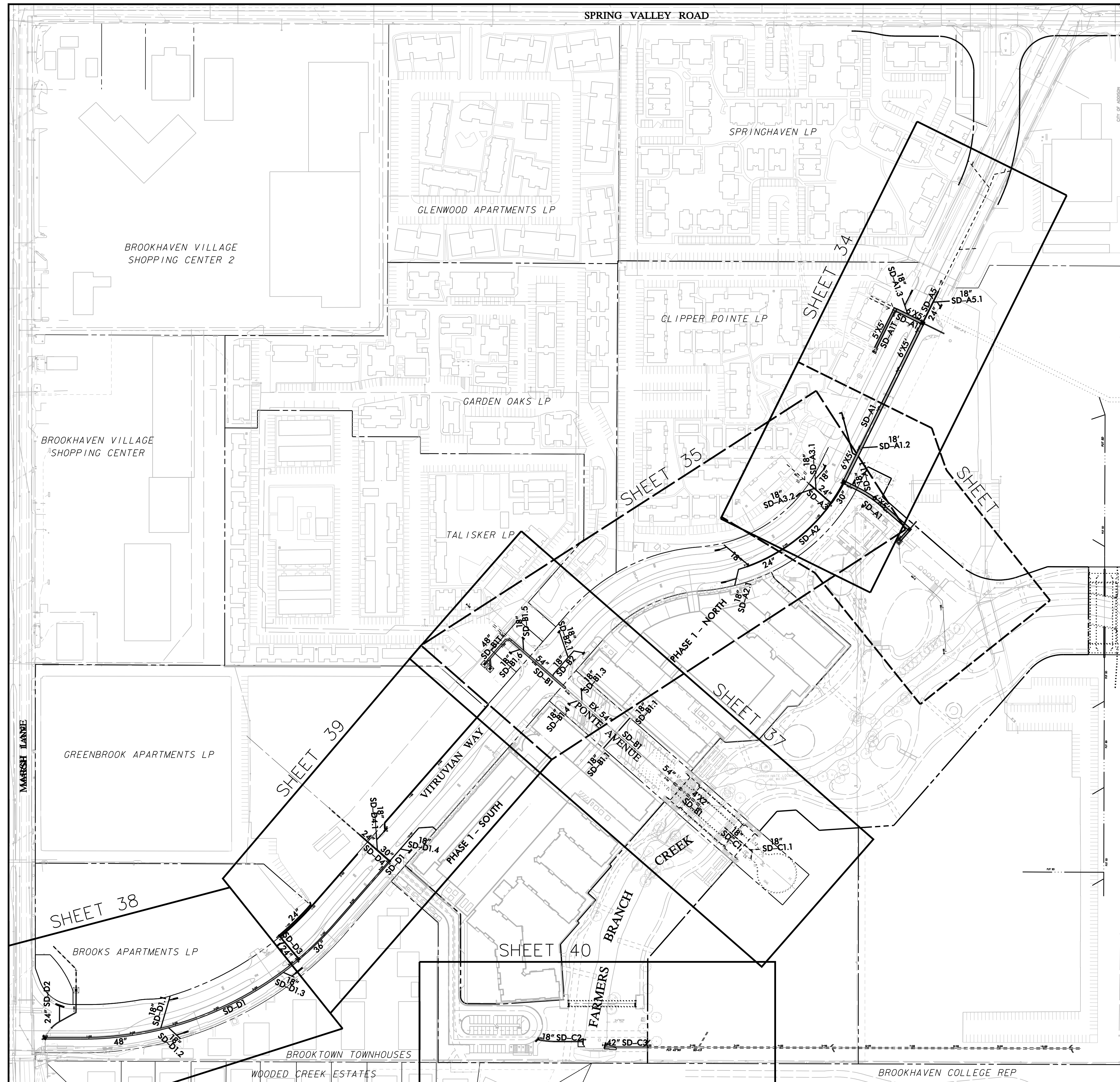


DRAINAGE AREA CALCULATIONS - PROPOSED

AREA NO.	AREA (acres)	RUNOFF COEFF	CA	Tc (min)	Q (cfs)	Q2 (cfs)	Q5 (cfs)	Q10 (cfs)	Q25 (cfs)	Q50 (cfs)	Q100 (cfs)	Q1000 (cfs)	COLLECTION POINT					
A10S	40.00	0.79	31.60	15.0	4.3	136.2	5.0	158.0	5.5	174.4	6.3	199.7	7.0	219.9	7.6	238.9	Existing Storm Drain	
A20S	0.40	0.95	0.38	15.0	4.3	1.6	5.0	1.9	5.5	2.1	6.3	2.4	7.0	2.6	7.6	2.9	8.2	Existing Storm Drain
A30S	0.30	0.95	0.29	15.0	4.3	1.2	5.0	1.4	5.5	1.6	6.3	1.8	7.0	2.0	7.6	2.2	7.6	Existing Storm Drain
A40S	0.50	0.95	0.48	15.0	4.3	2.0	5.0	2.4	5.5	2.6	6.3	3.0	7.0	3.3	7.6	3.6	7.6	Existing Storm Drain
A50S	0.60	0.95	0.57	15.0	4.3	2.5	5.0	2.9	5.5	3.1	6.3	3.6	7.0	4.0	7.6	4.3	7.6	Existing Storm Drain
A60S	0.50	0.95	0.48	15.0	4.3	2.0	5.0	2.4	5.5	2.6	6.3	3.0	7.0	3.3	7.6	3.6	7.6	Existing Storm Drain
A70S	0.30	0.95	0.29	15.0	4.3	1.2	5.0	1.4	5.5	1.6	6.3	1.8	7.0	2.0	7.6	2.2	7.6	Existing Storm Drain
A80S	0.50	0.95	0.48	15.0	4.3	2.0	5.0	2.4	5.5	2.6	6.3	3.0	7.0	3.3	7.6	3.6	7.6	Existing Storm Drain
A90S	0.60	0.95	0.57	15.0	4.3	2.5	5.0	2.9	5.5	3.1	6.3	3.6	7.0	4.0	7.6	4.3	7.6	Existing Storm Drain
A10	2.00	0.90	1.80	15.0	4.3	7.8	5.0	9.0	5.5	9.9	6.3	11.4	7.0	12.5	7.6	13.6	Future Storm Drain	
A11	1.80	0.90	1.62	15.0	4.3	7.1	5.0	8.3	5.5	8.9	6.3	10.2	7.0	11.3	7.6	12.2	Future Storm Drain	
A12	1.10	0.95	1.05	15.0	4.3	4.5	5.0	5.2	5.5	5.8	6.3	6.6	7.0	7.3	7.6	7.9	Future Storm Drain	
A13	0.60	0.90	0.54	15.0	4.3	2.3	5.0	2.7	5.5	3.0	6.3	3.4	7.0	3.8	7.6	4.1	Future Storm Drain	
A14	1.00	0.90	0.90	15.0	4.3	3.9	5.0	4.5	5.5	5.0	6.3	5.7	7.0	6.3	7.6	6.8	Future Storm Drain	
A15	0.80	0.90	0.72	15.0	4.3	3.1	5.0	3.6	5.5	4.0	6.3	4.8	7.0	5.0	7.6	5.4	Future Storm Drain	
A16	0.80	0.95	0.76	15.0	4.3	3.3	5.0	3.8	5.5	4.2	6.3	4.8	7.0	5.3	7.6	5.7	Future Storm Drain	
A17	1.00	0.40	0.40	15.0	4.3	1.7	5.0	2.0	5.5	2.2	6.3	2.5	7.0	2.8	7.6	3.0	Future Storm Drain	
A18	0.70	0.95	0.67	15.0	4.3	2.9	5.0	3.3	5.5	3.7	6.3	4.2	7.0	4.6	7.6	5.0	Future Storm Drain	
A19	1.50	0.90	1.35	15.0	4.3	5.8	5.0	6.8	5.5	7.5	6.3	8.5	7.0	9.4	7.6	10.2	Future Storm Drain	
A20	0.50	0.95	0.48	15.0	4.3	2.0	5.0	2.4	5.5	2.6	6.3	3.0	7.0	3.3	7.6	3.6	Proposed Curb Inlet	
A21	1.60	0.90	1.44	15.0	4.3	6.2	5.0	7.2	5.5	7.9	6.3	9.1	7.0	10.0	7.6	10.9	Future Storm Drain	
A22	0.40	0.95	0.38	15.0	4.3	1.6	5.0	1.9	5.5	2.1	6.3	2.4	7.0	2.6	7.6	2.9	Future Storm Drain	
A23	0.40	0.95	0.38	15.0	4.3	1.6	5.0	1.9	5.5	2.1	6.3	2.4	7.0	2.6	7.6	2.9	Future Storm Drain	
A24	0.60	0.90	0.72	15.0	4.3	3.1	5.0	3.6	5.5	4.0	6.3	4.8	7.0	5.0	7.6	5.4	Future Storm Drain	
A25	0.70	0.90	0.63	15.0	4.3	2.7	5.0	3.2	5.5	3.5	6.3	4.0	7.0	4.4	7.6	4.8	Future Storm Drain	
A26	0.50	0.90	0.45	15.0	4.3	1.9	5.0	2.3	5.5	2.5	6.3	2.8	7.0	3.1	7.6	3.4	Future Storm Drain	
A27	0.60	0.95	0.57	15.0	4.3	2.5	5.0	2.9	5.5	3.1	6.3	3.6	7.0	4.0	7.6	4.3	Proposed Curb Inlet	
A28	0.90	0.90	0.81	15.0	4.3	3.5	5.0	4.1	5.5	4.5	6.3	5.1	7.0	6.6	7.6	6.1	Future Storm Drain	
A29	0.40	0.90	0.36	15.0	4.3	1.6	5.0	1.8	5.5	2.0	6.3	2.3	7.0	2.5	7.6	2.7	Future Storm Drain	
A30	1.80	0.90	1.62	15.0	4.3	7.0	5.0	8.1	5.5	8.9	6.3	10.2	7.0	11.3	7.6	12.2	Future Storm Drain	
A31	0.80	0.90	0.72	15.0	4.3	3.1	5.0	3.6	5.5	4.0	6.3	4.8	7.0	5.0	7.6	5.4	Future Storm Drain	
A32	1.10	0.90	0.99	15.0	4.3	4.3	5.0	5.0	5.5	4.0	6.3	6.3	7.0	6.9	7.6	7.5	Future Storm Drain	
A33	1.00	0.95	0.95	15.0	4.3	4.1	5.0	4.8	5.5	5.2	6.3	6.0	7.0	6.6	7.6	7.2	Proposed Curb Inlet	
A34	1.20	0.90	1.08	15.0	4.3	4.7	5.0	5.4	5.5	6.0	6.3	6.8	7.0	7.5	7.6	8.2	Future Storm Drain	
A35	0.40	0.95	0.38	15.0	4.3	1.6	5.0	1.9	5.5	2.1	6.3	2.4	7.0	2.6	7.6	2.9	Proposed Curb Inlet	
A36	0.40	0.95	0.38	15.0	4.3	1.6	5.0	1.9	5.5	2.1	6.3	2.4	7.0	2.6	7.6	2.9	Proposed Curb Inlet	
A37	0.70	0.90	0.63	15.0	4.3	2.7	5.0	3.2	3.5	3.5	6.3	4.0	7.0	4.4	7.6	4.8	Future Storm Drain	
A38	1.10	0.95	1.05	15.0	4.3	4.5	5.0	5.2	5.5	5.8	6.3	6.6	7.0	7.3	7.6	7.9	Proposed Curb Inlet	
70.3						254.0		294.6		325.3		372.4		410.1		445.5		
B1	0.90	0.90	0.81	10.0	5.2	4.2	5.9	4.8	6.5	5.3	7.4	6.0	8.2	6.6	8.9	7.2	Future Storm Drain	
B2	0.90	0.90	0.81	10.0	5.2	4.2	5.9	4.8	6.5	5.3	7.4	6.0	8.2	6.6	8.9	7.2	Future Storm Drain	
B3	0.70	0.95	0.67	10.0	5.2	3.5	5.9	3.9	6.5	4.3	7.4	4.9	8.2	5.4	8.9	5.9	Future Storm Drain	
B4	0.30	0.90	0.27	10.0	5.2	1.4	5.9	1.6	6.5	1.8	7.4	2.0	8.2	2.2	8.9	2.4	Future Storm Drain	
B5	0.30	0.90	0.27	10.0	5.2	1.4	5.9	1.6	6.5	1.8	7.4	2.0	8.2	2.2	8.9	2.4	Future Storm Drain	
B6	0.70	0.95	0.67	10.0	5.2	3.5	5.9	3.9	6.5	4.3	7.4	4.9	8.2	5.4	8.9	5.9	Future Storm Drain	
B7	0.80	0.90	0.72	10.0	5.2	3.8	5.9	4.2	6.5	4.7	7.4	5.4	8.2	5.9	8.9	6.4	Future Storm Drain	
B8	1.10	0.95	1.05	10.0	5.2	5.5	5.9	6.1	6.5	6.8	7.4	7.8	8.2	8.5	8.9	9.3	Future Storm Drain	
B9	0.80	0.90	0.72	10.0	5.2	3.8	5.9	4.2	6.5	4.7	7.4	5.4	8.2	5.9	8.9	6.4	Future Storm Drain	
B10	0.50	0.95	0.45	10.0	5.2	2.4	5.9	2.6	6.5	4.9	7.4	5.7	8.2	3.7	8.9	4.0	Future Storm Drain	
B11	1.00	0.90	0.90	10.0	5.2	4.7	5.9	5.3	6.5	5.8	7.4	6.7	8.2	7.3	8.9	8.0	Future Storm Drain	
B12	1.00	0.90	0.90	10.0	5.2	4.7	5.9	5.3	6.5	5.8	7.4	6.7	8.2	7.3	8.9	8.0	Future Storm Drain	
B13	0.70	0.90	0.63	10.0	5.2	3.3	5.9	3.7	6.5	4.1	7.4	4.7	8.2	5.1	8.9	5.6	Future Storm Drain	
B14	1.10	0.95	1.05	10.0	5.2	5.5	5.9	6.1	6.5	6.8	7.4	7.8	8.2	8.5	8.9	9.3	Future Storm Drain	
B15	0.90	0.90	0.81	10.0	5.2	4.2	5.9	4.8	6.5	5.3	7.4	6.0	8.2	6.6	8.9	7.2	Future Storm Drain	
B16	0.40	0.90	0.36	10.0	5.2	1.9	5.9	2.1	6.5	2.3	7.4	2.7	8.2	2.9	8.9	3.2	Future Storm Drain	
B17	0.90	0.95	0.86	10.0	5.2	4.5	5.9	5.0	6.5	5.5	7.4	6.4	8.2	7.0	8.9	7.6	Future Storm Drain	
B18	0.40	0.90	0.36	10.0	5.2	1.9	5.9	2.1	6.5	2.3	7.4	2.7	8.2	2.9	8.9	3.2	Future Storm Drain	
B19	0.60	0.90	0.54	10.0	5.2	2.8	5.9	3.2	6.5	3.5	7.4	4.0	8.2	4.4	8.9	4.8	Future Storm Drain	
B20	0.50	0.95	0.48	10.0	5.2	2.5	5.9	2.8	6.5	3.1	7.4	3.5	8.2	3.9	8.9	4.2	Future Storm Drain	
B21	0.60	0.90	0.54	10.0	5.2	2.8	5.9	3.2	6.5	3.5	7.4	4.0	8.2	4.4	8.9	4.8	Future Storm Drain	
B22	0.60	0.90	0.54	10.0	5.2	2.8	5.9	3.2	6.5	3.5	7.4	4.0	8.2	4.4	8.9	4.8	Future Storm Drain	
B23	0.20	0.95	0.19	10.0	5.2	1.0	5.9	1.2	6.5	1.2	7.4	1.4	8.2	1.6	8.9	1.7	Proposed Curb Inlet	
B24	0.20	0.95	0.19	10.0	5.2	1.0	5.9	1.1	6.5	1.2	7.4	1.4	8.2	1.6	8.9	1.7	Proposed Curb Inlet	
B25	0.60	0.90	0.54	10.0	5.2	2.8	5.9	3.2	6.5	3.5	7.4	4.0	8.2	4.4	8.9	4.8	Proposed Storm Drain	
B26	0.60	0.90	0.54	10.0	5.2	2.8	5.9	3.2	6.5	3.5	7.4	4.0	8.2	4.4	8.9	4.8	Proposed Storm Drain	
B27	0.50	0.95	0.48	10.0	5.2	2.5	5.9	2.8	6.5	3.1	7.4	3.5	8.2	3.9	8.9	4.2	Proposed Curb Inlet	
17.8						85.3		85.9		105.9		121.4		133.1		144.9		
C1	0.40	0.90	0.36	10.0	5.2	1.9	5.9	2.1	6.5	2.3	7.4	2.7	8.2	2.9	8.9	3.2	Future Storm Drain	
C2	0.10	0.40	0.10	5.2	2.4	0.9	5.9	0.9	6.5	0.9	7.4	0.9	8.2	0.9	8.9	0.9	Future Storm Drain	
C3	0.60	0.90	0.54	10.0	5.2	2.8	5.9	3.2	6.5	3.5	7.4	4.0	8.2	4.4	8.9	4.8	Future Storm Drain	
C4	0.30	0.40	0.12	10.0	5.2	0.6	5.9	0.7	6.5	0.8	7.4	0.9	8.2	1.0	8.9	1.1	Overland Flow	
C5	0.30	0.90	0.27	10.0	5.2	1.4	5.9	1.6	6.5	1.8	7.4	2.0	8.2	2.2	8.9	2.4	Future Storm Drain	
C6	0.30	0.90	0.27	10.0	5.2	1.4	5.9	1.6	6.5	1.8	7.4	2.0	8.2	2.2	8.9	2.4	Future Storm Drain	
C7	0.80	0.95	0.75	10.0	5.2	4.0	5.9	4.5	6.5	4.9	7.4	5.7	8.2	8.2	8.9	6.7	Future Storm Drain	
C8	1.00	0.90	0.90	10.0	5.2	4.7	5.9	5.3	6.5	5.8	7.4	6.7	8.2	7.3	8.9	8.0	Future Storm Drain	
C9	1.00	0.90	0.90	10.0	5.2	4.7	5.9	5.3	6.5	5.8	7.4	6.7	8.2	7.3	8.9	8.0	Future Storm Drain	
C10	0.80	0.68	0.54	10.0	5.2	2.8	5.9	3.2	6.5	3.5	7.4	4.0	8.2	4.4	8.9	4.8	Future Storm Drain	
C11	0.40	0.40	0.18	10.0	5.2	0.9	5.9	0.9	6.5	1.5	7.4	1.3	8.2	1.5	8.9	1.6	Overland Flow	
C12	0.20	0.90	0.18	10.0	5.2	0.9	5.9	1.1	6.5	1.2	7.4	1.3	8.2	1.5	8.9	1.6	Future Storm Drain	

STORM DRAIN CALCULATIONS - 100 YR														HEAD LOSS AT CHANGE IN SECTION										Elev Difference		REMARKS
MH or INLET		DISTANCE	Peak Flow	FRICTIONAL SLOPE	HYDRAULIC GRADIENT		V1				V2				Elev		TC/FG - HGL		at Des Pt							
UPSTRM STATION	DNSTRM STATION				Between Points	in Pipe	PIPE SIZE	UPSTRM	DNSTRM	Flow IN	Flow OUT	V2(2)	V1(2)	Kj	Kj/V1(2)	Hj	Elev of Hyd	TC/FG		TC/FG - HGL						
STATION	STATION	(ft)	(cfs)	(in)	(ft / ft)	(ft MSL)	(ft MSL)	(ft/s)	(ft/s)	(ft)	(ft)	(const)	(ft)	(ft)	(ft MSL)	(ft MSL)										
LINE A1																										
1094.53	1094.53	0.00	66	0.0061	566.39	566.39	—	0.00	0.00	—	0.50	—	0.00	566.39	566.80	0.41	EXISTING MANHOLE W / 90° BEND									
1094.53	991.40	103.13	329.3	66	0.0096	566.39	565.40	0.00	13.86	2.98	0.00	0.50	2.98	0.00	566.39	567.70	1.31	90° BEND								
991.40	956.95	34.45	329.3	72	0.0060	563.16	562.95	13.86	11.65	2.11	2.98	0.75	-0.13	2.24	565.40	567.70	2.30	45° WYE								
956.95	920.40	36.55	343.1	72	0.0066	561.90	561.66	11.65	12.13	2.29	2.11	0.50	1.23	1.05	562.95	567.70	4.75	MANHOLE W / 90° BEND								
920.40	603.24	317.16	383.8	72	0.0082	559.94	557.34	12.13	13.57	2.86	2.29	0.75	1.15	1.71	561.66	567.70	6.04	60° WYE								
603.24	515.93	87.31	386.5	72	0.0083	555.91	555.18	13.57	13.67	2.90	2.86	0.50	1.47	1.43	557.34	561.50	4.16	MANHOLE W / 90° BEND								
515.93	483.89	32.04	437.6	72	0.0107	555.18	555.18	13.67	15.48	3.72	2.90	3.72	0.00	555.18	561.80	6.62	SUBMERGED 45° WYE									
483.89	351.58	132.31	445.5	72	0.0111	555.18	555.18	15.48	15.76	3.86	3.72	3.86	0.00	555.18	562.00	6.82	SUBMERGED MANHOLE									
351.58	329.69	21.89	445.5	66	0.0176	555.18	555.18	15.76	18.75	5.46	3.86	5.46	0.00	555.18	562.00	6.82	SUBMERGED 45° BEND									
329.69	322.19	7.50	445.5	66	0.0176	555.18	555.18	18.75	18.75	5.46	5.46	5.46	0.00	555.18	562.00	6.82	SUBMERGED 45° BEND									
322.19	305.26	16.93	445.5	66	0.0176	555.18	555.18	18.75	18.75	5.46	5.46	5.46	0.00	555.18	562.00	6.82	SUBMERGED OUTLET									
LINE A2																										
385.87	385.87	0.00	18	0.0061	560.34	560.34	—	0.00	0.00	—	—	—	0.00	560.34	567.80	7.46	END & PLUG									
385.87	383.20	22.67	8.2	18	0.0061	560.34	560.21	0.00	4.64	0.33	0.00	0.75	0.33	0.00	560.34	567.60	7.26	60° WYE								
383.20	313.44	49.76	11.1	18	0.0112	560.49	559.93	4.64	6.28	0.61	0.33	0.25	0.53	0.08	560.57	567.60	7.03	60° BEND								
313.44	303.32	10.12	11.1	24	0.0024	559.95	559.93	4.64	3.53	0.19	0.33	0.75	-0.06	0.25	560.21	567.50	7.29	60° WYE								
303.32	55.94	247.38	18.8	24	0.0069	559.88	558.17	3.53	5.98	0.56	0.19	0.25	0.51	0.05	559.93	562.30	2.37	MANHOLE W / 90° BRANCH								
55.94	0.00	55.94	51.1	24	0.0510	558.03	555.18	5.98	16.27	4.11	0.56	0.25	3.97	0.14	558.17	561.50	3.33	MANHOLE W / 90° BRANCH								
LINE A3																										
64.94	64.94	0.00	24	0.0061	559.47	559.47	—	0.00	0.00	—	—	—	0.00	559.47	562.00	2.53	END & PLUG									
64.94	50.77	14.17	12.2	24	0.0029	559.47	559.43	0.00	3.88	0.23	0.00	0.75	0.23	0.00	559.47	561.75	2.28	60° WYE								
50.77	45.90	4.87	17.6	24	0.0061	559.26	559.23	3.88	5.60	0.49	0.23	0.75	0.31	0.18	559.43	561.75	2.32	60° WYE								
45.90	0.00	45.90	32.3	24	0.0204	559.11	558.17	5.60	10.28	1.64	0.49	0.25	1.52	0.12	559.23	562.30	3.07	MANHOLE W / 90° BRANCH								
LINE A4																										
55.00	55.00	0.00	24	0.0061	561.70	561.70	—	0.00	0.00	—	—	—	0.00	561.70	567.50	5.80	END & PLUG									
55.00	0.00	55.00	6.1	24	0.0007	561.70	561.66	0.00	1.94	0.06	0.00	0.25	0.06	0.00	561.70	567.70	6.00	MANHOLE W / 90° BRANCH								
LINE A5																										
81.00	81.00	0.00	24	0.0000	563.58	563.58	—	0.00	0.00	—	—	—	0.00	563.58	570.00	6.42	END & PLUG									
81.00	53.00	28.00	26.9	24	0.0141	563.58	563.18	0.00	8.56	1.14	0.00	0.75	1.14	0.00	563.58	569.30	5.72	60° WYE								
53.00	0.00	53.00	34.6	24	0.0234	562.90	561.66	8.56	11.01	1.88	1.14	0.25	1.60	0.28	563.18	567.70	4.52	MANHOLE W / 90° BRANCH								
LINE B1																										
759.86	759.86	0.00	48	0.0000	567.94	567.94	—	0.00	0.00	—	—	—	0.00	567.94	566.60	-1.34	TEMPORARY EXISTING FLOW									
759.86	741.86	18.00	126.0	48	0.0077	567.94	567.80	0.00	10.03	1.56	0.00	1.56	0.00	567.94	568.10	0.16	TEMPORARY EXISTING FLOW									
741.86	673.93	67.93	126.0	48	0.0077	567.80	567.28	10.03	10.03	1.56	1.56	1.56	0.00	567.80	568.40	0.60	TEMPORARY EXISTING FLOW									
673.93	644.52	29.41	130.8	54	0.0044	567.28	567.15	10.03	8.22	1.05	1.56	1.05	0.00	567.28	568.80	1.52	PIPE SIZE CHANGE									
644.52	635.86	8.66	132.9	54	0.0046	566.36	566.32	8.22	8.36	1.08	1.05	0.75	0.30	0.79	567.15	568.80	1.65	60° WYE								
635.86	548.02	87.84	135.0	54	0.0047	565.51	565.09	8.36	8.49	1.12	1.08	0.75	0.31	0.81	566.32	568.80	2.48	45° WYE								
548.02	467.81	80.21	138.4	54	0.0050	564.25	563.85	8.49	8.70	1.18	1.12	0.75	0.34	0.84	565.09	565.85	0.76	60° WYE								
467.81	459.14	8.67	140.5	54	0.0051	562.97	562.93	8.70	8.83	1.21	1.18	0.75	0.33	0.88	563.85	568.80	4.95	60° WYE								
459.14	354.25	104.89	142.6	54	0.0053	562.02	561.47	8.83	8.97	1.25	1.21	0.75	0.34	0.91	562.93	568.80	5.87	45° WYE								
354.25	145.77	208.48	142.6	54	0.0053	561.16	560.06	8.97	8.97	1.25	1.25	0.25	0.94	0.31	561.47	570.05	8.58	MANHOLE W / 90° BRANCH								
145.77	101.00	44.77	142.6	54	0.0053	554.30	554.30	8.97	8.97	1.25	1.25	0.44	0.70	0.55	554.30	566.00	11.70	MANHOLE (Enlargement)								

STORM DRAIN CALCULATIONS - 100 YR														HEAD LOSS AT CHANGE IN SECTION										Elev Difference		REMARKS
MH or INLET		DISTANCE	Peak Flow	FRICTIONAL SLOPE	HYDRAULIC GRADIENT		V1				V2				Elev		TC/FG - HGL		at Des Pt							
UPSTRM STATION	DNSTRM STATION				Between Points	in Pipe	PIPE SIZE	UPSTRM	DNSTRM	Flow IN	Flow OUT	V2(2)	V1(2)	Kj	Kj/V1(2)	Hj	Elev of Hyd	TC/FG		TC/FG - HGL						
STATION	STATION	(ft)	(cfs)	(in)	(ft / ft)	(ft MSL)	(ft MSL)	(ft/s)	(ft/s)	(ft)	(ft)	(const)	(ft)	(ft)	(ft MSL)	(ft MSL)										
LINE B2																										
98.45	98.45	0.00	18	0.0000	560.22	560.22	—	0.00	0.00	—	—	—	0.00	560.22	568.85	8.63	CURB INLET									
98.45	77.09	21.36	1.7	18	0.0003	560.21	560.20	0.00	0.96	0.01	0.00	1.25	0.01	0.02	560.22	568.85	8.63	CURB INLET								
77.09	57.45	19.64	1.7	18	0.0003	560.19	560.18	0.96	0.96	0.01	0.01	0.75	0.00	0.01	560.20	568.80	8.70	60° BEND								
57.45	0.00	57.45	3.4	18	0.0010	560.18	560.12	0.96	1.92	0.06	0.01	0.25	0.05	0.00	560.18	569.25	9.07	60° BEND								
LINE C1																										
166.09	166.09	0.00	18	0.0000	555.57	555.57	—	0.00	0.00	—	—	—	0.00	555.57	566.60	11.03	CURB INLET									
166.09	148.77	17.32	5.1	18	0.0024	555.41	555.37	0.00	2.89	0.13	0.00	1.25	0.13	0.16	555.57	566.60	11.03	CURB INLET								
148.77	142.61	6.16	5.1	18	0.0024	555.32	555.30	2.89	2.89	0.13	0.13	0.43	0.07	0.06	555.37	568.10	12.73	60° BEND								
142.61	113.76	28.85	10.1	18	0.0092	555.20	554.94	2.89	5.72	0.51	0.13	0.75	0.41	0.10	555.30	568.40	13.10	45° WYE								
113.76	69.00	44.76	10.1	18	0.0092	554.71	554.30	5.72	5.72	0.51	0.51	0.44	0.28	0.22	554.94	568.80	13.86	MANHOLE (Enlargement)								
LINE C2																										
106.50	0.00	106.50	5.1	18	0.0024	552.25	552.00	—	2.89	0.13	—	1.25	—	0.16	552.41	552.50	0.09	CURB INLET								
LINE C3																										
97.82	97.82	0.00	42	0.0000	553.27	553.27	—	0.00	0.00	—	—	—	0.00	553.27	553.90	0.63	END & PLUG									
97.82	87.41	10.41	73.3	42	0.0053	553.27	553.22	0.00	7.62	0.90	0.00	0.90	0.00	553.27	553.90	0.63	END & PLUG									
87.41	0.00	87.41	79.2	42	0.0062	552.54	552.00	7.62	8.23	1.05	0.90	0.75	0.38	0.68	553.22	553.70	0.48	60° BEND								
LINE D1																										
1068.38	1068.38	0.00	18	0.0000	551.20	551.20	—	0.00	0.00	—	—	—	0.00	551.20	556.05	4.85	END & PLUG									
1068.38	1057.88	10.50	5.6	18	0.0028	551.20	551.17	0.00	3.17	0.16	0.00	0.16	0.00	551.20	556.05	4.85	END & PLUG									
1057.88	1026.06	31.82	5.6	18	0.0028	551.11	551.02	3.17	3.17	0.16	0.16	0.35	0.10	0.05	551.17	555.70	4.53	45° BEND								
1026.06	962.38	63.68	5.6	18	0.0028																					



- STORM DRAIN GENERAL NOTES**
- REFER TO SHEET 4 "GENERAL CONSTRUCTION NOTES, LEGEND AND ABBREVIATIONS" FOR THE GENERAL CONSTRUCTION NOTES FOR THIS PROJECT.
 - ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH STANDARD SPECIFICATIONS FOR STORM DRAINAGE CONSTRUCTION AS PUBLISHED BY NORTH CENTRAL TEXAS COUNCIL OF GOVERNMENTS, AND ANY AND ALL AMENDMENTS BY THE TOWN OF ADDISON, AS WELL AS STANDARD CONSTRUCTION DETAILS OF THE TOWN OF ADDISON.
 - PRIOR TO COMMENCING CONSTRUCTION, THE TOWN OF ADDISON, THE CONSULTING ENGINEERS, THE SUCCESSFUL CONTRACTOR, UTILITY COMPANIES, AND ANY OTHER AFFECTED PARTIES, SHALL CONVENE FOR A PRE-CONSTRUCTION CONFERENCE AT LEAST 48 HOURS PRIOR TO THE BEGINNING OF CONSTRUCTION.
 - THE CONTRACTOR SHALL OBTAIN A RIGHT-OF-WAY PERMIT FROM THE TOWN OF ADDISON PRIOR TO WORKING WITHIN THE PUBLIC RIGHT-OF-WAY.
 - IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT ANY PUBLIC UTILITY COMPANIES FOR LOCATION OF EXISTING FACILITIES IN OR NEAR THE WORK AREAS. THESE INCLUDE, BUT ARE NOT LIMITED TO THE FOLLOWING:
 TOWN OF ADDISON (WATER, SEWER, SIGNALS) ATMOS ENERGY (GAS)
 ONCOR ELECTRIC DELIVERY VERIZON / MCI
 AT&T (SOUTHWESTERN BELL) TIME-WARNER CABLE
 - THE CONTRACTOR SHALL PROVIDE SUBMITTALS TO THE ENGINEER (SIX SETS EACH), FOR APPROVAL OF ALL MATERIALS TO BE ADDED TO THE PUBLIC INFRASTRUCTURE, PRIOR TO INCORPORATING MATERIALS INTO THE JOB.
 - THE CONTRACTOR SHALL PROVIDE AND SUBMIT TO THE TOWN OF ADDISON (SIX SETS EACH), AN APPROVED TRENCH SAFETY PLAN, SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF TEXAS, FOR THE INSTALLATION OF STORM DRAINAGE FACILITIES GREATER THAN FIVE (5) FEET IN DEPTH.
 - THE CONTRACTOR SHALL FULLY COMPLY WITH, AND SUPPLEMENT AS NECESSARY, THE CONDITIONS OF THE STORM WATER POLLUTION PREVENTION PLAN WHILE CONDUCTING HIS ACTIVITIES ON THIS PROJECT.
 - IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO IMPLEMENT, AND SUPPLEMENT AS NECESSARY, THE TRAFFIC CONTROL MEASURES ON THIS PROJECT, INCLUDING PROVIDING ADEQUATE FLAGMEN, SIGNAGE, STRIPING AND WARNING DEVICES, ETC., DURING CONSTRUCTION IN ACCORDANCE WITH THE TEXAS MANUAL OF UNIFORM CONTROL DEVICES. THE CONTRACTOR SHALL MAINTAIN AT LEAST ONE LANE OF TRAFFIC IN EACH DIRECTION DURING WORKING HOURS OR PROVIDE AN ALL-WEATHER DETOUR AROUND THE CONSTRUCTION SITE, INCLUDING PUBLIC NOTIFICATION AND SIGNAGE.
 - THE TOWN OF ADDISON PUBLIC WORKS DEPARTMENT WILL APPROVE AND/OR DETERMINE THE TRAFFIC CONTROL PLAN AND WORKING HOURS. CONTACT THE ASSISTANT CITY ENGINEER OR THE PUBLIC WORKS INSPECTOR AT (972) 450-2871.
 - TEMPORARY OR PERMANENT BARRICADES SHALL REMAIN AT ALL POINTS OF INGRESS OR EGRESS TO PREVENT PUBLIC USE UNTIL THE WORK RECEIVES FINAL ACCEPTANCE.
 - THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ADEQUATE DRAINAGE AT ALL TIMES DURING CONSTRUCTION, INCLUDING PROVIDING ALL TEMPORARY STRUCTURES OR IMPROVEMENTS AS NECESSARY FOR THE SAFETY OF THE PUBLIC PLATING, DURING NON-WORKING HOURS, ALONG EXISTING ROADWAYS AND TRAFFIC AREAS.
 - THE TOWN OF ADDISON WILL PROVIDE A GEOTECHNICAL LABORATORY TO PERFORM APPROPRIATE TESTING DURING CONSTRUCTION ACTIVITIES. ANY TEST THAT FAILS TO MEET CITY REQUIREMENTS SHALL BE RETESTED AT THE CONTRACTORS EXPENSE.
 - THE CONTRACTOR SHALL PROVIDE A MAINTENANCE BOND FOR PUBLIC INFRASTRUCTURE WORK IN THE FOLLOWING AMOUNTS:
 - 100% FOR VALUATIONS LESS THAN OR EQUAL TO \$5,000
 - \$5,000 FOR VALUATION GREATER THAN \$5,000 AND LESS THAN \$50,000
 - 10% FOR VALUATIONS GREATER THAN \$50,000
 BONDS SHALL BE FOR A PERIOD OF TWO YEARS BEGINNING WITH THE DATE OF FINAL ACCEPTANCE BY THE TOWN.
 - THE CONTRACTOR SHALL VERIFY THE SIZE, TYPE, ELEVATION, CONFIGURATION, AND ANGLULATION OF EXISTING STORM DRAIN LINES PRIOR TO CONSTRUCTION OF TIE-IN MATERIALS. THE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR REPAIRS TO EXISTING FACILITIES DAMAGED BY HIS ACTIVITIES.
 - THE CONTRACTOR SHALL MAKE NECESSARY PROVISIONS FOR THE SUPPORT AND PROTECTION OF ALL UTILITY POLES, FENCES, TREES, SHRUBS, GAS MAINS, TELEPHONE CABLES, ELECTRIC CABLES, DRAINAGE PIPES, UTILITY SERVICES, AND ALL OTHER UTILITIES AND STRUCTURES BOTH ABOVE AND BELOW THE GROUND, THE COST OF WHICH SHALL BE INCLUDED IN THE CONTRACT AMOUNT.
 - ROUGH GRADING SHALL BE COMPLETED PRIOR TO CONSTRUCTION OF STORM DRAIN FACILITIES.
 - ALL STORM DRAIN STRUCTURES INCLUDING MANHOLES, INLETS AND CLEANOUTS MUST BE ADJUSTED TO FINAL FINISHED GRADE BY THE CONTRACTOR.
 - ALL PIPE FOR PUBLIC STORM DRAIN IMPROVEMENTS SHALL BE REINFORCED CONCRETE PIPE (RCP, CLASS II), UNLESS OTHERWISE NOTED. REINFORCED CONCRETE PIPE JOINTS SHALL BE SEALED WITH RAMMECK OR APPROVED EQUAL.
 - ALL STORM SEWER SYSTEMS WITH RADII LESS THAN 100' SHALL UTILIZE 4' LONG PIPE JOINTS WITH BEVELED ENDS (R-2 RADIUS PIPE). ALL JOINTS MUST BE TIGHT AND SHALL NOT GAP MORE THAN 1/8" THE JOINT LENGTH.
 - ALL PIPE ENTERING PUBLIC STORM DRAIN STRUCTURES SHALL BE GROUTED TO ASSURE WATERTIGHT CONNECTIONS.
 - EMBEDMENT FOR STORM DRAIN PIPING SHALL CONSIST OF GRADE 4 CRUSHED STONE (3" BELOW PIPE FOR 24" AND SMALLER PIPES, AND 4" BELOW PIPE FOR 30" PIPES AND LARGER) TO THE CRADLE OF THE PIPE, WITH SELECT NATIVE SOIL LESS THAN 3" IN DIAMETER OR GRANULAR MATERIAL TO 6" OVER THE TOP OF PIPE.
 - FINISH BACKFILL SHALL BE NATIVE SOIL FREE OF ALL ROCKS AND CLODS GREATER THAN THREE INCHES IN DIAMETER, COMPACTED TO 95% STANDARD PROCTOR DENSITY, IN SIX (6) INCH MAXIMUM LOOSE LIFTS, WITH ZERO TO PLUS THREE, OPTIMUM MOISTURE.
 - CONCRETE COLLARS SHALL BE INSTALLED AT ALL CHANGES IN CONDUIT SIZE AND AT ALL JOINTS THAT ARE PULLED IN EXCESS OF THAT RECOMMENDED BY THE CONDUIT MANUFACTURER.
 - THE CONTRACTOR SHALL COMPLETELY REMOVE AND DISPOSE OF EXISTING STORM DRAIN FACILITIES DESIGNATED TO BE REMOVED, UPON COMPLETION AND ACCEPTANCE OF NEW STORM DRAINAGE FACILITIES.
 - ALL STORM DRAIN PIPE SHALL BE CAMERA INSPECTED BY THE CONTRACTOR AFTER THE INSTALLATION OF ALL UTILITIES AND PRIOR TO FINAL ACCEPTANCE OF THE PROJECT.
 - THE CONTRACTOR SHALL CALL (972) 450-2847 TO REQUEST A FINAL WALK-THROUGH INSPECTION OF THE PUBLIC INFRASTRUCTURE WORK.
 - ANY ADJACENT PROPERTIES AFFECTED BY THE CONSTRUCTION SHALL BE RESTORED TO PRE-CONSTRUCTION CONDITIONS, OR BETTER.

WARNING

CONTRACTOR IS TO CONTACT TEXAS ONE-CALL SYSTEM (1-800-245-4545) OR OTHER UTILITY LOCATING SERVICES AT LEAST 48 HOURS PRIOR TO CONSTRUCTION ACTIVITIES. ICON CONSULTING ENGINEERS, INC. IS NOT RESPONSIBLE FOR KNOWING ALL EXISTING UTILITIES IN THE PROJECT AREA NOR FOR DEPICTING THE EXACT LOCATIONS OF UTILITIES ON THESE DRAWINGS.

BM #1 REF. ELEVATION = 559.47
 SQUARE CUT IN TOP OF CURB, SOUTH MEDIAN END NOSE, MARSH LANE 1127' NORTH OF VITRUVIAN WAY.

BM #2 REF. ELEVATION = 547.84
 SQUARE CUT IN TOP OF CURB, NORTH MEDIAN END NOSE, AT INTERSECTION OF VITRUVIAN WAY AND MARSH LANE.

BRUCE F. DUNNE
 LICENSED PROFESSIONAL ENGINEER

NO.	REVISION	BY	DATE

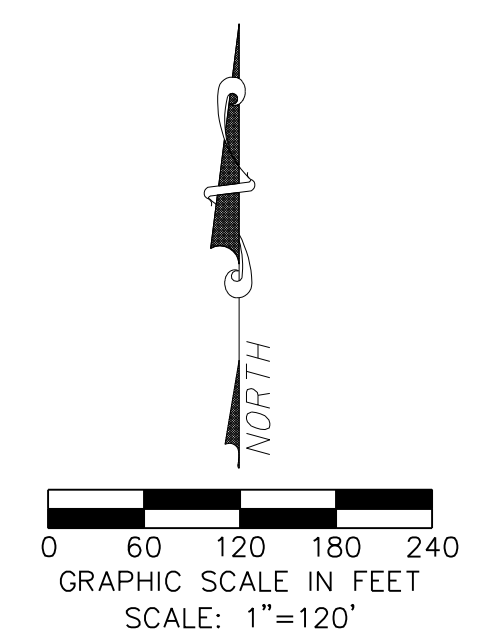
TOWN OF ADDISON
DALLAS COUNTY, TEXAS

PAVING, DRAINAGE & UTILITY IMPROVEMENTS
VITRUVIAN WAY & PONTE AVENUE

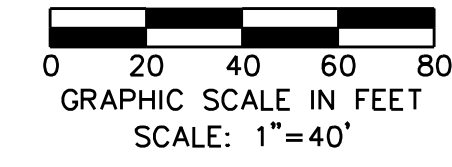
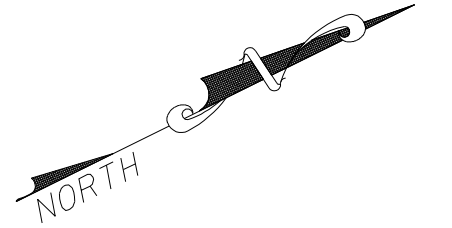
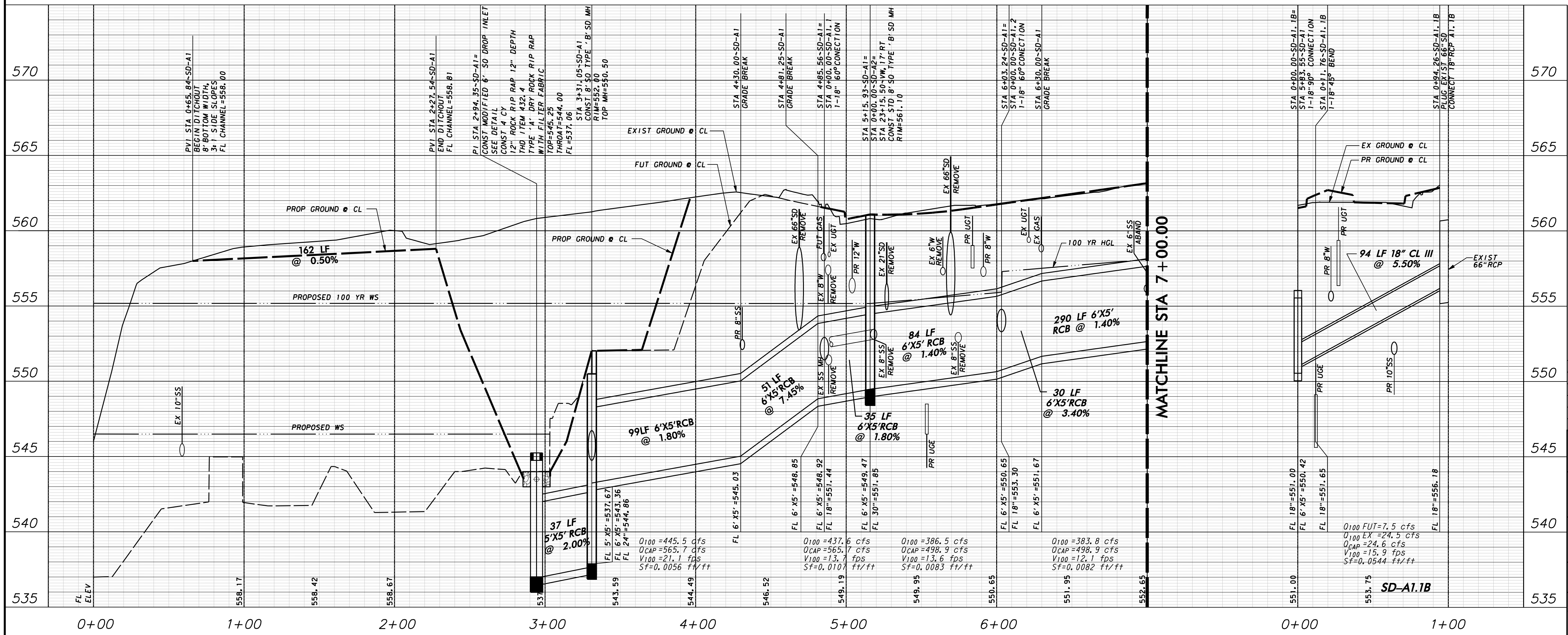
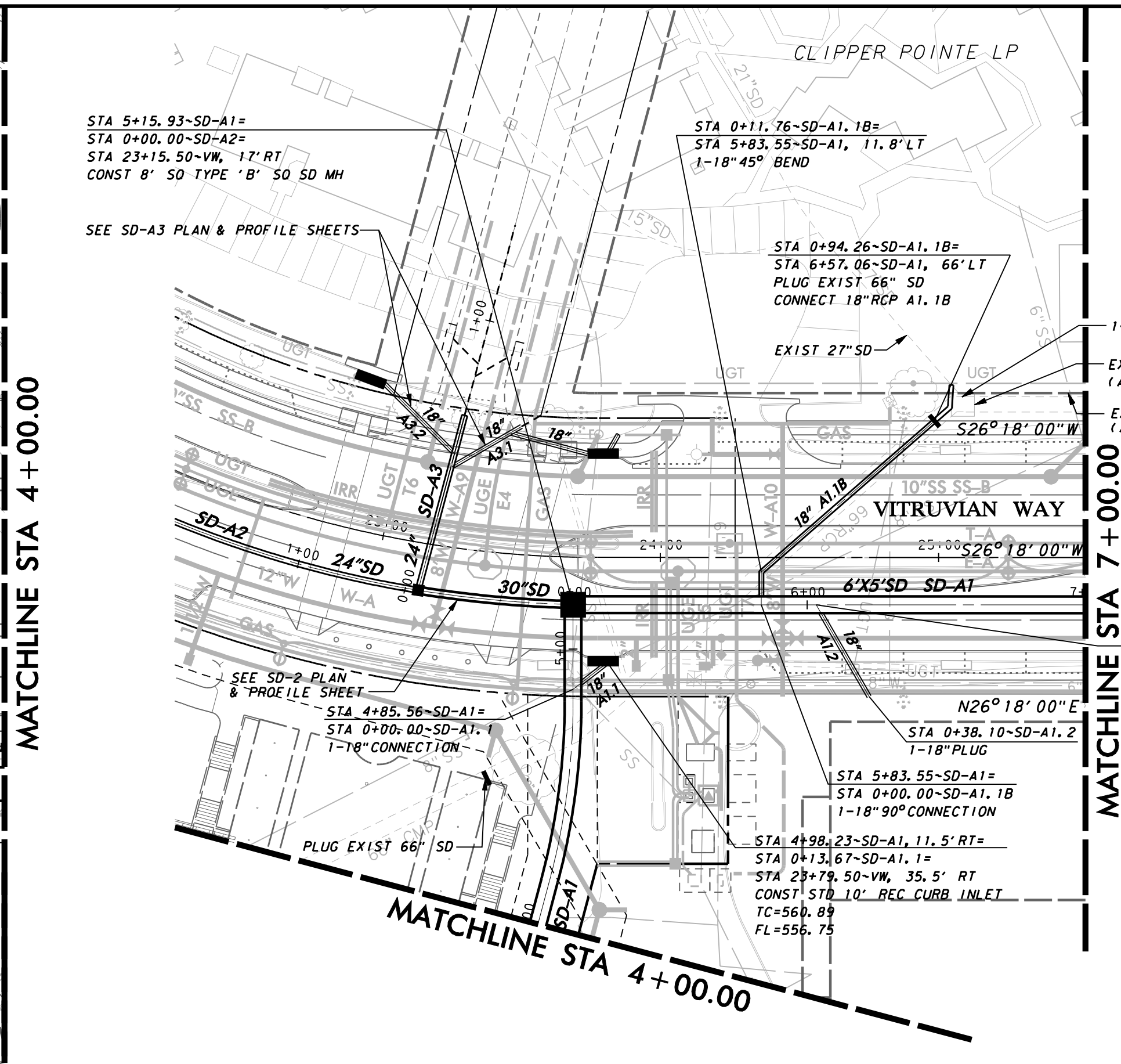
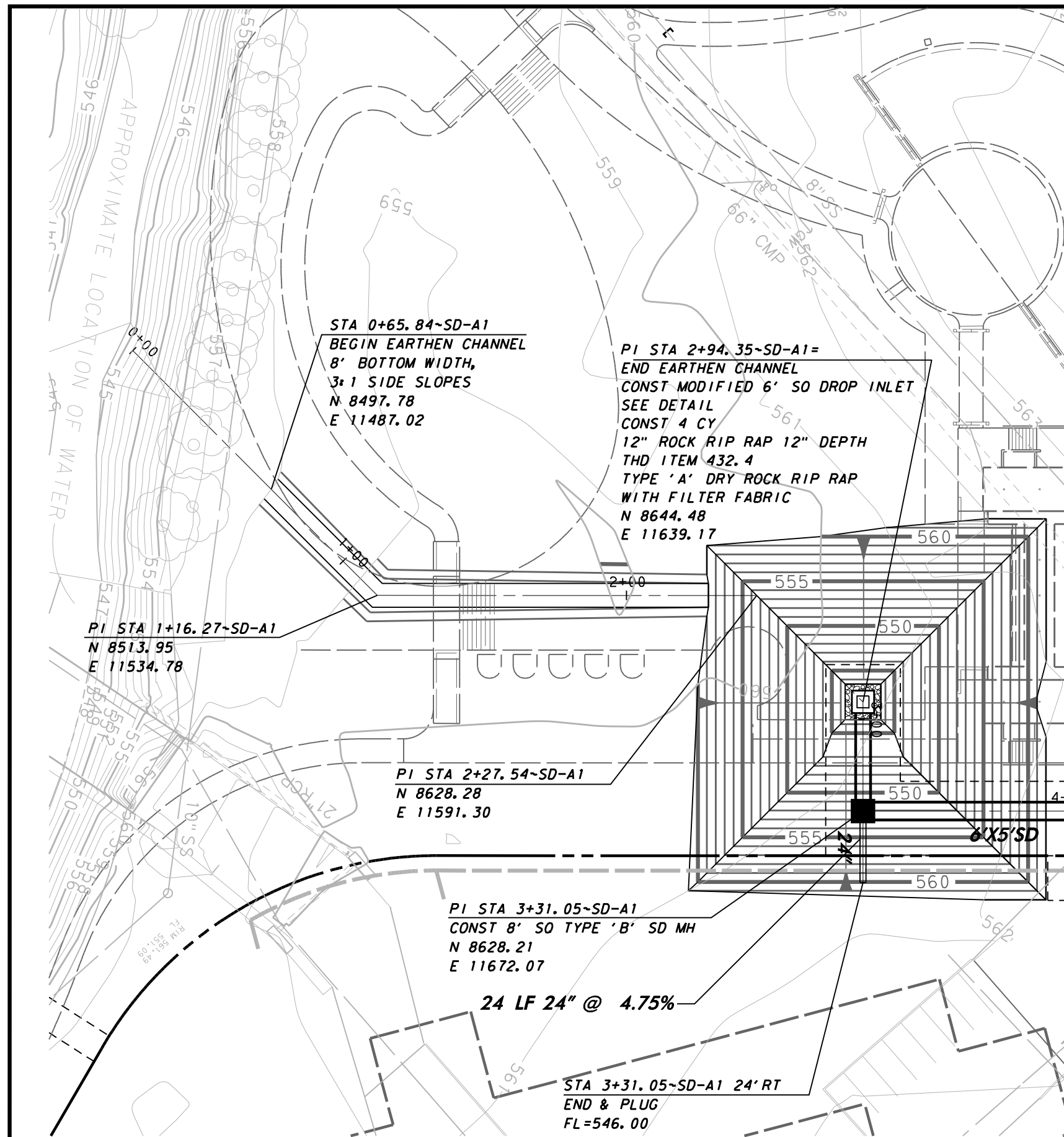
OVERALL STORM DRAIN LAYOUT & NOTES

PROJECT	DESIGN	DRAWN	DATE	FILE	SHEET
5029-01	ICE	ICE	MAY 7 2009	PW# 2009-01	32

icon Consulting Engineers, Inc. 250 W. Southlake Blvd., Suite 117
 Civil Engineers - Designers - Planners Southlake, Tx 76092 (817) 552-6210

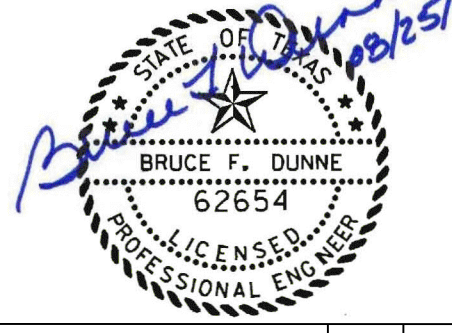
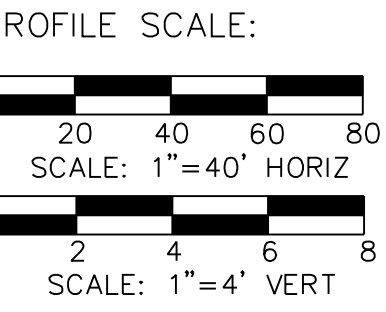


PAVING, DRAINAGE, & UTILITY IMPROVEMENTS - VITRUVIAN WAY & PONTE AVENUE



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NO.	REVISION	BY	DATE

Addison! TOWN OF ADDISON
 DALLAS COUNTY, TEXAS

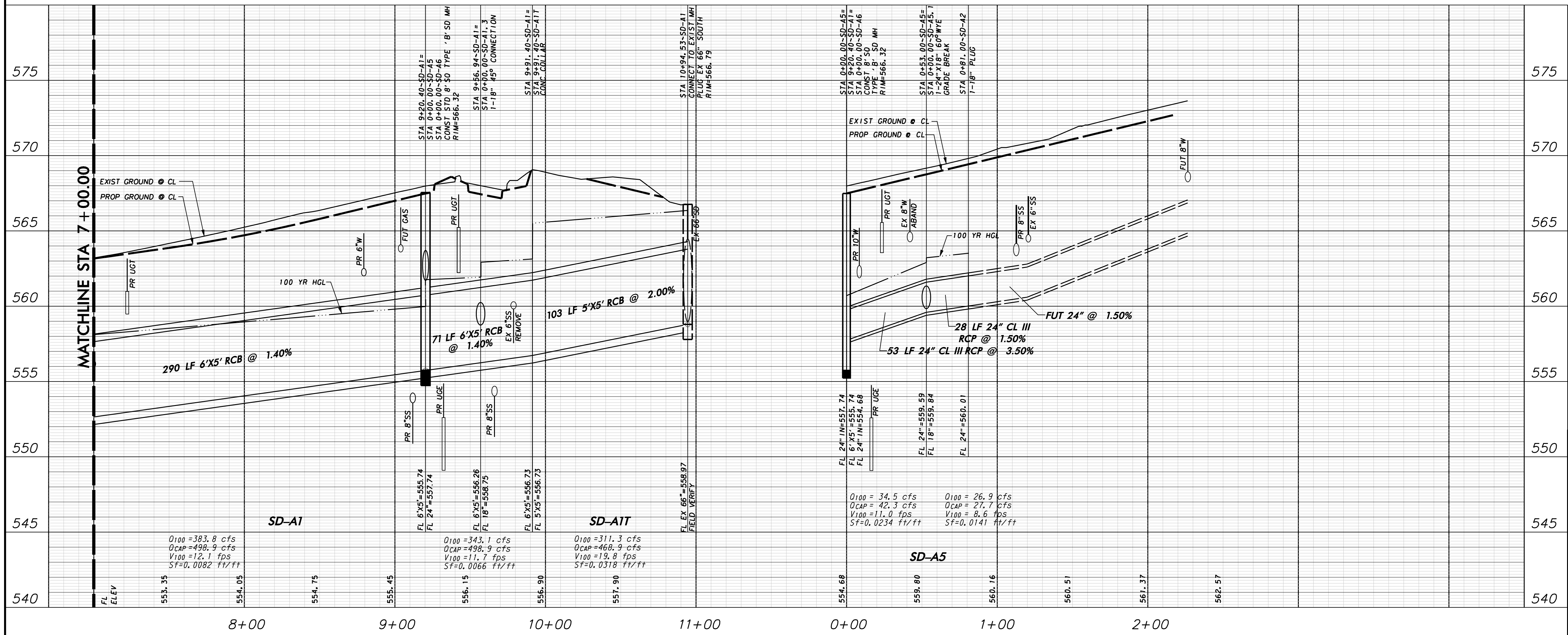
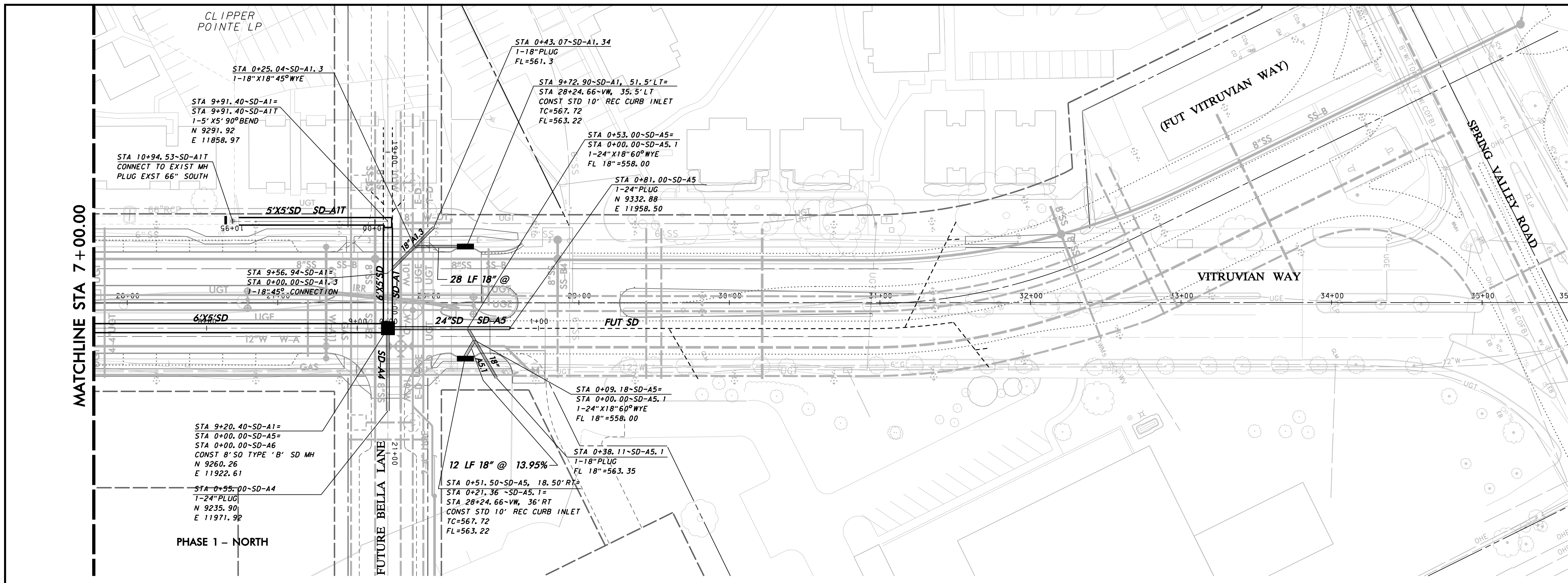
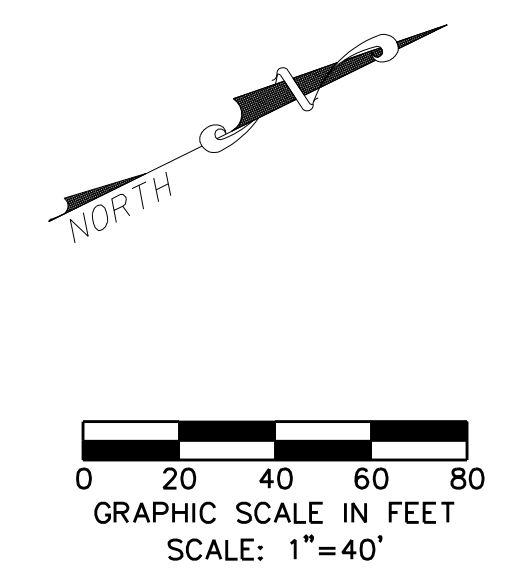
PAVING, DRAINAGE & UTILITY IMPROVEMENTS
 VITRUVIAN WAY & PONTE AVENUE

STORM DRAIN PLAN & PROFILE-VW
 LINE A1-STA. 0+00.00 TO 7+00.00

icon Consulting Engineers, Inc. 250 W. Southlake Blvd., Suite 117
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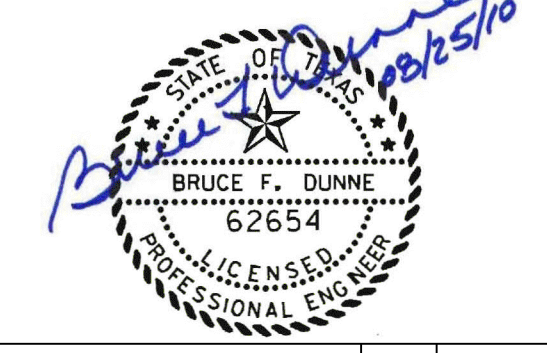
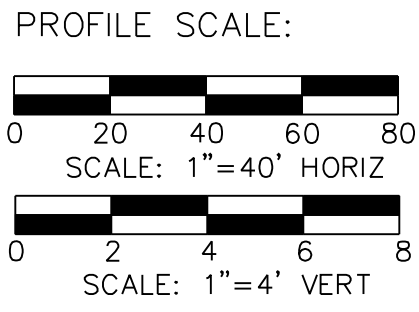
PROJECT	DESIGN	DRAWN	DATE	FILE	SHEET
5029-01	ICE	ICE	MAY 7 2009	FW# 2009-01	33

PAVING, DRAINAGE, & UTILITY IMPROVEMENTS - VITRUVIAN WAY & PONTE AVENUE



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NO.	REVISION	BY	DATE

Addison! TOWN OF ADDISON
 DALLAS COUNTY, TEXAS

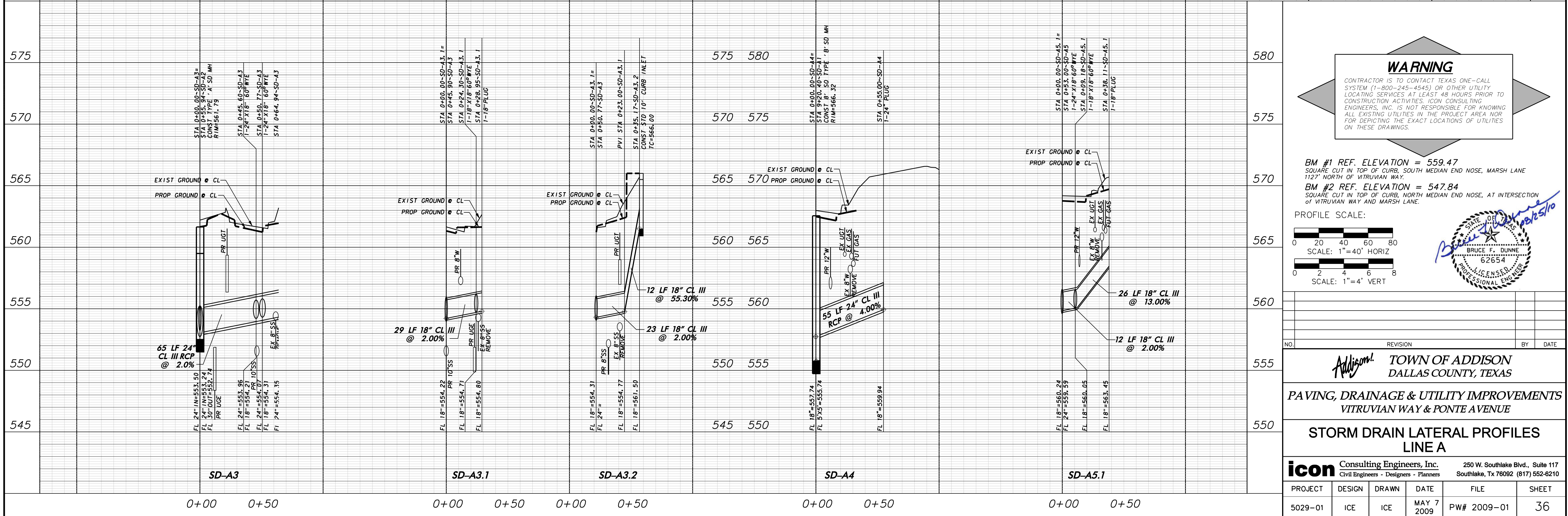
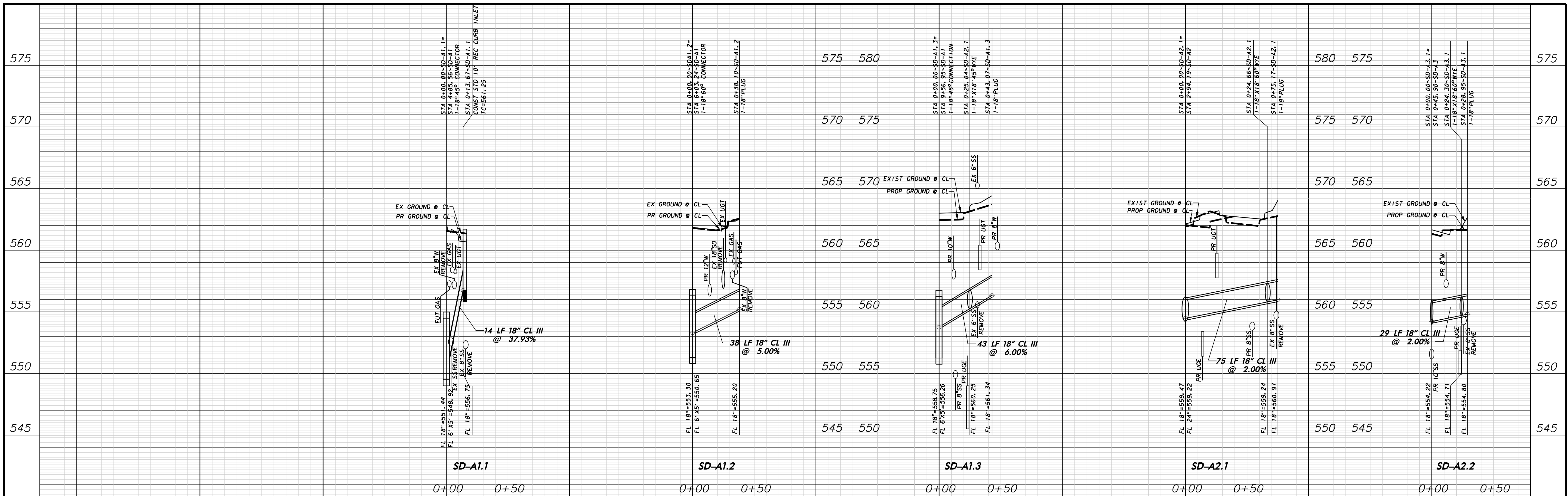
PAVING, DRAINAGE & UTILITY IMPROVEMENTS
 VITRUVIAN WAY & PONTE AVENUE

STORM DRAIN PLAN & PROFILE-VW
 LINE A1-STA. 7+00.00 TO 9+81.00 & LINE A5

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PROJECT	DESIGN	DRAWN	DATE	FILE	SHEET
5029-01	ICE	ICE	MAY 7 2009	PW# 2009-01	34

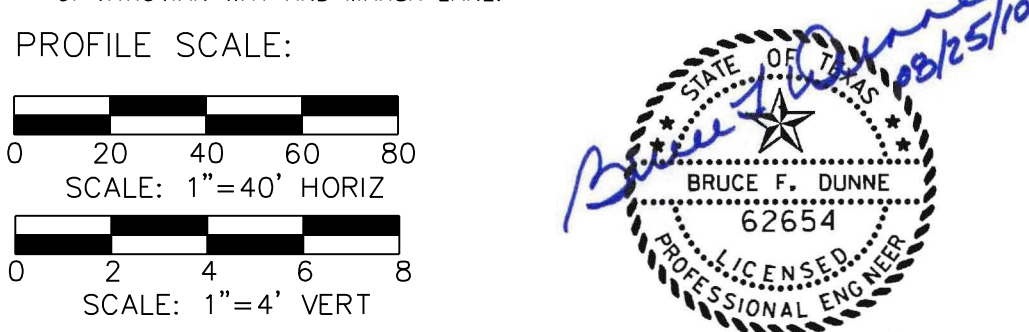
PAVING, DRAINAGE, & UTILITY IMPROVEMENTS - VITRUVIAN WAY & PONTE AVENUE



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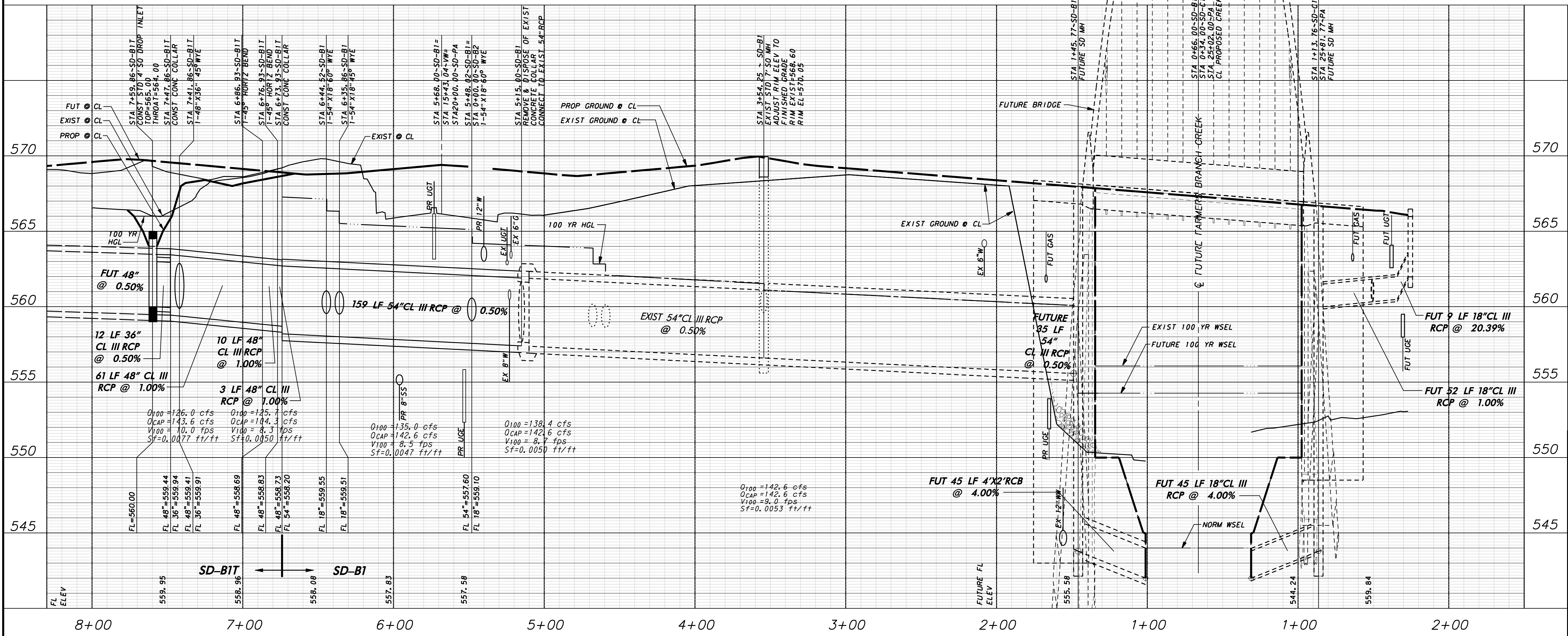
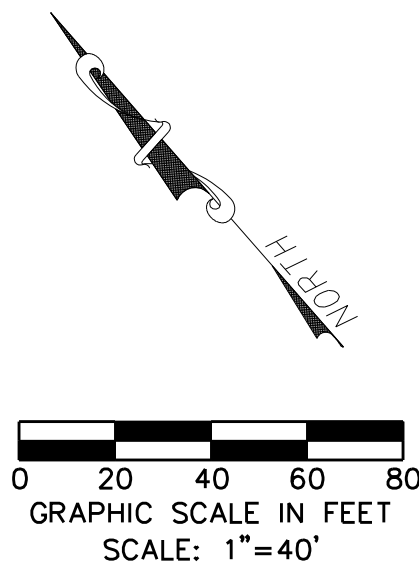
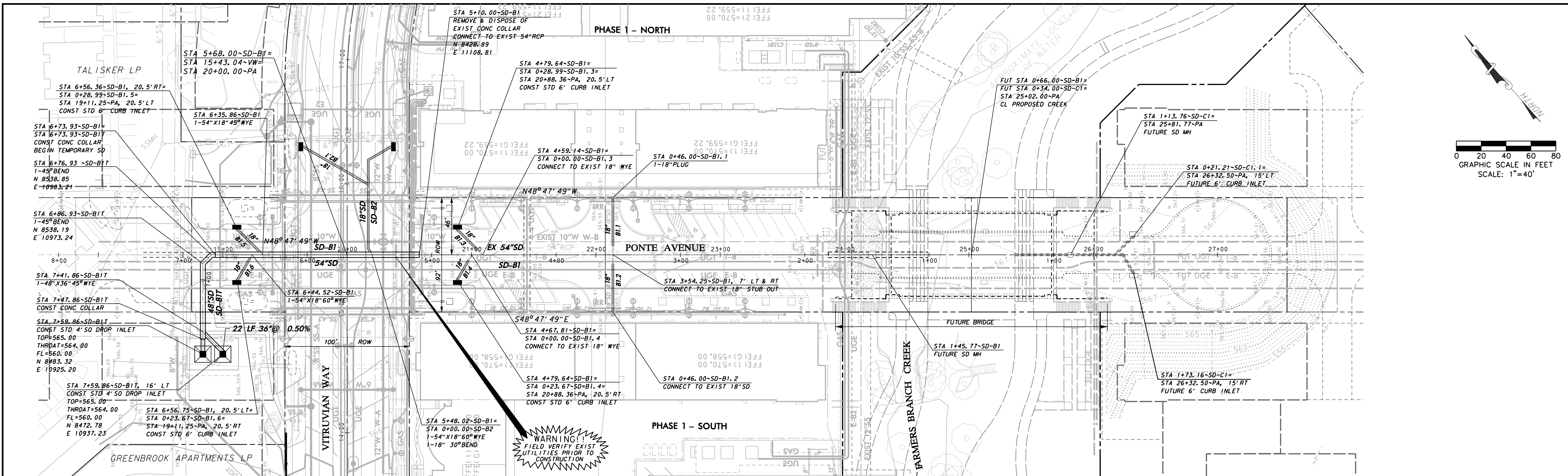
Addison! TOWN OF ADDISON
 DALLAS COUNTY, TEXAS

PAVING, DRAINAGE & UTILITY IMPROVEMENTS
 VITRUVIAN WAY & PONTE AVENUE

**STORM DRAIN LATERAL PROFILES
 LINE A**

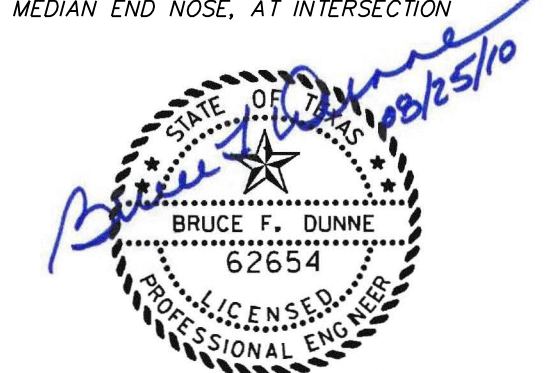
icon Consulting Engineers, Inc.		250 W. Southlake Blvd., Suite 117 Southlake, Tx 76092 (817) 552-6210	
PROJECT	DESIGN	DRAWN	DATE
5029-01	ICE	ICE	MAY 7 2009
FILE	SHEET		
PW# 2009-01	36		

PAVING, DRAINAGE, & UTILITY IMPROVEMENTS - VITRUVIAN WAY & PONTE AVENUE



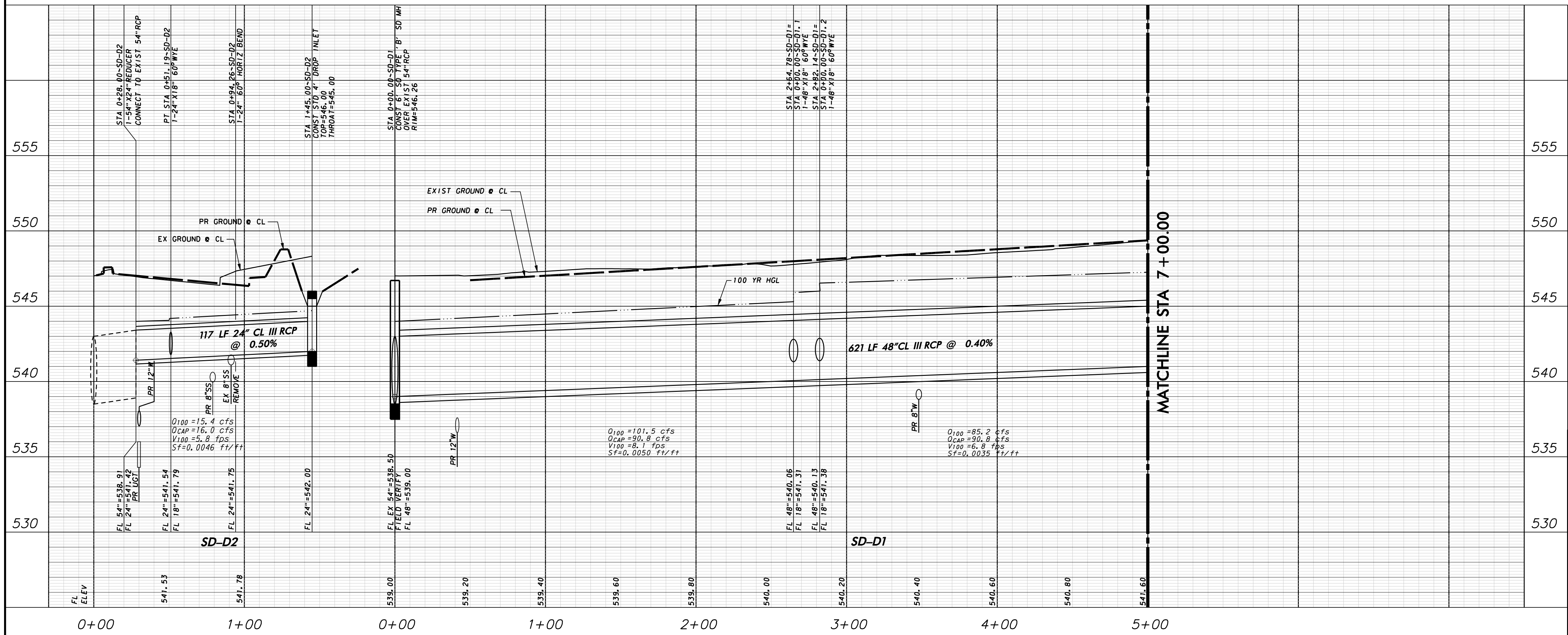
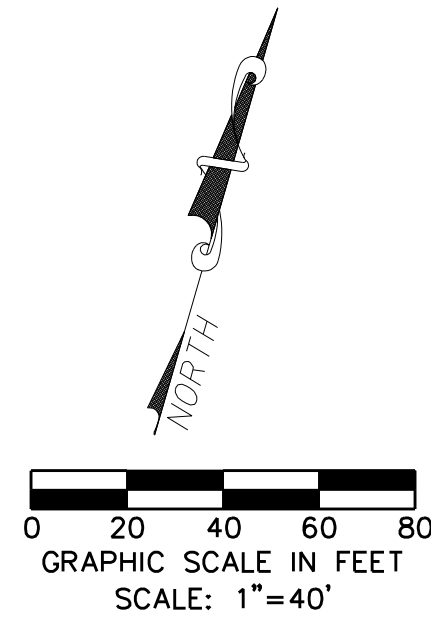
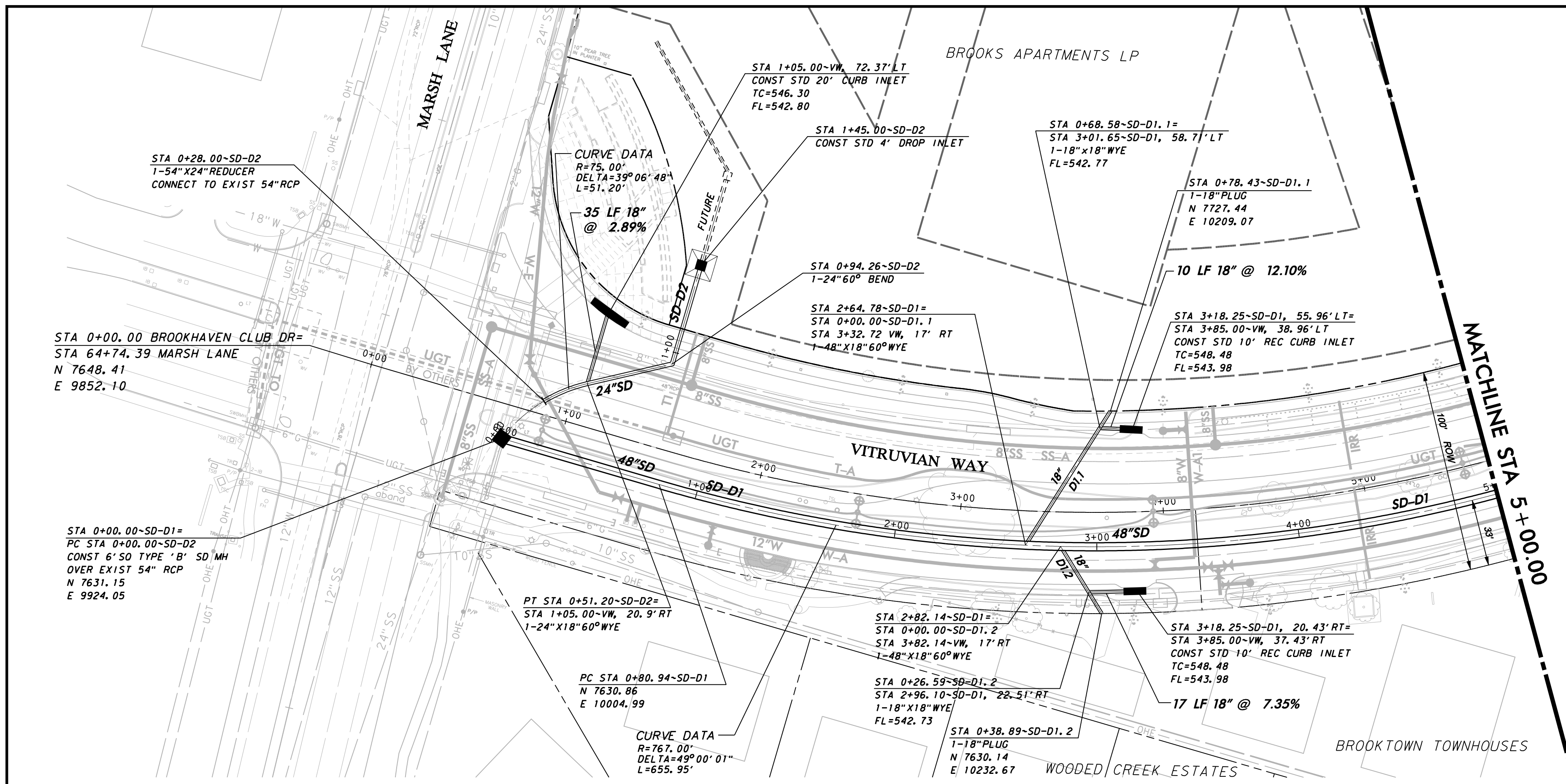
WARNING
 CONTRACTOR IS TO CONTACT TEXAS ONE-CALL SYSTEM (1-800-245-4545) OR OTHER UTILITY LOCATING SERVICES AT LEAST 48 HOURS PRIOR TO CONSTRUCTION ACTIVITIES. ICON CONSULTING ENGINEERS, INC. IS NOT RESPONSIBLE FOR KNOWING ALL EXISTING UTILITIES IN THE PROJECT AREA NOR FOR DEPICTING THE EXACT LOCATIONS OF UTILITIES ON THESE DRAWINGS.

BM #1 REF. ELEVATION = 559.47
 SQUARE CUT IN TOP OF CURB, SOUTH MEDIAN END NOSE, MARSH LANE 1127' NORTH OF VITRUVIAN WAY.
 BM #2 REF. ELEVATION = 547.84
 SQUARE CUT IN TOP OF CURB, NORTH MEDIAN END NOSE, AT INTERSECTION OF VITRUVIAN WAY AND MARSH LANE.



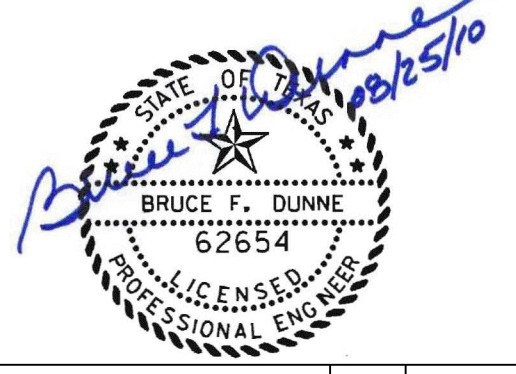
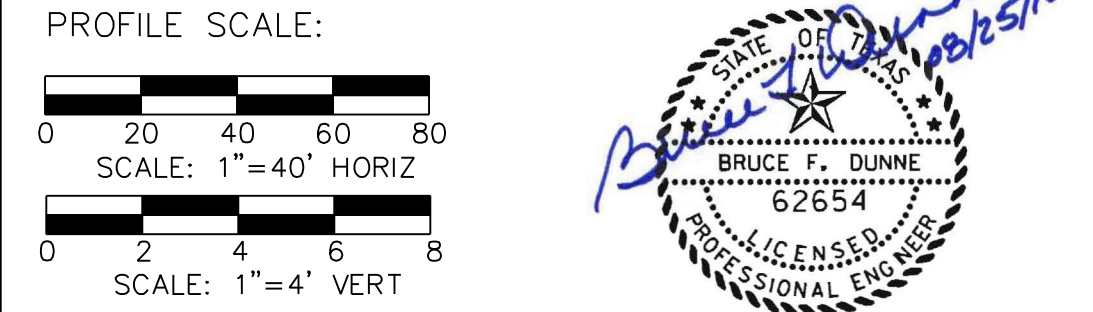
NO.		REVISION		BY		DATE	
Addison! TOWN OF ADDISON DALLAS COUNTY, TEXAS PAVING, DRAINAGE & UTILITY IMPROVEMENTS VITRUVIAN WAY & PONTE AVENUE STORM DRAIN PLAN & PROFILE-PA LINES B1 & C1							
icon		Consulting Engineers, Inc.		250 W. Southlake Blvd., Suite 117		Southlake, Tx 76092 (817) 552-6210	
PROJECT	DESIGN	DRAWN	DATE	FILE	SHEET		
5029-01	ICE	ICE	MAY 7 2009	PW# 2009-01	37		

PAVING, DRAINAGE, & UTILITY IMPROVEMENTS - VITRUVIAN WAY & PONTE AVENUE



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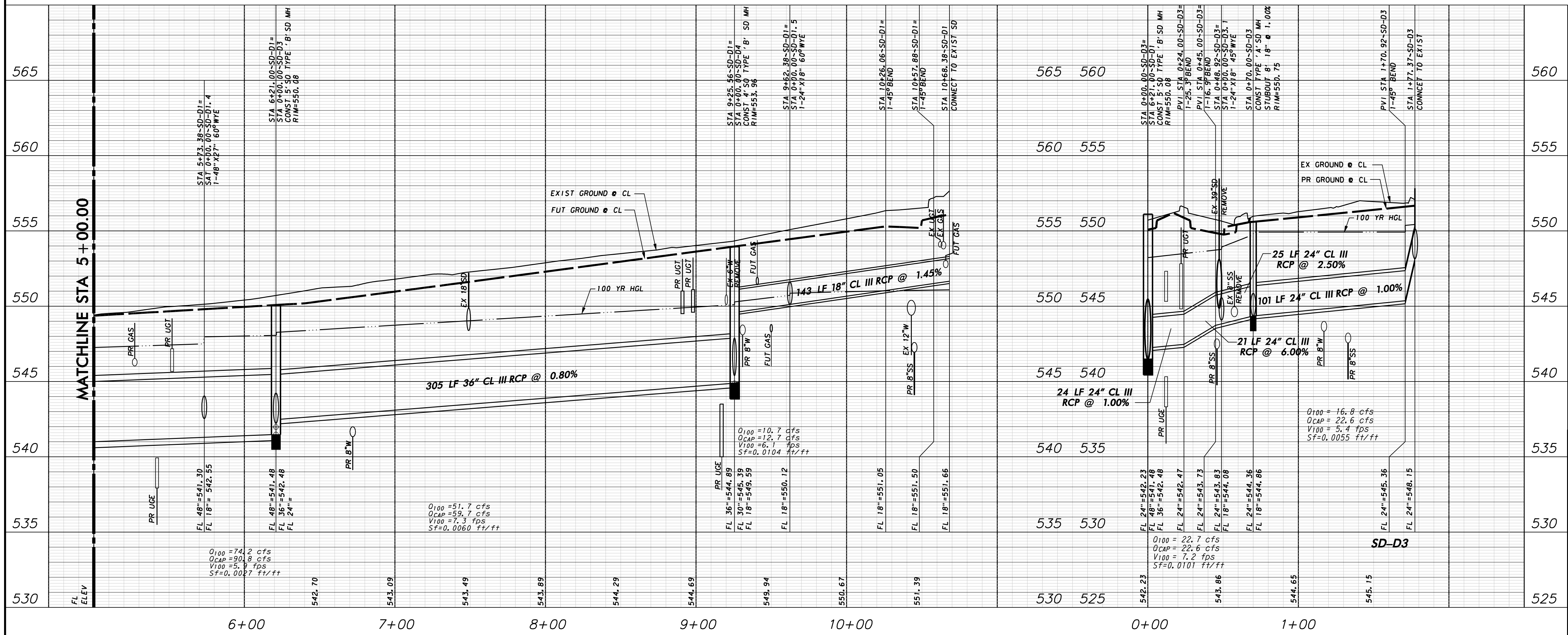
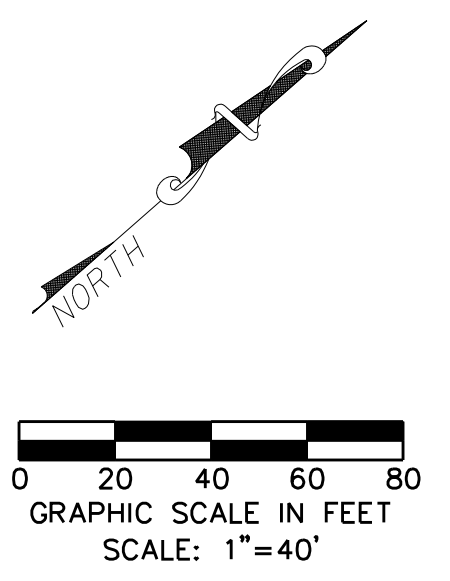
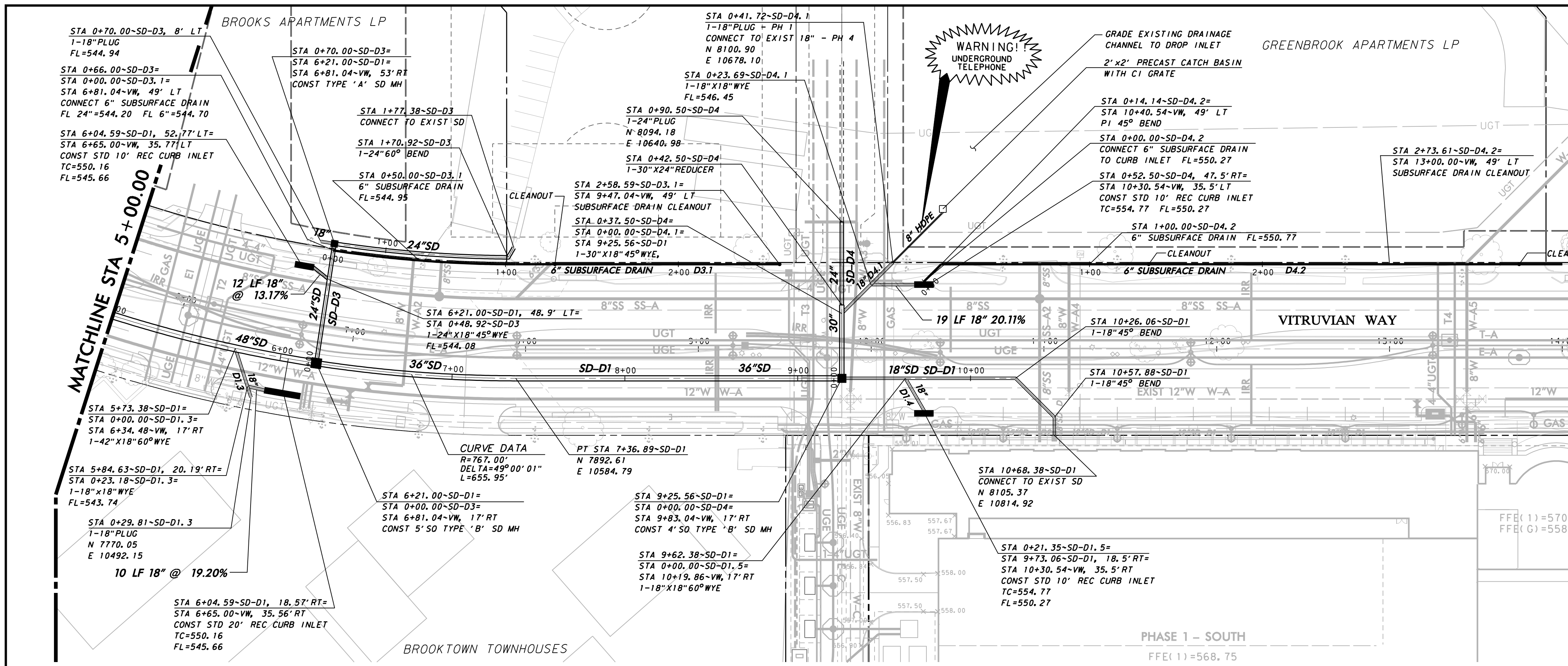
NO.	REVISION	BY	DATE

Addison! TOWN OF ADDISON
DALLAS COUNTY, TEXAS
PAVING, DRAINAGE & UTILITY IMPROVEMENTS
VITRUVIAN WAY & PONTE AVENUE

STORM DRAIN PLAN & PROFILE-VW
LINE D1-STA. 0+00.00 TO 5+00.00 & LINE D2

icon Consulting Engineers, Inc.		250 W. Southlake Blvd., Suite 117 Southlake, TX 76092 (817) 552-6210	
PROJECT	DESIGN	DRAWN	DATE
5029-01	ICE	ICE	MAY 7 2009
FILE	SHEET		
PW# 2009-01	38		

PAVING, DRAINAGE, & UTILITY IMPROVEMENTS - VITRUVIAN WAY & PONTE AVENUE



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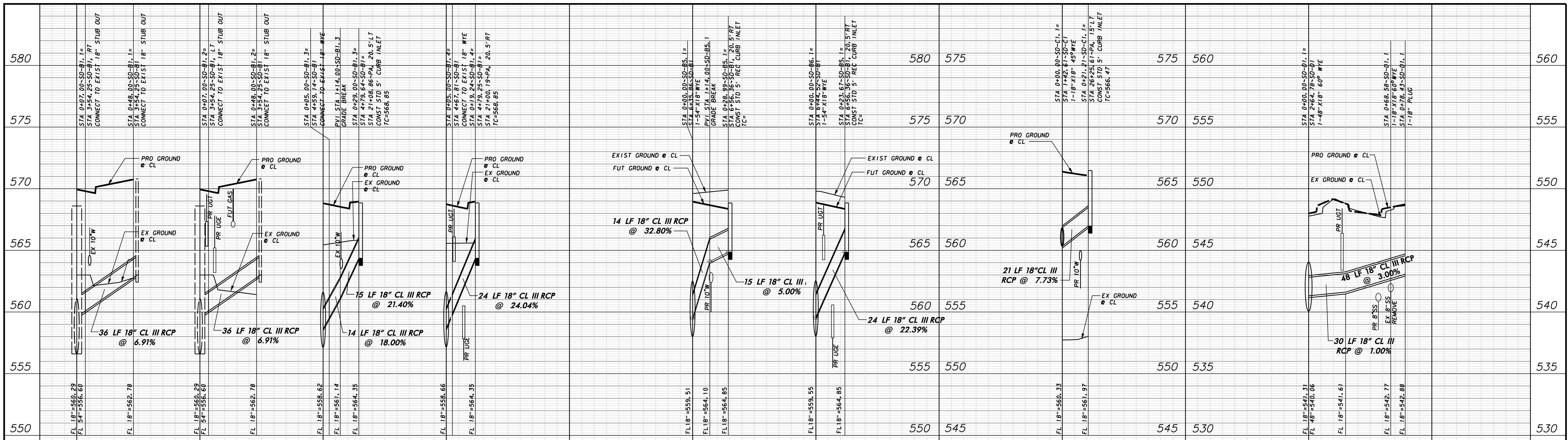
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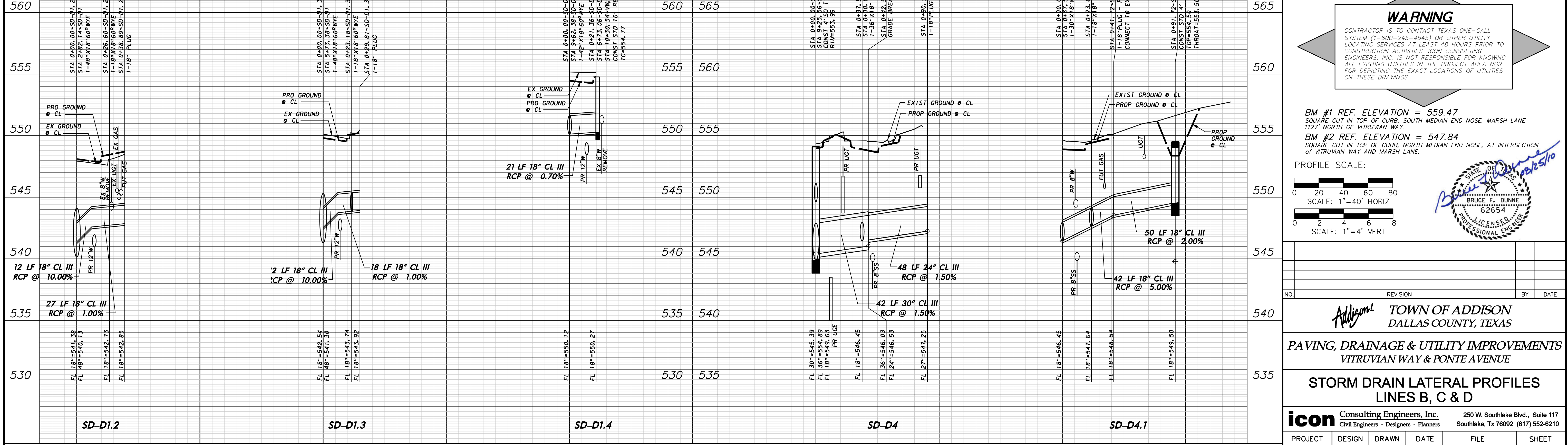
NO.	REVISION	BY	DATE
TOWN OF ADDISON DALLAS COUNTY, TEXAS			
PAVING, DRAINAGE & UTILITY IMPROVEMENTS VITRUVIAN WAY & PONTE AVENUE			
STORM DRAIN PLAN & PROFILE-VW LINE D1-STA. 5+00.00 TO 11+29.06 & LINE D3			
icon Consulting Engineers, Inc. Civil Engineers - Designers - Planners		250 W. Southlake Blvd., Suite 117 Southlake, TX 76092 (817) 552-6210	
PROJECT	DESIGN	DRAWN	DATE
5029-01	ICE	ICE	MAY 7 2009
FILE	SHEET		
PW# 2009-01	39		

PAVING, DRAINAGE, & UTILITY IMPROVEMENTS - VITRUVIAN WAY & PONTE AVENUE



SD-B1.1 SD-B1.2 SD-B1.3 SD-B1.4 SD-B1.5 SD-B1.6 SD-C1.1 SD-D1.1

0+00 0+50 0+00 0+50 0+00 0+50 0+00 0+50 0+00 0+50 0+00 0+50 0+00 0+50 0+00 0+50



SD-D1.2 SD-D1.3 SD-D1.4 SD-D4 SD-D4.1

0+00 0+50 0+00 0+50 0+00 0+50 0+00 0+50 0+00 0+50 1+00 1+00

WARNING

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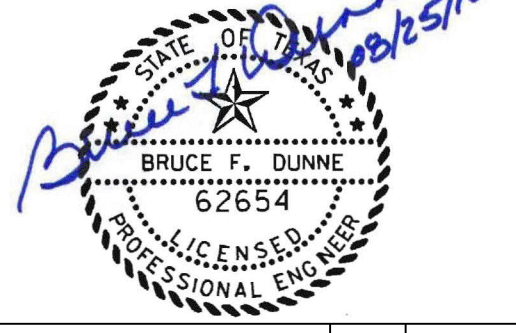
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PROFILE SCALE:

0 20 40 60 80
 SCALE: 1"=40' HORIZ

0 2 4 6 8
 SCALE: 1"=4' VERT



NO. REVISION BY DATE

Addison! TOWN OF ADDISON
 DALLAS COUNTY, TEXAS

PAVING, DRAINAGE & UTILITY IMPROVEMENTS
 VITRUVIAN WAY & PONTE AVENUE

**STORM DRAIN LATERAL PROFILES
 LINES B, C & D**

icon Consulting Engineers, Inc.		250 W. Southlake Blvd., Suite 117 Southlake, Tx 76092 (817) 552-6210	
PROJECT	DESIGN	DRAWN	DATE
5029-01	ICE	ICE	MAY 7 2009
			FILE
			PW# 2009-01
			SHEET
			41

PAVING, DRAINAGE, & UTILITY IMPROVEMENTS - VITRUVIAN WAY & PONTE AVENUE