.
9. Where cement stabilized sand bedding is required, the cement stabilized sand shall have a minimum of 10% cement per cubic yard of cement stabilized sand mixture, based on loose dry weight volume (at least 2.5 bags of cement per cubic yard of mixture). The cement stabilized sand bedding shall be a minimum of six inches above and four inches below the wastewater main or lateral.
10. Unless otherwise noted, all PVC pipe used for sanitary sewer construction shall be green in color, and all PVC pipe used for water construction shall be blue in color.
11. Unless otherwise noted, all parallel water and sanitary sewer lines shall be installed in separate trenches.
12. All sewer lines shown on this plan are private.
13. All water fittings shall have (a) mechanical joints with EBAA 2000PV or approved equal restraining glands, or (b) flanged joints.
14. Domestic water service for proposed building shall be connected to existing water lines within existing high school building. Refer MEP plans for details.
15. Refer to Sheet C1.0 for other general notes applicable to water and sewer installation.

BENCHMARKS

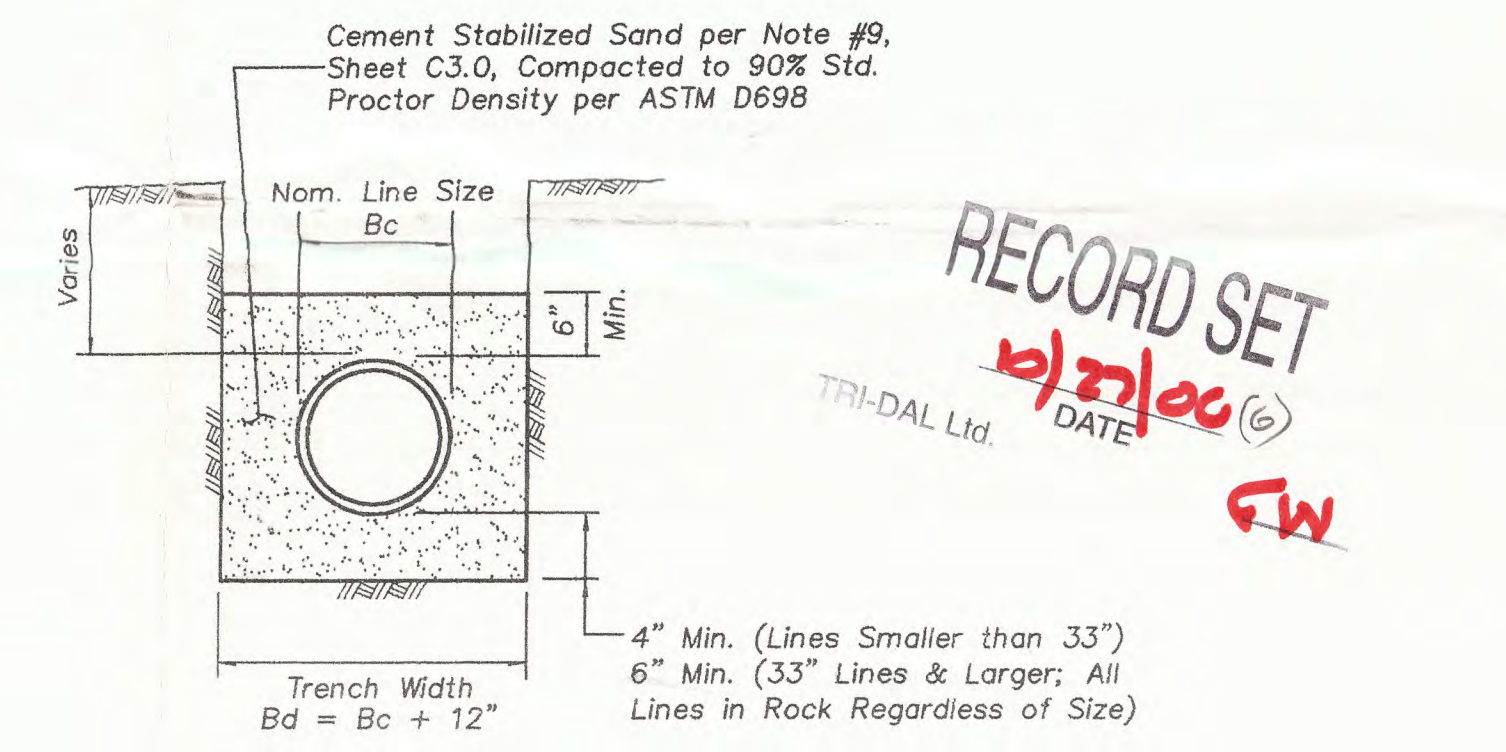
Temporary Bench Mark: "□" cut on northwest corner of Y inlet at the northeast corner of property Elev = 639.63"
 Bench Mark: "□" cut on inlet sw corner Soujorn & Addison Rd. Elev.=641.95



01 WATER MAIN EMBEDMENT NOT TO SCALE



02 SANITARY SEWER EMBEDMENT NOT TO SCALE



03 CEMENT-STABILIZED SAND SAN. SEWER EMBEDMENT NOT TO SCALE

***NOTE:** Exact size and depth of existing sanitary sewer line is unknown. Contractor to verify size and grade and notify the Engineer if it differs from what is shown on this plan

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 [Seal of the State of Texas, David A. Vilbig, P.E., 67207]

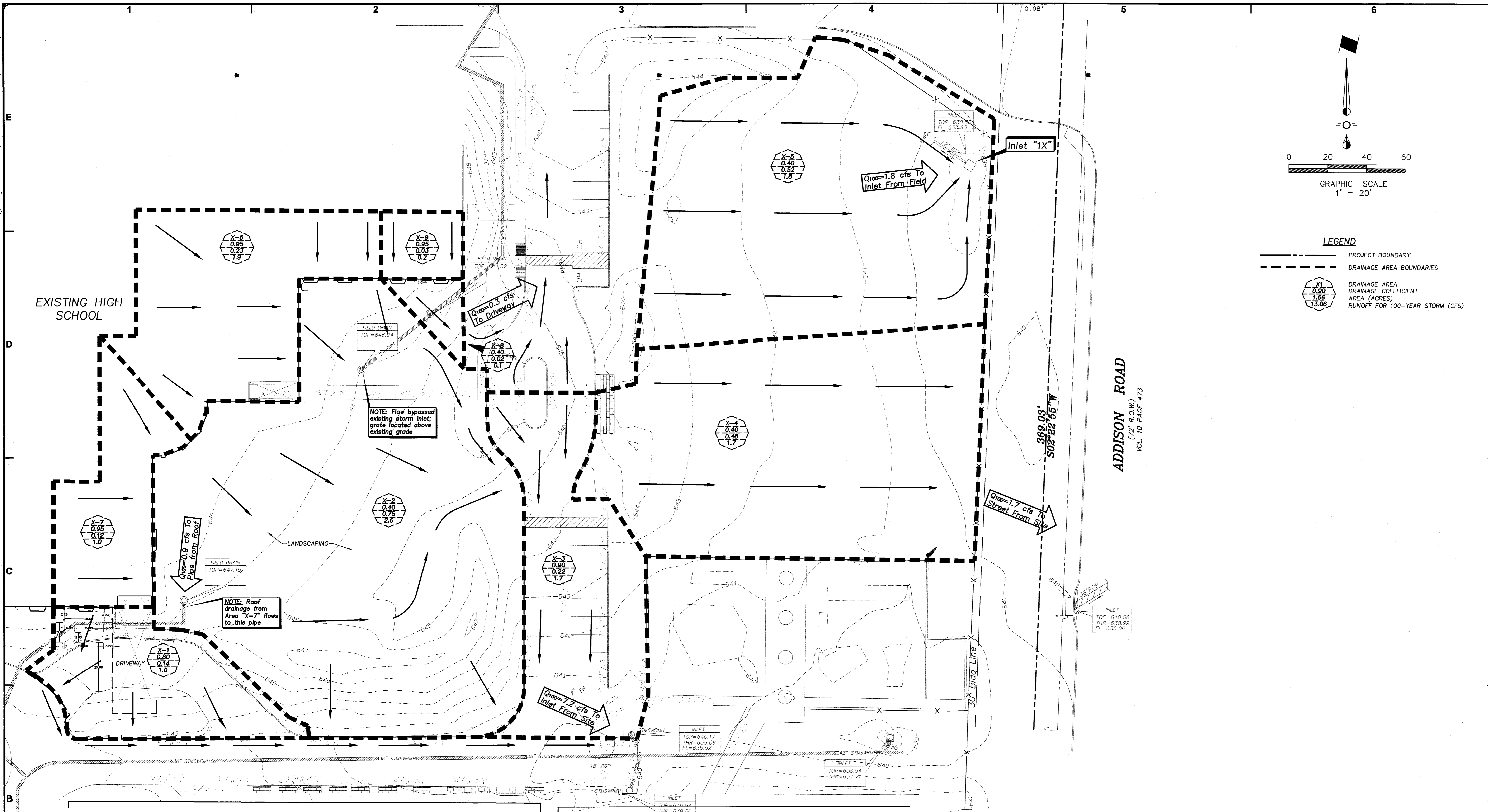
**TRINITY CHRISTIAN ACADEMY
 NEW PERFORMING ARTS BLDG.
 EARLY SITE & FOUNDATION PACKAGE
 17001 ADDISON RD., ADDISON, TX 75001**

WATER & SEWER PLAN

Project : AS-081
C3.0
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Sheet No. 2009-2-080



LEGEND

- PROJECT BOUNDARY
- DRAINAGE AREA BOUNDARIES
- DRAINAGE AREA
- DRAINAGE COEFFICIENT
- AREA (ACRES)
- RUNOFF FOR 100-YEAR STORM (CFS)

Graphic Scale: 1" = 20'

01 EXISTING DRAINAGE AREA MAP
SCALE: 1" = 20'

TABLE 1: Existing Drainage Areas

Area No.	Area ac.	"C"	C*A	Tc min	I100 in/hr	Q100 cfs	Remarks
X-1	0.14	0.80	0.11	10	8.74	1.0	Existing Driveway & Entry
X-2	0.75	0.40	0.30	10	8.74	2.8	Existing Landscaping
X-3	0.22	0.90	0.20	10	8.74	1.9	Existing Drive & Sidewalks
X-4	0.48	0.40	0.19	10	8.74	1.7	To Addison Road
X-5	0.52	0.40	0.21	10	8.74	1.8	To Existing Inlet "1X"
X-6	0.23	0.95	0.22	10	8.74	1.9	Existing Roof to Area "X-2"
X-7	0.12	0.95	0.11	10	8.74	1.0	Existing Roof to Ex. Drain Line
X-8	0.02	0.40	0.01	10	8.74	0.1	Existing Landscaping to East/North
X-9	0.03	0.95	0.03	10	8.74	0.2	Existing Roof to Area "X-8"
TOTAL FLOW	2.51	0.55	1.38			12.1	

THE SEAL APPEARING ON THIS DRAWING IS AUTHORIZED BY DAVID A. VILBIG, P.E. NO. 67207 ON MAR. 30, 2009

David A. Vilbig

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Project Name	Date	Description
March 30, 2009	June 19, 2009	Issued for Construction
David A. Vilbig, P.E.		Engineer
Chris Walton, P.E.		Project Manager
CMW		Drawings

**TRINITY CHRISTIAN ACADEMY
NEW PERFORMING ARTS BLDG.
BUILDING CONSTRUCTION PLANS
17001 ADDISON RD., ADDISON, TX 75001**

EXISTING DRAINAGE AREA MAP

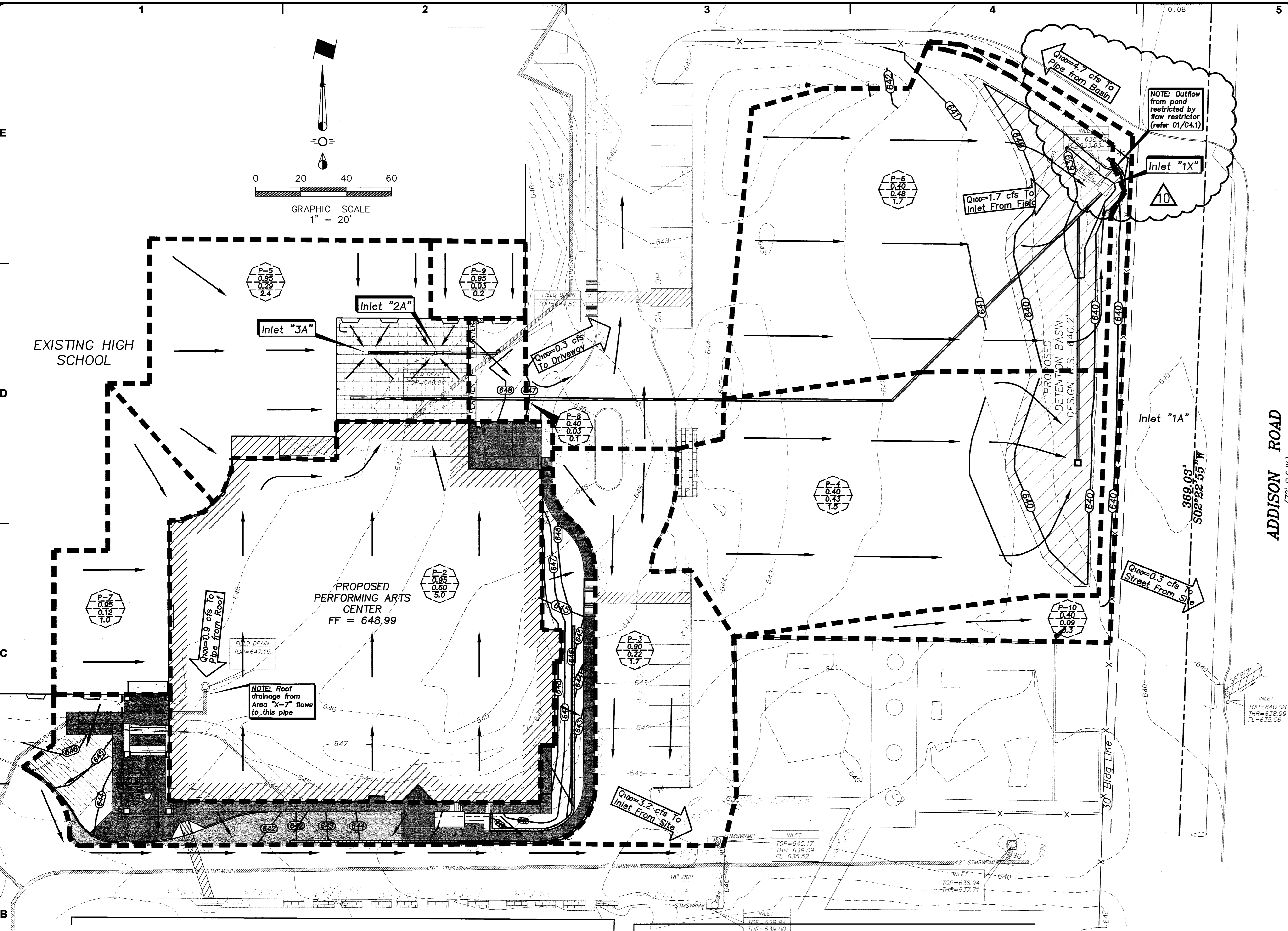
Project: AS-081

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LEGEND
 --- PROJECT BOUNDARY
 - - - DRAINAGE AREA BOUNDARIES
 X1 0.80 1.66 13.06
 DRAINAGE AREA (ACRES)
 DRAINAGE COEFFICIENT
 RUNOFF FOR 100-YEAR STORM (CFS)

TABLE 4: Detention Basin Calculations - Dallas Method
100-Year Storm

Settings:
 Max Outflow, cfs: 4.7 (From Table 2 Outflow Calculations)
 Time of Concentration: 10.0
 CFA to Pond, proposed: 0.93 (Areas P-2, P-4, & P-6)

Storm Duration (min)	I100 (in/hr)	CFA	Inflow (cfs)	Outflow (cfs)	Storage Required (cfs)	Storage (cy)
10	8.74	0.93	4898	2827	2071	77
20	6.80	0.93	7821	4240	3382	125
30	5.75	0.93	9667	5653	4014	149
40	5.00	0.93	11208	7066	4142	153
50	4.45	0.93	12469	8480	3989	148
60	4.00	0.93	13450	9893	3557	132
70	3.63	0.93	14240	11306	2934	109

Volume Provided:
 152 cy @ Design Water Surface of 640.25'

TABLE 5: Baumertner-Morris Synthetic Inflow Hydrograph
3 Hour Storm - Modified Rational Method

Constants:
 Time of Conc. (to min): 10 (From Table 2)
 Storm Length (min): 180 (Suggested by Authors of Method)
 # of Increments (min): 18 (Length divided by Tc)
 Suggested Peak (min): 67.5 (3/8 of Storm Length)
 Actual Peak (min): 70 (Nearest Length Increment)

Time (min)	Intensity from Manual (in/hr)	Inches of Rainfall (in)	Incremental Intensity (in/hr)	Rainfall Intensity (in/hr)	CFA to Basin (From Table 2)	Total Runoff Rate (cfs)	Order	Calculated Time Location (min)
10	8.74	1.46	1.46	8.74	0.93	8.16	1	70
20	6.82	2.27	0.82	4.90	0.93	4.58	2	80
30	5.75	2.88	0.60	3.61	0.93	3.37	3	80
40			0.41	2.46	0.93	2.30	4	90
50			0.33	1.98	0.93	1.85	5	90
60	3.97	3.91	0.29	1.74	0.93	1.63	6	100
70			0.26	1.66	0.93	1.46	7	40
80			0.22	1.32	0.93	1.23	8	110
90	3.07	4.61	0.22	1.32	0.93	1.23	9	30
100			0.22	1.32	0.93	1.23	10	120
110			0.21	1.26	0.93	1.18	11	20
120	2.62	5.24	0.20	1.20	0.93	1.12	12	130
130			0.18	1.08	0.93	1.01	13	10
140			0.15	0.90	0.93	0.84	14	140
150			0.09	0.54	0.93	0.50	15	150
160			0.04	0.24	0.93	0.22	16	160
170			0.02	0.12	0.93	0.11	17	170
180	1.91	5.73	0.01	0.06	0.93	0.06	18	180

01 EXISTING DRAINAGE AREA MAP
 SCALE: 1" = 20'

TABLE 2: Proposed Drainage Areas

Area No.	Area ac.	"C"	C*A	Tc min	I100 in/hr	Q100 cfs	Remarks
P-1	0.22	0.80	0.18	10	8.74	1.5	Proposed Sidewalks & Landscaping
P-2	0.60	0.95	0.57	10	8.74	5.0	Proposed Building to Line "A"
P-3	0.22	0.90	0.20	10	8.74	1.9	Existing Drive & Sidewalks
P-4	0.43	0.40	0.17	10	8.74	3.1	Surface Flow To Inlet "1A"
P-5	0.29	0.95	0.28	10	8.74	2.5	Patio & Roof to Inlets "1B" & "2B"
P-6	0.48	0.40	0.19	10	8.74	4.2	To Existing Inlet "1X"
P-7	0.12	0.95	0.11	10	8.74	1.0	Existing Roof to Ex. Drain Line
P-8	0.03	0.40	0.01	10	8.74	0.3	Proposed Landscaping to East/North
P-9	0.03	0.95	0.03	10	8.74	0.3	Existing Roof to Area "P-8"
P-10	0.09	0.40	0.04	10	8.74	0.8	To Addison Road
TOTAL FLOW	2.51	0.71	1.77			15.2	

Outflow Calculations
 Max Outflow from site, total, cfs: 12.1 (Table 1)
 Flow Bypassing Prop. Basin, cfs: 7.3 (Areas P-1, P-3, P-5, & P-7 thru P-10)
 Max Outflow from Basin, cfs: 4.7

TABLE 3: Stage-Storage-Discharge Table

Constants:
 Contraction coefficient (K): 0.81
 Elevation of outlet pipe (ft): 636.8
 Diameter of outlet restrictor (ft): 0.75

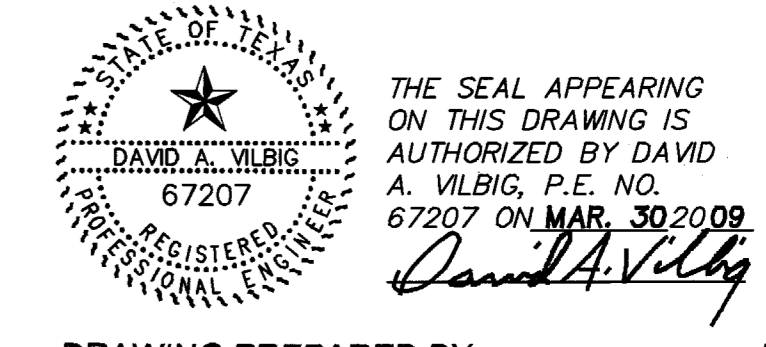
1	2	3	4	5
Elevation (ft)	Storage (cy)	Incremental Storage (cy)	Discharge (cfs)	Elevation (ft)
637.0	0.0	0.0	2.0	637.0
638.0	1.5	1.5	3.0	638.0
639.0	3.0	1.5	4.0	639.0
639.6	2.4	0.4	3.6	639.6
638.7	2.7	0.3	3.7	638.7
639.8	3.3	0.6	3.7	639.8
639.9	4.3	1.0	3.8	639.9
639.0	5.8	1.5	3.9	639.0
639.1	7.9	2.1	3.9	639.1
639.2	10.6	2.7	4.0	639.2
639.3	14.5	3.9	4.0	639.3
639.4	19.8	5.3	4.1	639.4
639.5	26.9	7.1	4.2	639.5
639.6	36.1	9.2	4.2	639.6
639.7	47.6	11.5	4.3	639.7
639.8	61.8	14.2	4.3	639.8
639.9	78.3	16.5	4.4	639.9
640.0	96.8	18.5	4.4	640.0
640.1	117.3	20.5	4.5	640.1
640.2	139.6	22.3	4.5	640.2
640.3	163.2	27.6	4.6	640.3
640.4	189.7	26.5	4.6	640.4
640.5	217.6	27.9	4.7	640.5
640.6	247.2	29.6	4.7	640.6
640.7	287.7	31.5	4.8	640.7

TABLE 6: Pond Flow Calculations - Stage-Discharge

Outflow Restriction Safety Factor: 0.80 (unitless)

1	2	3	4	5	6	7	8	9	10	11	12
Time, minutes	Runoff to Basin (cfs) from Table 5	Incremental Inflow (cy)	Pond Elevation (ft), start of step	Pond Volume (cy), start of step, from Table 3	Outflow Rate @ Column 5 Elevation (cfs), from Table 3	Safety Factored Outflow Rate, cfs	Incremental Outflow @ Column 7 Flow Rate (cy)	Incremental Volume (cy) (Column 8 - Column 9)	Cumulative Storage Volume (cy) (Column 5 + Column 9)	Pond Elevation, feet	Notes
10	1.01	22.4	637.0	0.0	2.0	1.6	35.6	-13.1	0.0	637.0	
20	1.18	26.2	637.0	0.0	2.0	1.6	35.6	-9.3	0.0	637.0	
30	1.23	27.3	637.0	0.0	2.0	1.6	35.6	-8.2	0.0	637.0	
40	1.46	32.4	637.0	0.0	2.0	1.6	35.6	-3.2	0.0	637.0	a
50	1.85	41.1	637.0	0.0	2.0	1.6	39.0	2.1	2.1	638.3	a
60	3.37	74.9	638.3	2.0	4.0	3.2	71.1	3.8	5.8	639.9	
70	8.16	181.3	638.9	4.3	3.8	3.0	67.7	113.6	117.9	640.1	b
80	4.58	101.8	640.1	117.3	4.5	3.6	79.7	22.1	139.4	640.1	b
90	2.30	51.1	640.1	117.3	4.5	3.6	79.7	-26.6	88.7	639.9	
100	1.63	36.2	639.9	78.3	4.4	3.5	77.8	-41.6	36.7	639.6	
110	1.23	27.3	639.6	36.1	4.2	3.4	74.9	-47.6	0.0	637.0	
120	1.23	27.3	637.0	0.0	2.0	1.6	35.6	-8.2	0.0	637.0	
130	1.12	24.9	637.0	0.0	2.0	1.6	35.6	-10.7	0.0	637.0	c

Notes:
 a Incremental outflow manually adjusted to provide smooth and even increase in storage curve. Curve dipped and spiked before adjustment.
 b Peak storage and outflow value. Outflow restriction safety factor (Column 7) serves as conservative design assumption in case of additional outlet restriction.
 c Time increments past 130 minutes omitted because basin is already empty.



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Original Issue Date: March 30, 2009
 Date: June 18, 2009
 Description: Issued for Construction
 Engineer: David A. Vilbig, P.E.
 Project Manager: Chris Walton, P.E.
 Designer: CW

TRINITY CHRISTIAN ACADEMY
NEW PERFORMING ARTS BLDG.
 BUILDING CONSTRUCTION PLANS
 17001 ADDISON RD., ADDISON, TX 75001

PROPOSED DRAINAGE AREA MAP
 Project: AS-081
C4.0b

DO NOT SCALE DRAWING

