## CODES AND DESIGN SPECIFICATIONS

- 1. BUILDING CODE: 2006 EDITION OF THE INTERNATIONAL BUILDING
- SUPPLEMENTAL CODES AND REFERENCES TO BE USED FOR DESIGN, DETAILING AND CONSTRUCTION ARE:
- A. "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318-05).
- B. "SPECIFICATION FOR STRUCTURAL CONCRETE FOR BUILDINGS"
- C. "STEEL CONSTRUCTION MANUAL" (AISC, THIRTEENTH EDITION).
- D. "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" (2005).
- 3. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS LRFD BRIDGE DESIGN SPECIFICATIONS 2007 4TH EDITION.

# DESIGN LIVE LOADING:

1.	ALL PUBLIC ACCESS & GATHERING AREAS	100 PSF
2.	WIND 3 SEC. GUST EXPOSURE B	90 MPH
3.	SEISMIC SITE CLASSIFICATION	С
4.	SPECTRAL RESPONSE ACCELERATION (Se. S1)	0 125 0 052

### CONCRETE MIX DESIGNS:

PROVIDE NORMAL WEIGHT CONCRETE HAVING THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTHS AND GENERAL CHARACTERISTICS:

USAGE

MIN. 28 DAY COMPRESSIVE STRENGTH

4,000 PSI

- MINIMUM CONCRETE 28 DAY COMPRESSIVE STRENGTH SHALL BE BASED ON HISTORICAL PERFORMANCE DATA FROM THE SELECTED TRANSIT MIX CONCRETE SUPPLIER AND APPROVED BY THE ENGINEER.
- IT SHALL BE THE RESPONSIBILITY OF THE CONCRETE SUPPLIER TO SELECT THE PROPER TYPE OF PORTLAND CEMENT (INCLUDING QUANTITIES), AGGREGATES (INCLUDING QUANTITIES) AND WATER CEMENT RATIO TO PRODUCE THE REQUIRED MINIMUM 28 DAY COMPRESSIVE
- 4. IT SHALL BE THE RESPONSIBILITY OF THE CONCRETE SUPPLIER TO RECOMMEND ANY ENHANCING AGENTS OR ADMIXTURES TO PROVIDE A WORKABLE AND DURABLE PRODUCT.
- IT SHALL BE THE RESPONSIBILITY OF THE CONCRETE SUPPLIER TO COORDINATE WITH THE GENERAL CONTRACTOR FOR ANY ADJUSTMENTS WHICH MAY BE NECESSARY TO PROVIDE FOR HIGH EARLY STRENGTHS TO FACILITATE AND ELIMINATE DELAYS IN CONSTRUCTION.
- IT SHALL BE THE RESPONSIBILITY OF THE CONCRETE SUPPLIER TO COORDINATE WITH THE GENERAL CONTRACTOR FOR ANY ADJUSTMENTS WHICH MAY BE NECESSARY TO PROVIDE FOR HOT WEATHER OR COLD WEATHER CONCRETING PRECAUTIONS.

- REINFORCING STEEL SHALL BE NEW DEFORMED BILLET STEEL CONFORMING TO A.S.T.M. A-615, GRADE 60.
- ALL REINFORCING STEEL IN THE LAKE EDGE WALL, WIERS, PEDESTRIAN BRIDGES, & GROTTO SHALL BE EPOXY COATED.
- REINFORCING BARS SHALL BE DETAILED IN ACCORDANCE WITH THE A.C.I. DETAILING MANUAL. PROVIDE BAR SUPPORTS AND SPACERS AS REQUIRED.
- PROVIDE CORNER BARS AT ALL INTERSECTING REINFORCING MEMBERS IN WALLS AND BEAMS. CORNER BARS SHALL BE THE SAME SIZE AS THE LARGER INTERSECTING BAR AND SHALL PROVIDE A MINIMUM LAP OF 30 BAR DIAMETERS.
- 5. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

A.	CONCRETE SLABS-ON-GRADE	12" CLEAR	
В.	STRUCTURAL SLABS	2" CLEAR	
C.	PIERS	3" CLEAR	
D.	PILASTERS	1½" CLEAR	

- 5. DETAILING OF REINFORCING BARS IN BEAMS SHALL BE AS FOLLOWS:
- A. TOP AND BOTTOM BARS TO BE CONTINUOUS BETWEEN SUPPORTS.
- B. TOP BARS AT THE ENDS OF BEAMS TO HAVE STANDARD 90 DEGREE HOOKS.
- C. SPLICE TOP BARS AT THE MIDSPAN BETWEEN SUPPORTS. (U.N.O.)
- D. SPLICE BOTTOM BARS DIRECTLY OVER SUPPORTS. (U.N.O.)
- E. ALTERNATE SPLICES IN MIDDLE BARS BETWEEN SUPPORTS AND MIDSPANS WITH NO MORE THAN 1/2 OF THE BARS SPLICED AT ANY ONE LOCATION.
- F. ALL BAR SPLICES SHALL BE 30 BAR DIAMETERS MINIMUM

- STRAIGHT SHAFT PIER DESIGN IS BASED ON AN ALLOWABLE VALUE OF 18,000 PSF END BEARING AND 2,000 PSF SIDE FRICTION AS RECOMMENDED IN THE SUBSURFACE REPORT PREPARED BY GEOTEL ENGINEERING, INC. THEIR REPORT NO. E07—312, DATED DECEMBER 3,000,7
- BOTTOM OF ALL PIER HOLES SHALL BE SMOOTH, DRY AND FREE OF ALL LOOSE MATERIAL BEFORE PLACING CONCRETE.
- THE CONTRACTOR SHALL VERIFY THE DEPTH OF THE PIER PRIOR TO CUTTING PIER REINFORCING CAGES. PIER STEEL SHALL BE DELIVERED TO THE JOBSITE IN STANDARD LENGTHS AND CUT AS REQUIRED. 30 BAR DIAMETER LAPS WILL BE ALLOWED IN THE PIER STEEL IF NO MORE THAN 50 PERCENT OF THE BARS ARE LAPPED IN ANY 8 FOOT LENGTH OF THE PIER.
- REINFORCING STEEL SHOP DRAWINGS SHALL INDICATE PLACING DRAWINGS FOR TEMPLATES
  TO SET DOWELS AND ANCHOR BOLTS. REINFORCING CAGES SHALL BE ADEQUATELY
  SUPPORTED TO PROVIDED CLEARANCES INDICATED ON THE DRAWINGS.
- 6. PIER HOLES SHALL BE CONCRETED WITHIN 8 HOURS OF DRILLING.
- IF SUBSURFACE WATER CONDITIONS ARE SUCH THAT THE PIER HOLES CANNOT BE INSTALLED IN THE DRY CONDITION, THE CONTRACTOR SHALL INSTALL TEMPORARY CASING TO CONTROL WATER INTRUSION.

# STRUCTURAL STEEL:

- STRUCTURAL STEEL SHALL CONFORM TO THE A.I.S.C. "STEEL CONSTRUCTION MANUAL", THIRTEENTH EDITION.
- WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY. WELDING ELECTRODES SHALL CONFORM TO AWS A5.1 OR A5.5 FOR SERIES #E70 ELECTRODES.
- 3. BOLTS AND BOLTED CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF THE "SPECIFICATIONS" FOR STRUCTURAL JOINTS USING A.S.T.M. A-325 BOLTS OR A490 BOLTS, AS APPROVED BY THE RESEARCH COUNCIL ON RIVETED AND BOLTED JOINTS USING ASTM A-325 BOLTS. USE BEARING TYPE BOLTS WITH THREADS EXCLUDED FROM THE SHEAR
- 4. IN GENERAL, IT IS THE INTENT OF THESE PLANS AND SPECIFICATIONS THAT ALL SHOP CONNECTIONS ARE TO BE WELDED OR BOLTED AND ALL FIELD CONNECTIONS ARE TO BE BOLTED EXCEPT WHERE NOTED ON THE DRAWINGS OTHERWISE.
- STRUCTURAL STEEL SHAPES, PLATES, ETC., SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS:

A.	W SHAPE	ASTM $A992$ , $Fy = 50ksi$
B.	RECTANGULAR HSS	ASTM A500 Gr.B. Fy = 46ksi
C.	ROUND HSS	ASTM A500 Gr.B, Fy = 42ksi
D.	PIPE	ASTM A53 Gr.B. Fy = 35ksi
E.	M,S,C,MC,L	ASTM A36. $FV = 36ksi$
		ASTM A572 Gr. 50, Fy = 50ksi
G.	PLATES & BARS	ASTM A572 Gr. 50. Fv = 50ksi

- ALL BEAM CONNECTIONS SHALL BE STANDARD DOUBLE ANGLE TYPE UNLESS DETAILED OTHERWISE. PLATE SIZES, NUMBER AND SIZE OF BOLTS FOR FIELD CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR. DESIGN STANDARD CONNECTIONS USING 55% OF THE TOTAL LOAD CAPACITY SHOWN IN THE BEAM TABLES, PART 3, THIRTEENTH EDITION OF THE A.I.S.C. CODE FOR GIVEN BEAM SPAN IF THE SHEAR IS NOT GIVEN. IN NO CASE SHALL THE MINIMUM NUMBER OF BOLTS BE LESS THAN 2 ROWS FOR W12 & SMALLER, 3 ROWS FOR W14, W16, & W18, 4 ROWS FOR W21 AND W24, 5 ROWS FOR W27 & W30 AND 6 ROWS FOR W33 AND W36.
- 7. PROVIDE A PRIME COAT OF RED OXIDE PAINT TO ALL STRUCTURAL STEEL PRIOR TO
- 8. ALL SCARRED, MARRED, OR OTHERWISE DAMAGED PAINT SHALL BE TOUCHED UP AFTER COMPLETION OF CONSTRUCTION.
- PROVIDE WEB STIFFENERS TO ALL BEAMS AT SUPPORT LOCATIONS. WELD STIFFENERS TO EACH SIDE OF BEAM CENTERED DIRECTLY ABOVE SUPPORT. MIN. PLATE THICKNESS IS X<sup>\*</sup>

- LAKE EDGE WALL OUTSIDE FORM FOR UPPER 4'-0" SHALL BE FORMED WITH MEDIUM DENSITY OVERLAY PLYWOOD.
- UPPER 2'-0" & CAP OF LAKE EDGE WALL SHALL BE SAND BLASTED IN THE SAME MANNER AS THE APPROVED MOCK-UP AVAILABLE AT THE SITE.
- 3. ALL SITE WALLS THAT ARE EXPOSED CONCRETE SHALL BE FORMED AND FINISHED IN THE SAME MANNER AS THE LAKE EDGE WALL.
- 4. ALL SITE EXPOSED CONCRETE WALKS SHALL HAVE A ROCK SALT FINISH TO MATCH THE APPROVED MOCK-UP AVAILABLE AT THE SITE.

### STONE VENEER:

- 1. ALL STONE VENEER SHALL BE IMPORTED BLUE/GREEN (COLOR) CHINESE STONE OR BLUE/GRAY PENNSYLVANIA.
- 2. THE THICKNESS OF STONE VENEER SHALL BE 3 CM.
- 3. THE FINISH SURFACE OF THE STONE SHALL BE FLAMED (THERMAL).
- TOP AND BOTTOM EDGES OF VENEER SHALL BE SAW CUT, SIDES SHALL BE CHOPPED, SNAPPED OR SAW CUT.
- 5. LENGTH OF STONE SHALL BE AS FOLLOWS:
- A. FLAT SURFACE VARY LENGTHS FROM 5" TO 21". B. CURVED SURFACE (r<= 20'-0") VARY LENGTHS FROM 5" TO 13". C. CURVED SURFACE (r<= 38'-0") VARY LENGTHS FROM 5" TO 16".
- 6. ALL VENEER COURSES SHALL BE 4"
- 7. MORTAR JOINTS SHALL BE AS SMALL AS POSSIBLE NOT TO EXCEED 1/4".
- 8. MORTAR SHALL BE ACID—R, FROST PROOF; COLOR TO BE SELECTED BY LANDSCAPE ARCHITECT.
- 9. VENEER SHALL BE APPLIED TO SURFACES W/ EPOXY BASED MASTIC SUCH AS "LATTICRETE" OR APPROVED EQUIVALENT. MASTIC SHALL BE FROST PROOF.
- 10. ALL SURFACES TO RECEIVE STONE VENEER SHALL BE CLEAN, DRY AND FREE OF ALL DELETERIOUS SUBSTANCES.
- 11. COURSING AND RANDOM LENGTH PATTERN SHALL MATCH APPROVED ON-SITE MOCK UP.
- 12. CONTRACTOR, AT CONTRACTOR'S EXPENSE, SHALL PREPARE A MOCK—UP TO DEMONSTRATE PROFICIENCY IN MATCHING THE APPROVED MOCK UP PRIOR TO COMMENCEMENT OF THE WORK.



03.07.2013 95% CONSTRUCTION SET

DRAWINGS, WRITTEN MATERIAL, AND DESIGN CONCEPTS SHALL NOT BE USED OR REPRODUCED IN WHOLE OR PART IN ANY FORM OR FORMAT WITHOUT PRIOR WRITTEN CONSENT OF KEVIN SLOAN STUDIO, LLC. DO NOT SCALE DRAWINGS. USE GIVEN DIMENSIONS ONLY. IF NOT SHOWN, VERIFY CORRECT DIMENSIONS WITH THE LANDSCAPE ARCHITECT. CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND CONDITIONS AT JOB SITE.



# VITRUVIAN PARK **AMPHITHEATRE**

TOWN OF ADDISON

UDR 3875 Ponte Avenue, Suite 400 Addison, TX 75001

PROJECT: AMPHITHEATRE IMPROVEMENTS

ISSUED FOR: 100% CONSTRUCTION DOCUMENTS

**GENERAL NOTES** 

SHEET

SP101

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