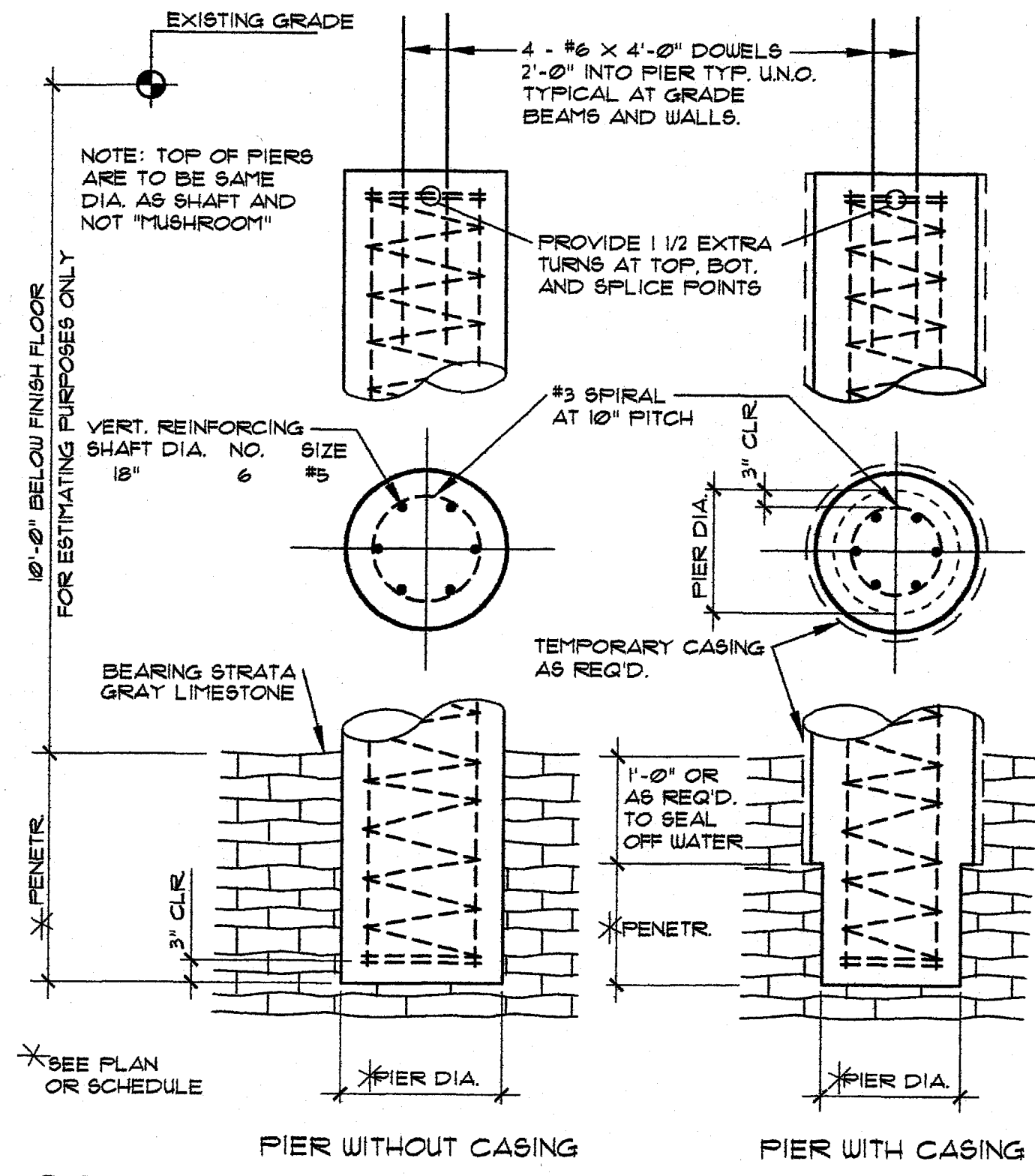


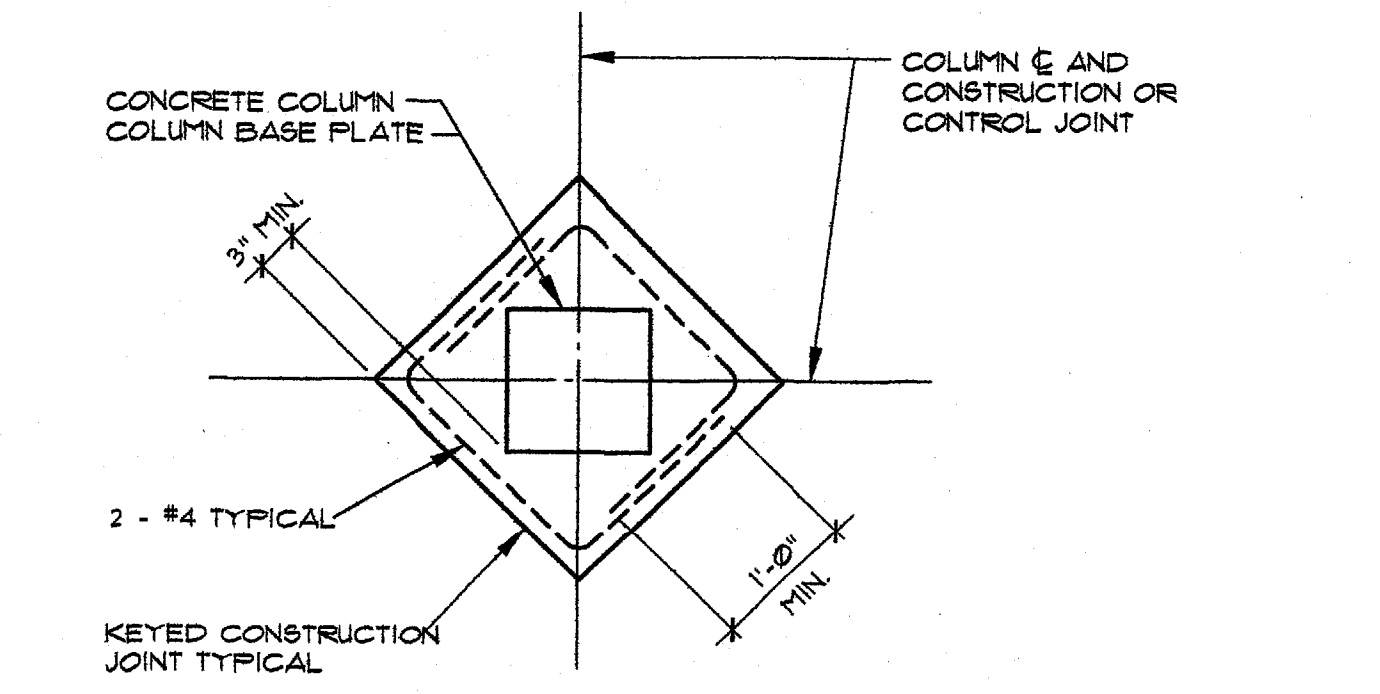
GENERAL NOTES

- THIS PROJECT SHALL MEET ALL REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE AND THE CITY OF ADDISON, TEXAS.
- DESIGN LOADS:
ROOF = 20 PSF
WIND LOAD
BASIC WIND SPEED = 90 MPH
W = 1.0
WIND EXPOSURE C
ROOF
NET UPLIFT IS PSF
- FOUNDATIONS ARE DESIGNED TO MEET THE RECOMMENDATIONS CONTAINED IN A REPORT PREPARED FOR THIS PROJECT BY CHJ ENGINEERING, INC. DATED MARCH 2009.
- FOUNDATION DESIGN IS BASED ON AN ALLOWABLE BEARING VALUE OF 40,000 PSF AND AN ALLOWABLE SIDE FRICTION VALUE OF 6,500 PSF IN GRAY LIMESTONE, AT AN AVERAGE OF 10 FEET BELOW EXISTING GRADE. DRILLED PIER SHAFT SIZES SHOWN ARE MINIMUM PER STRUCTURAL REQUIREMENTS.
- ALL FILL MATERIAL SHALL HAVE A MINIMUM PLASTICITY INDEX OF 4 AND A MAXIMUM PLASTICITY INDEX OF 12 WITH A LIQUID LIMIT OF 30 OR LESS, AND SHALL BE COMPACTED TO A MINIMUM DENSITY OF 95% OF ASTM D698 (STANDARD PROCTOR) UNLESS OTHERWISE NOTED.
- ALL PIERS SHALL BE CENTERED UNDER COLUMNS UNLESS OTHERWISE SHOWN.
- SHOP DRAWINGS: TWO PRINTS AND ONE SET OF TRACING OF EACH DRAWING ARE TO BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. DISTRIBUTION OF PRINTS IS TO BE MADE ONLY FROM RETURNED SETS BEARING A SIGNED REVIEW STAMP. NO WORK ON ITEMS SHOWN THEREON IS TO PROCEED UNLESS THE STAMP CLEARLY INDICATES "NO EXCEPTIONS TAKEN" OR "MAKE CORRECTIONS NOTED". GENERAL CONTRACTOR SHALL PRECHECK ALL SHOP DRAWINGS BEFORE SUBMISSION TO ENGINEER FOR REVIEW. THE CONTRACTOR SHALL ALLOW THE ENGINEER TWO WEEKS FOR REVIEWING SHOP DRAWINGS.
- STRUCTURAL DRAWINGS MAY NOT BE USED AS SHOP DRAWINGS.
- PROVIDE ALL CONCRETE PADS, TRAPS, BASINS, ETC., SHOWN ON ARCHITECTURAL OR MECHANICAL DRAWINGS WHERE INDICATED TO BE SUPPLIED BY GENERAL CONTRACTOR.
- VERIFY ALL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS. IN CASE OF DISCREPANCY BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS, CONTRACTOR IS TO NOTIFY ARCHITECT AND OBTAIN CLARIFICATION IN WRITING PRIOR TO PROCEEDING.
- ALL CONCRETE AND METAL REINFORCEMENT SHALL BE FABRICATED AND PLACED IN CONFORMITY WITH THE "ACI STANDARD BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318-99).
- FOURD IN PLACE CONCRETE SHALL STRICTLY ADHERE TO THE PROPORTIONS ESTABLISHED IN DESIGN MIXES, CONSISTING OF THE ACTUAL MATERIALS TO BE USED DURING CONSTRUCTION, FOR THE SEVERAL STRENGTHS AND USES INTENDED. THESE DESIGN MIXES ARE TO BE PREPARED BY A PREQUALIFIED LABORATORY, AND THE MATERIALS AND TEST RESULTS ARE TO BE REVIEWED BY THE ENGINEER AND OWNER'S LAB PRIOR TO USE.
- FOURD IN PLACE CONCRETE IS TO BE NORMAL WEIGHT AND IS TO DEVELOP A COMPRESSIVE STRENGTH f'_c OF 3000 PSI AT 28 DAYS.
- UNLESS NOTED OTHERWISE, METAL REINFORCEMENT FOR FOURD IN PLACE CONCRETE IS TO BE ASTM A-615, GRADE 60, WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.
- UNLESS OTHERWISE SHOWN, ALL SLABS AND STEPS ON FILL SHALL BE REINFORCED WITH #3 BARS AT 12" CENTERS EACH WAY, SUPPORTED TWO INCHES FROM THE TOP OF SLAB, LAP 10" AT SPLICES.
- THE CONTRACTOR SHALL VERIFY DEPTHS OF PIERS BEFORE PIER STEEL IS CUT. PIER STEEL SHALL BE DELIVERED TO THE JOB SITE IN STANDARD 60"-0" LENGTHS AND CUT AS REQUIRED. CLASS B LAPS WILL BE ALLOWED IN THE PIER STEEL. NO MORE THAN 90% OF THE BARS ARE TO BE LAPPED IN ANY 3'-0" LENGTH OF THE PIER.
- ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERRECTED IN CONFORMITY WITH THE REQUIREMENTS OF THE 9TH EDITION, AISC MANUAL OF STEEL CONSTRUCTION.
- UNLESS OTHERWISE SHOWN OR NOTED, CONNECTIONS AT NONCONTINUOUS JOINTS SHALL BE DETAILED AS REQUIRED BY PART 4, CONNECTIONS FOR THE END REACTION FOR BEAMS AS TABULATED IN PART 2. FIELD CONNECTIONS AT NONCONTINUOUS JOINTS ARE TO BE BOLTED OR WELDED, SHOP CONNECTIONS TO BE WELDED.
- EXCEPT AS SHOWN OR NOTED, ALL STRUCTURAL SHAPES ARE TO BE ASTM A-992 OR 50 MATERIAL. TUBE STEEL TO BE FT 46 K51, ASTM A500, GRADE B. STRUCTURAL PLATES AND MISCELLANEOUS STEEL SHALL BE ASTM A36 MATERIAL.
- ALL WELDS SHALL BE MADE ONLY BY PREQUALIFIED WELDERS PER AWS D11 CERTIFIED WITHIN THE LAST 12 MONTHS. ALL WELDS SHALL BE MADE USING E70 ELECTRODES. ALL WELDS TO BE MINIMUM 3/16 INCH FILLET CONTIGUOUS WELDS UNLESS NOTED OTHERWISE.
- ERECTION TOLERANCES SHALL CONFORM TO THE AISC CODE OF STANDARD PRACTICE EXCEPT THAT THE MAXIMUM TOTAL DISPLACEMENT AT THE CENTERLINES OF ANY COLUMN FROM THE ESTABLISHED COLUMN CENTERLINES SHALL NOT EXCEED 1/2 INCH AT ANY LEVEL.
- A RECOGNIZED TESTING LABORATORY, REVIEWED BY THE STRUCTURAL ENGINEER, SHALL BE ENGAGED FOR THE PURPOSE OF SHOP AND FIELD INSPECTION. THE LABORATORY SHALL ASSURE THAT APPROVED WELDING MATERIALS AND SEQUENCES ARE USED, AND SHALL CERTIFY IN WRITING THAT THE QUALITY AND STRENGTH REQUIREMENTS OF ALL CONNECTIONS HAVE BEEN ATTAINED AND THAT ALL TOLERANCES ARE WITHIN SPECIFIED LIMITS.
- PROVIDE BOLTS AND HUNCH HOLES IN STRUCTURAL AND MISCELLANEOUS METAL FOR ATTACHMENT OF WOOD NAILERS AS REQUIRED ON THE ARCHITECTURAL, MECHANICAL OR STRUCTURAL DRAWINGS.
- ALL STRUCTURAL AND MISCELLANEOUS METAL IS TO BE CLEANED PRIOR TO SHOP PAINTING AND SHIPMENT IN ACCORD WITH THE STRUCTURAL STEEL PAINTING COUNCIL REQUIREMENTS FOR THE FOLLOWING GRADE: POWER TOOL.
- ALL EXPOSED STEEL SHALL BE HOT DIP GALVANIZED AND PAINTED PER ARCHITECTURAL REQUIREMENTS.
- PROVIDE MINIMUM 1/4 INCH CAP PLATE AT ENDS OF ALL TUBE STEEL MEMBERS, UNLESS NOTED OTHERWISE.
- ROOF DECK SHALL BE A 3 1/2" 16 GAGE, VERSA-DEK 35LB, AS MANUFACTURED BY METAL DEK GROUP OR EQUAL, MEETING THE REQUIREMENTS OF THE STEEL DECK INSTITUTE. DECK SHALL BE CONTINUOUS OVER ALL SPANS AND SHALL BE WELDED TO THE SUPPORTING STEEL WITH 5/8" DIAMETER PUDDLE WELDS. WELDS TO BE 6" O.C. AT END LAPS AND INTERMEDIATE SUPPORTS. EQUIVALENT SCREW ATTACHMENT IS ACCEPTABLE.
- SECURE PERIMETER OF DECK TO STRUCTURAL MEMBERS WITH 5/8" DIAMETER PUDDLE WELDS AT 6" O.C. UNLESS SHOWN OTHERWISE. AT PERIMETER MEMBERS RUNNING PARALLEL TO DECK ATTACH WITH 5/8" DIAMETER PUDDLE WELDS AT 12" O.C. (OR SCREWS).
- SIDE LAPS SHALL HAVE MINIMUM OF TWO #10 TKS FASTENERS PER SPAN AT MAXIMUM SPACING OF 3'-0" O.C.
- WHERE PARTIAL PANELS MAY BE REQUIRED TO COMPLETE DECK INSTALLATION AT PERIMETER OF STRUCTURE, PROVIDE WELDS IN EACH FLUTE TO STRUCTURAL MEMBERS.
- CEILING LIGHT FIXTURES, DUCTS, ETC., SHALL NOT BE SUPPORTED BY STEEL DECK.



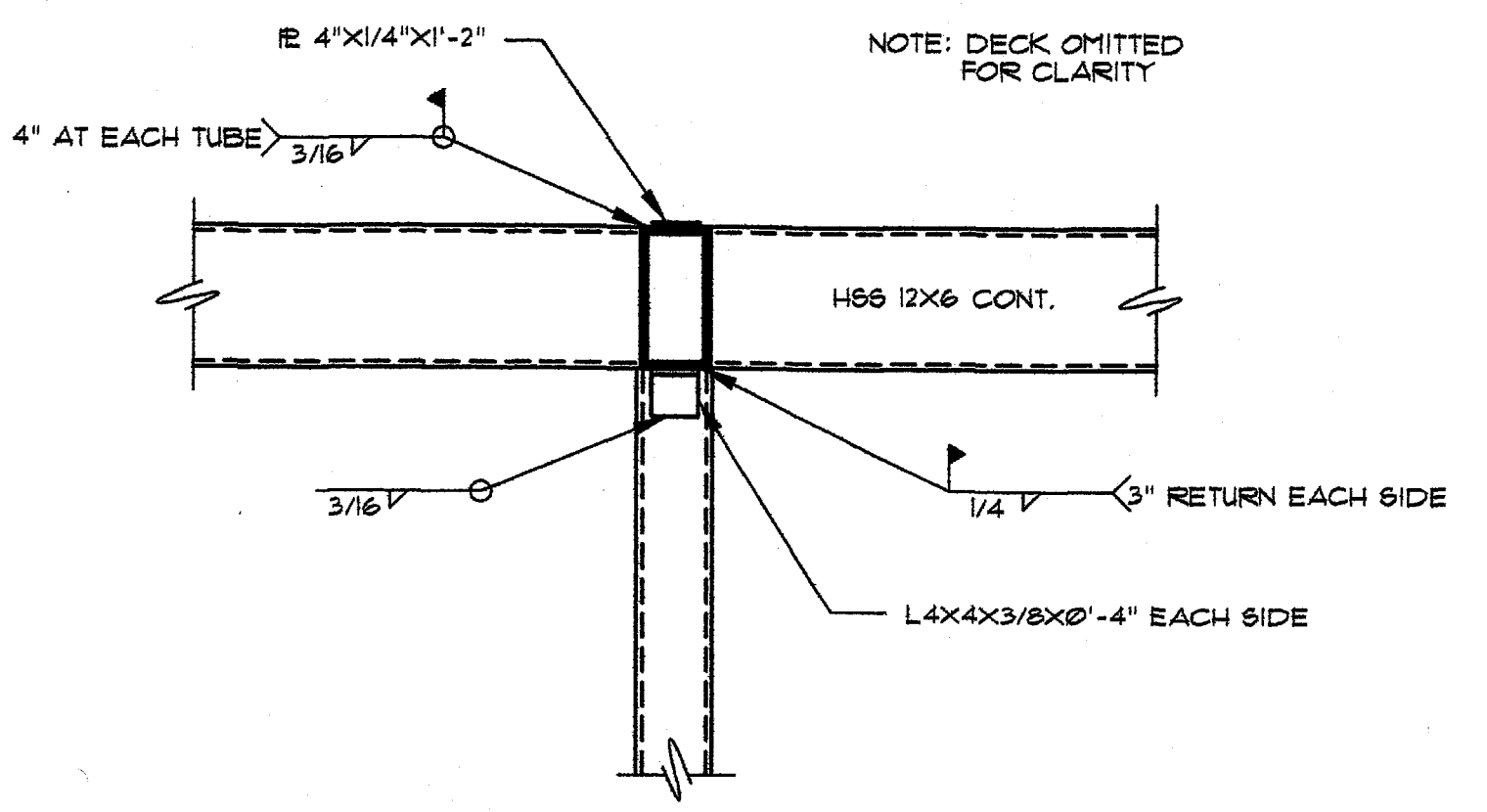
01 TYPICAL DRILLED PIER

NOT TO SCALE



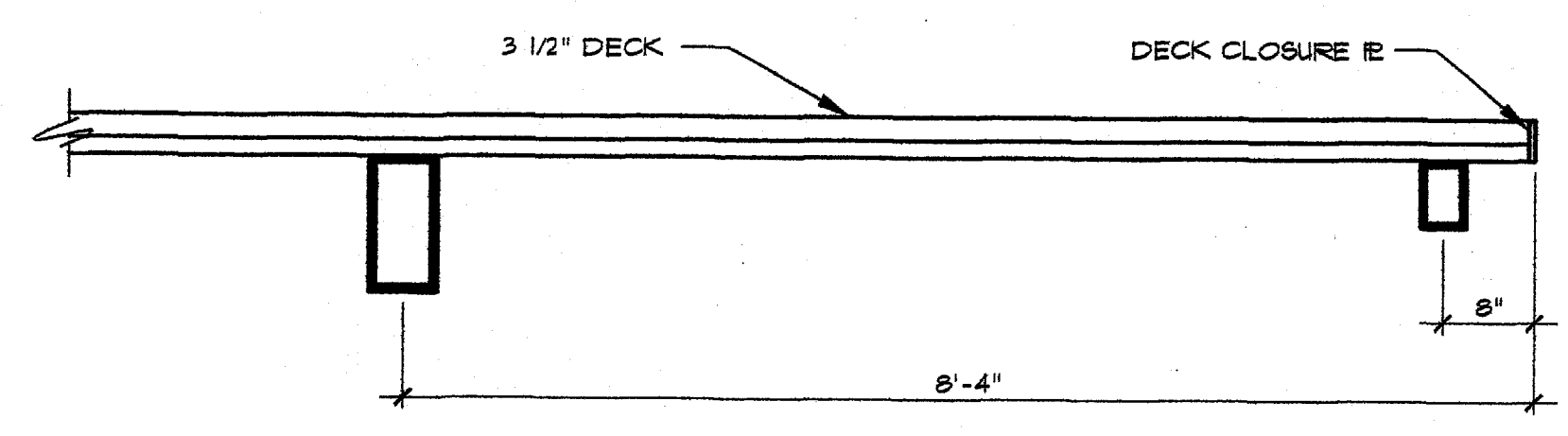
02 TYPICAL INT. COL. BLOCK-OUT

NOT TO SCALE



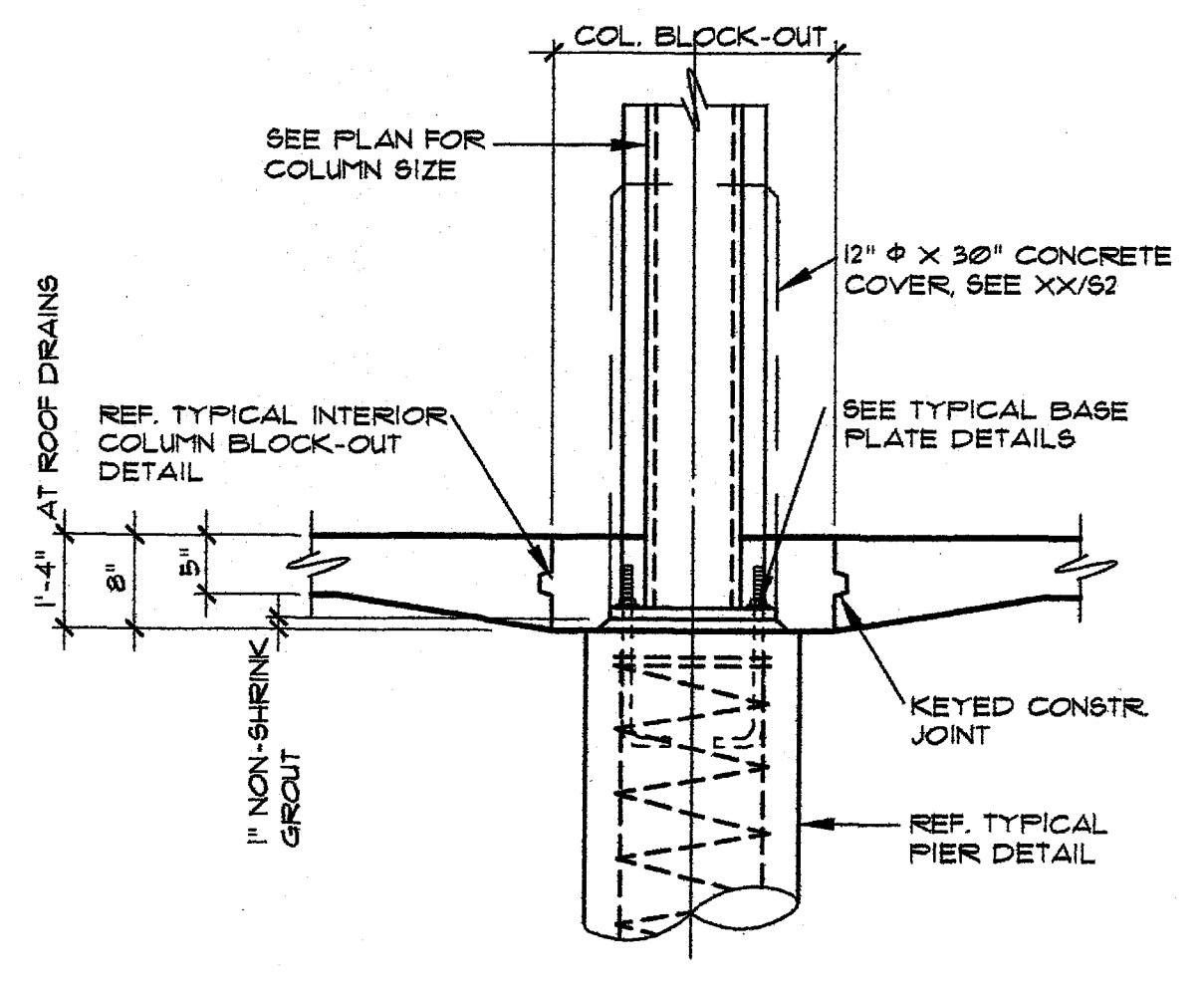
05 SECTION

3/4" x 1'-0"



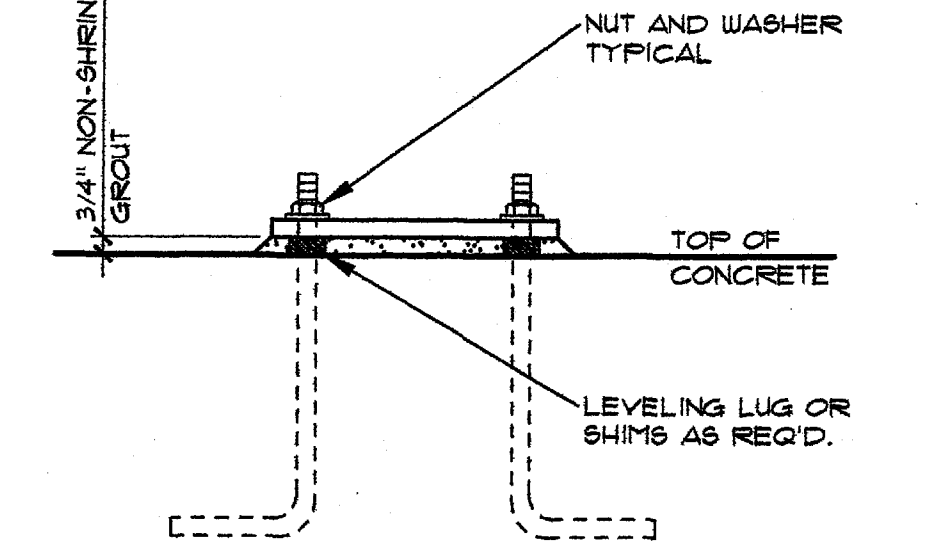
07 SECTION

3/4" x 1'-0"



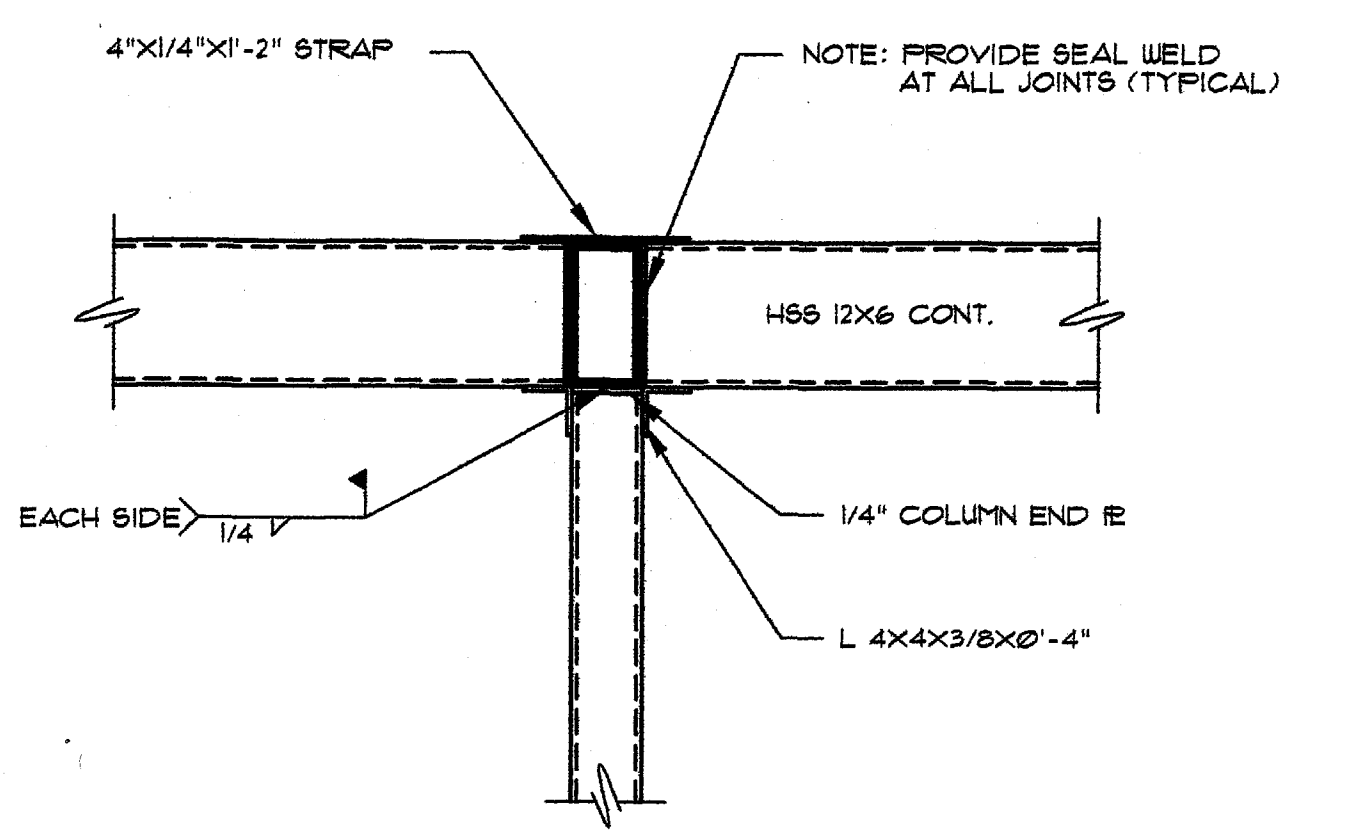
03 TYPICAL COLUMN TO PIER

NOT TO SCALE



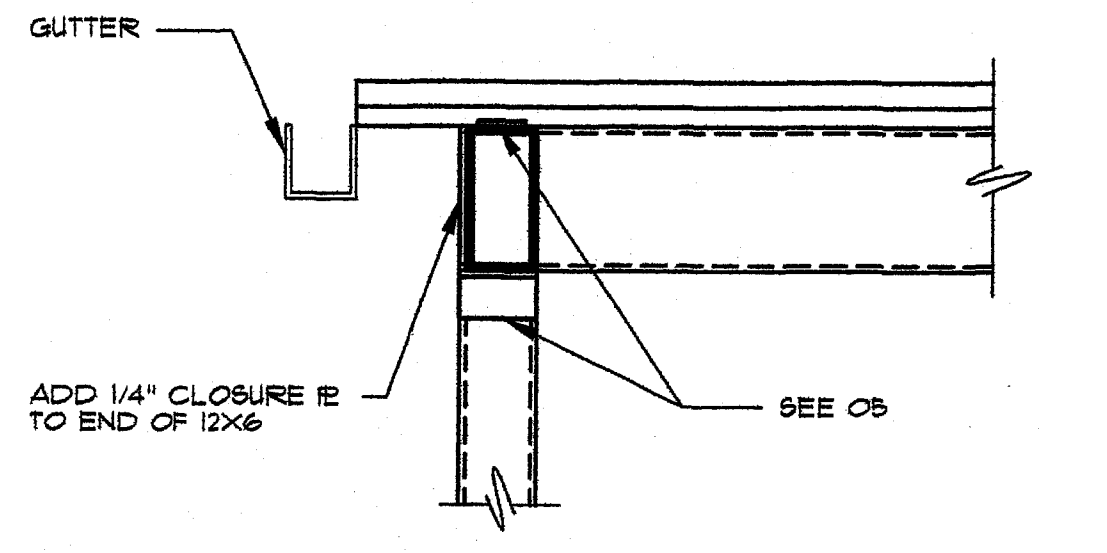
04 TYPICAL BASE PLATE DETAILS

NOT TO SCALE



06 SECTION

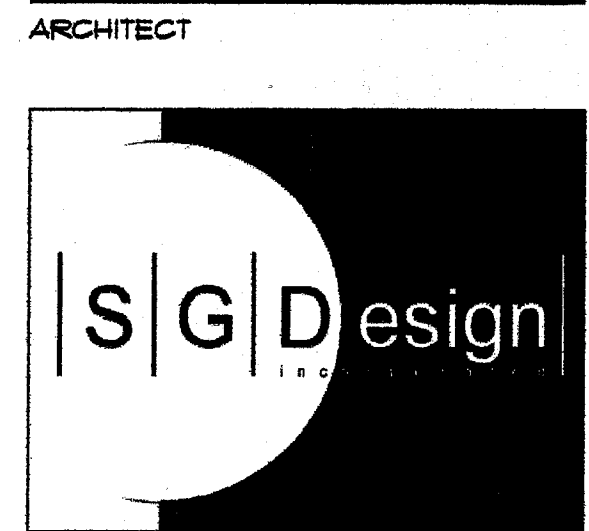
3/4" x 1'-0"



08 SECTION

3/4" x 1'-0"

PROJECT:
UNITED STATES ENVIRONMENTAL PROTECTION AGENCY
16650 WESTGROVE DRIVE SUITES 200 & 300 ADDISON, TEXAS 75001

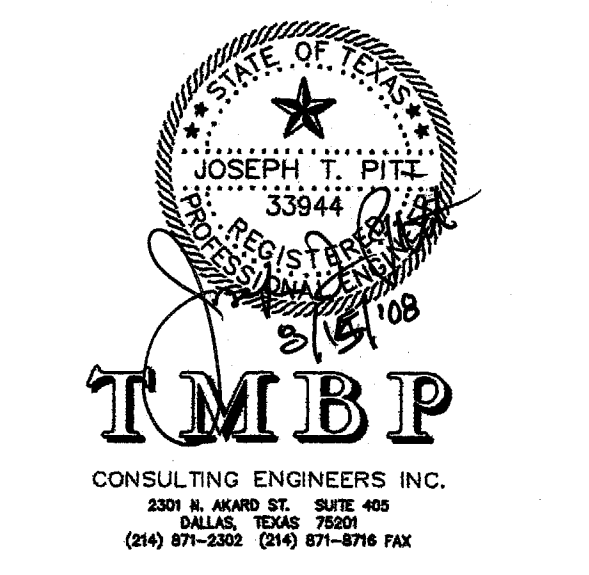


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ARCHITECT'S SEAL:

SIGNATURE _____
SEALED _____

CONSULTANT:



ISSUE HISTORY:

CLIENT 25% REVIEW	02/02/07
CLIENT 50% REVIEW	02/21/07
CLIENT 100% REVIEW	10/19/07
FOR BID	08/15/08

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FILE:
CURRENT ISSUE:
DRAWN BY: RGM
CHECKED BY: JTP

SHEET NUMBER:
GENERAL NOTES AND DETAILS
S1.00