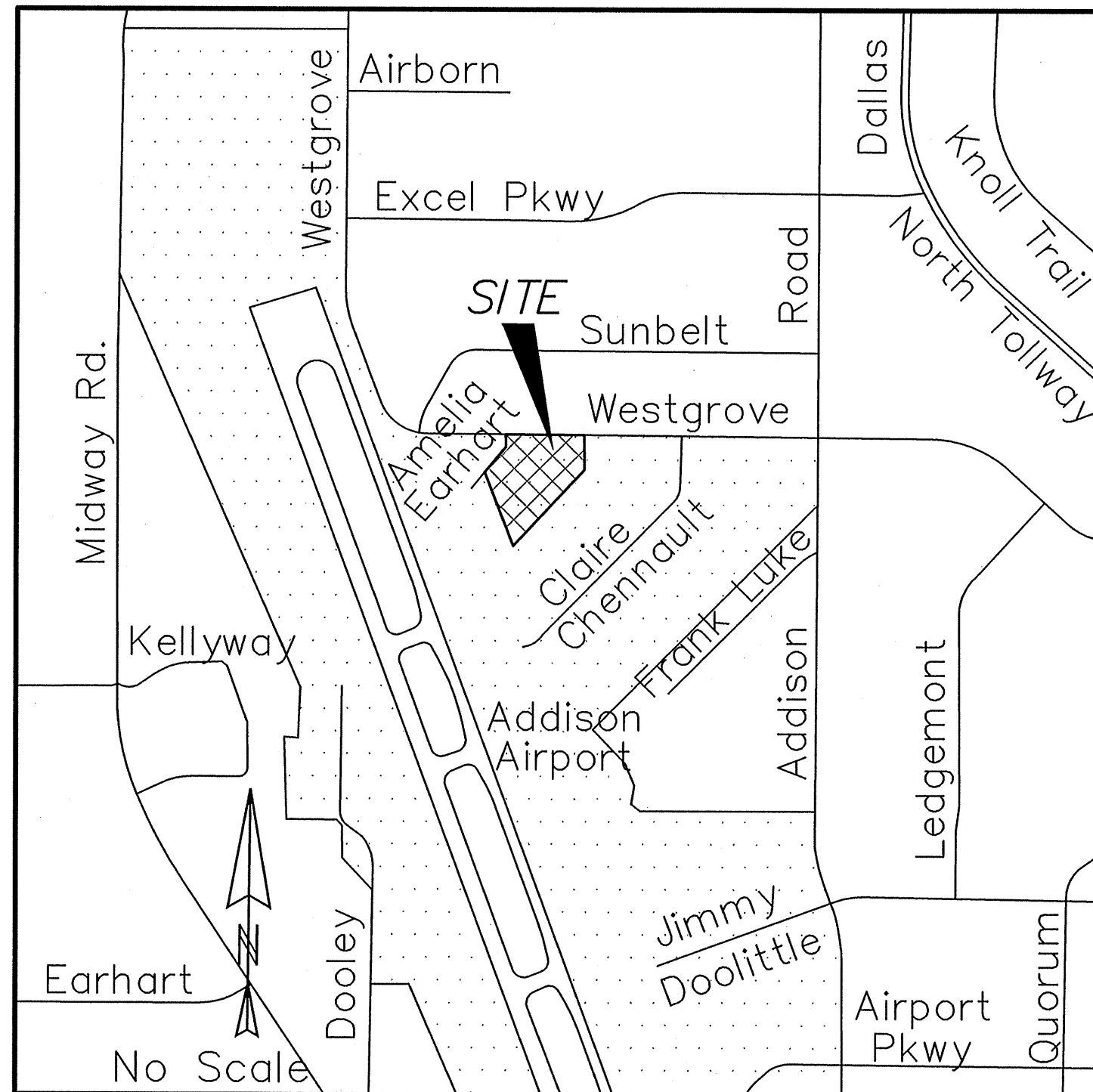


# CONSTRUCTION PLANS FOR MILLION AIR TERMINAL / HANGAR - PHASE 2 LEASE PARCEL No. 70 - WESTGROVE ROAD TOWN OF ADDISION, DALLAS COUNTY, TEXAS



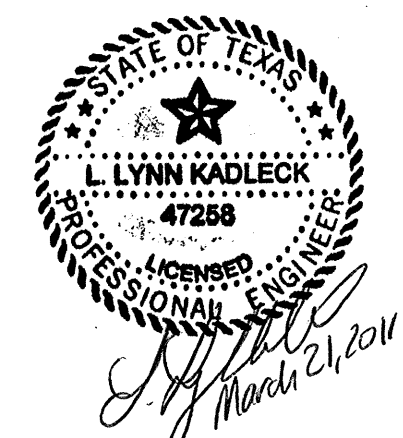
VICINITY MAP

## SHEET INDEX

SHEET NO.	DESCRIPTION
1	COVER
2	DEMOLITION PLAN
3	DIMENSIONAL CONTROL PLAN
4	DRAINAGE AREA MAP
5	GRADING & PAVING PLAN
6	CIRCLE AREA GRAD/PAVING
7	RAMP JOINT PLAN
8 - 10	UTILITY PLAN & PROFILES
11 & 12	EROSION CONTROL & NOTES
13 & 14	DETAILS & NOTES

RECORD DRAWING  
*[Signature]*  
DATE: *October 26, 2010*

Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.



1  
14

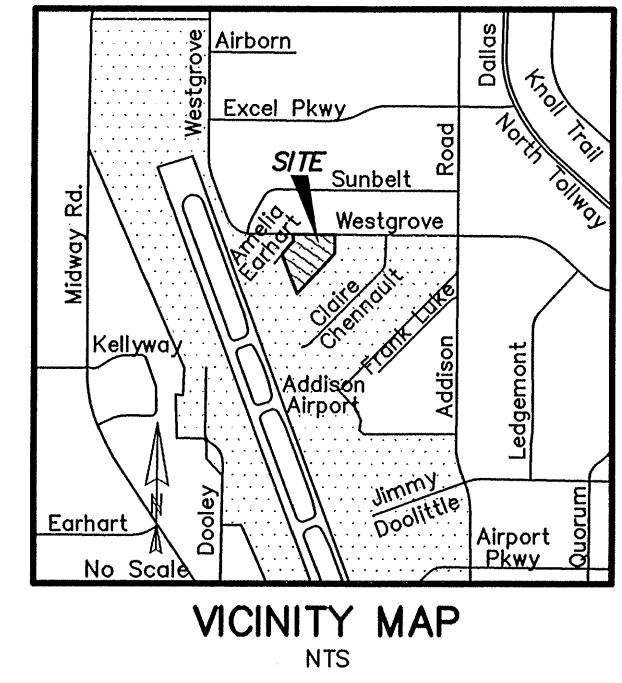
THE MISSION COMPANIES  
P.O. BOX 639  
ADDISION, TEXAS 75001  
PH: (972)248-4500

PREPARED BY  
**KADLECK & ASSOCIATES**  
ENGINEERING PLANNING SURVEYING  
2000 N. CENTRAL EXPY. SUITE 113  
(972) 881-0771 PLANO, TX 75074  
TBPB Reg. No. T-6460 TBPBLS Reg. No. 100555-00

K&A PROJECT #10544

SCALE: 1"=30'

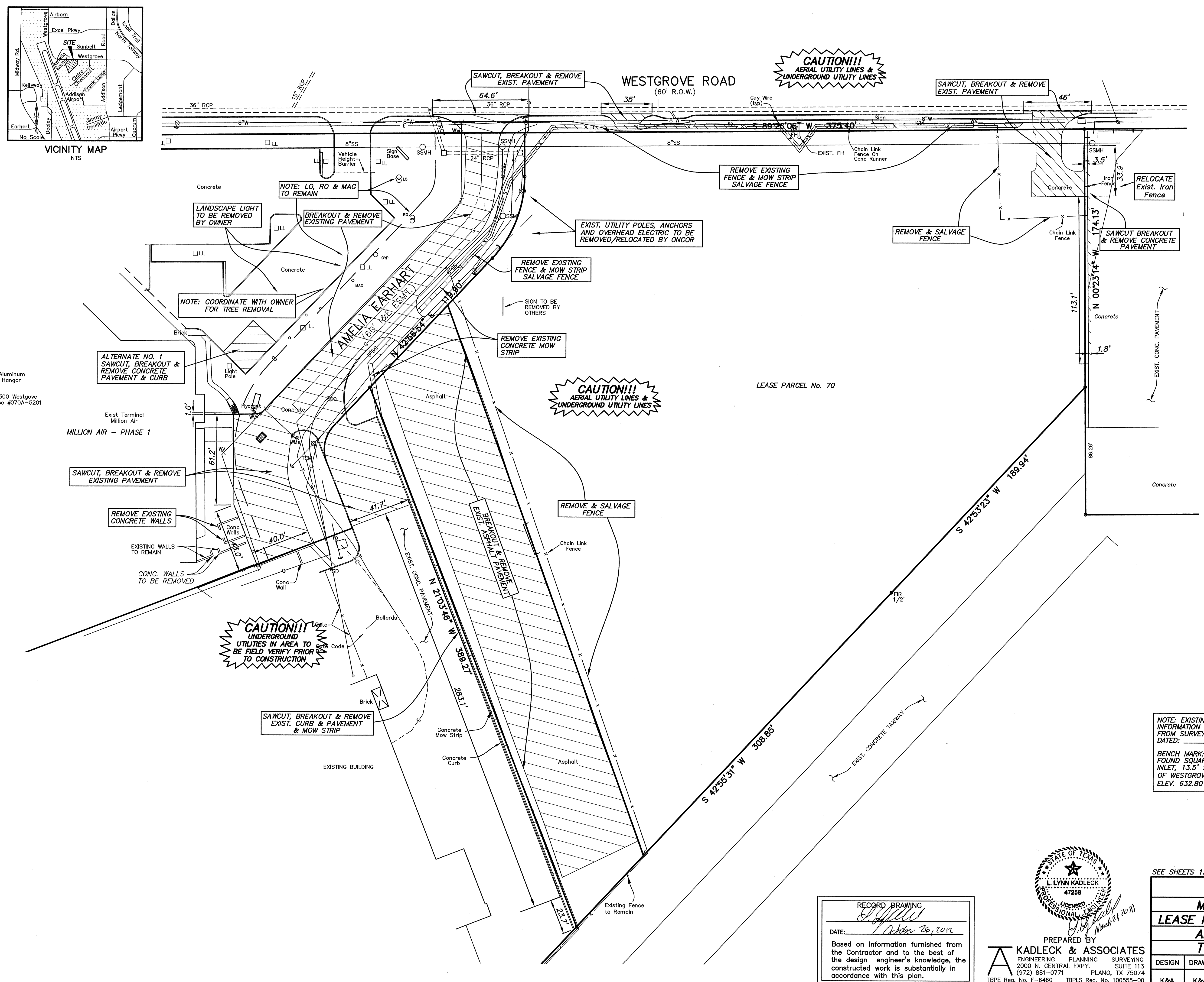
- LEGEND**
- FIR Found Iron Rod
  - EB Electric Box
  - PM Parking Meter
  - TSL Traffic Signal Light
  - LP Light Pole
  - FH Fire Hydrant
  - WM Water Meter
  - MW Monitoring Well
  - TV Telephone Vault
  - SD Storm Drain
  - CM Controlling Monument



VICINITY MAP  
NTS

Aluminum Hangar  
4300 Westgrove  
Lease #070A-5201

EXIST. TERMINAL  
MILLION AIR  
MILLION AIR - PHASE 1



**CAUTION!!!**  
AERIAL UTILITY LINES &  
UNDERGROUND UTILITY LINES

**CAUTION!!!**  
AERIAL UTILITY LINES &  
UNDERGROUND UTILITY LINES

**CAUTION!!!**  
UNDERGROUND  
UTILITIES IN AREA TO  
BE FIELD VERIFY PRIOR  
TO CONSTRUCTION

NOTE: EXISTING FEATURES AND TOPOGRAPHIC INFORMATION SHOWN WAS FURNISHED BY OWNER FROM SURVEY PREPARED BY: BRAD SPARR DATED: \_\_\_\_\_

BENCH MARK:  
FOUND SQUARE CUT AT NORTHWEST CORNER OF BOX INLET, 13.5' SOUTH OF INTERSECTION OF SOUTH LINE OF WESTGROVE ROAD & EAST LINE OF AMELIA EARHART. ELEV. 632.80

RECORD DRAWING  
DATE: *October 26, 2010*  
Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.

PREPARED BY  
**KADLECK & ASSOCIATES**  
ENGINEERING PLANNING SURVEYING  
2000 N. CENTRAL EXPY. SUITE 113  
(972) 881-0771 PLANO, TX 75074  
TBPE Reg. No. F-6460 TBPLS Reg. No. 100555-00

SEE SHEETS 13 & 14 FOR DETAILS AND NOTES

DEMOLITION PLAN				
MILLION AIR - PHASE TWO				
LEASE PARCEL No. 70-WESTGROVE RD.				
ADDISON MUNICIPAL AIRPORT				
TOWN OF ADDISON, TEXAS				
DESIGN	DRAWN	DATE	SCALE	
K&A	K&A	SEPT.2010	1"=30'	PROJECT No. 10544

F:\10544-10555\10544\_Million Air - Westgrove\DWG\3 DEMOLITION PLAN.dwg, 10/21/2010 12:42:17 PM







DRAINAGE AREA TABLE - EXISTING CONDITIONS							
DRAINAGE AREA	AREA (acres)	C	CA	Tc (min)	i 100 (in/hr)	Q 100 (cfs)	COMMENTS
A	0.13	0.90	0.12	10	8.74	1.0	Offsite - exist. pavement
B	0.06	0.30	0.02	15	7.56	0.1	Onsite - turf
C	0.19	0.30	0.06	15	7.56	0.4	Onsite - turf
D(u)	0.06	0.30	0.02	15	7.56	0.1	Onsite - turf
D(l)	0.08	0.90	0.07	10	8.74	0.6	Offsite - exist. pavement
E	0.46	0.30	0.14	15	7.56	1.0	Onsite - turf
F	0.46	0.30	0.14	15	7.56	1.0	Onsite - turf
G	0.10	0.90	0.09	10	8.74	0.8	Offsite - exist. pavement
H	0.45	0.90	0.41	10	8.74	3.5	Offsite - exist. pavement & turf
I	1.55	0.30	0.47	15	7.56	3.5	Onsite - turf
J	0.15	0.30	0.05	15	7.56	0.3	Onsite - turf
K	0.38	0.90	0.34	10	8.74	3.0	Onsite - exist. pavement
L	0.57	0.90	0.51	10	8.74	4.5	Offsite - exist. pavement
M	0.55	0.90	0.50	10	8.74	4.3	Offsite - exist. pavement
N	0.12	0.90	0.11	10	8.74	0.9	Offsite - exist. pavement
O	0.23	0.90	0.21	10	8.74	1.8	Offsite - improved
P1	0.08	0.90	0.07	10	8.74	0.6	Offsite - exist. pavement & turf
P2(l)	0.18	0.90	0.16	10	8.74	1.4	Onsite - exist. pavement
P2(u)	0.06	0.30	0.02	15	7.56	0.1	Onsite - turf
Q	0.70	0.90	0.63	10	8.74	5.5	Offsite - exist. pavement
TOTAL	6.56					34.9	

DRAINAGE AREA TABLE - DEVELOPED CONDITIONS							
DRAINAGE AREA	AREA (acres)	C	CA	Tc (min)	i 100 (in/hr)	Q 100 (cfs)	COMMENTS
A	0.13	0.90	0.12	10	8.74	1.0	Offsite - improved to Street
B	0.06	0.90	0.05	10	8.74	0.5	Onsite - improved to street
C	0.19	0.90	0.17	10	8.74	1.5	Onsite - turf to street
D	0.14	0.90	0.13	10	8.74	1.1	Offsite - to Street
E	0.46	0.90	0.41	10	8.74	3.6	Roof Drg. - surface flow to detention
F	0.46	0.90	0.41	10	8.74	3.6	Roof Drg. - pipe to detention
G	0.10	0.90	0.09	10	8.74	0.8	Offsite - improved
H	0.45	0.90	0.41	10	8.74	3.5	Offsite - taxiway access
I	1.55	0.90	1.40	10	8.74	12.2	Onsite - ramp area
J	0.15	0.90	0.14	10	8.74	1.2	Roof Drg. - pipe to detention
K	0.38	0.90	0.34	10	8.74	3.0	Onsite - existing pavement
L	0.57	0.90	0.51	10	8.74	4.5	Offsite - improved
M	0.55	0.90	0.50	10	8.74	4.3	Offsite - improved
N	0.12	0.90	0.11	10	8.74	0.9	Offsite - improved
O	0.23	0.90	0.21	10	8.74	1.8	Offsite - improved
P	0.32	0.90	0.29	10	8.74	2.5	Onsite - part improved
Q	0.70	0.90	0.63	10	8.74	5.5	Offsite - improved
TOTAL	6.56					51.6	

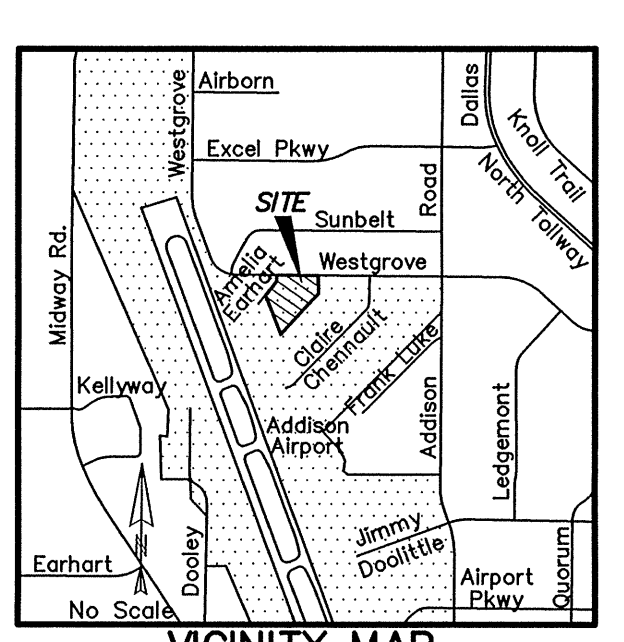
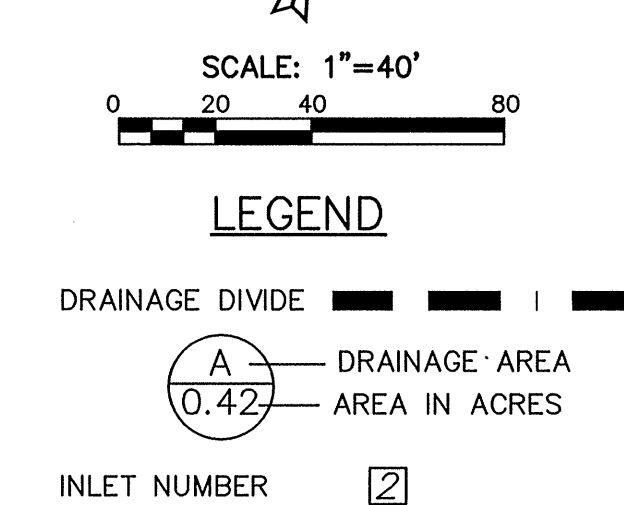
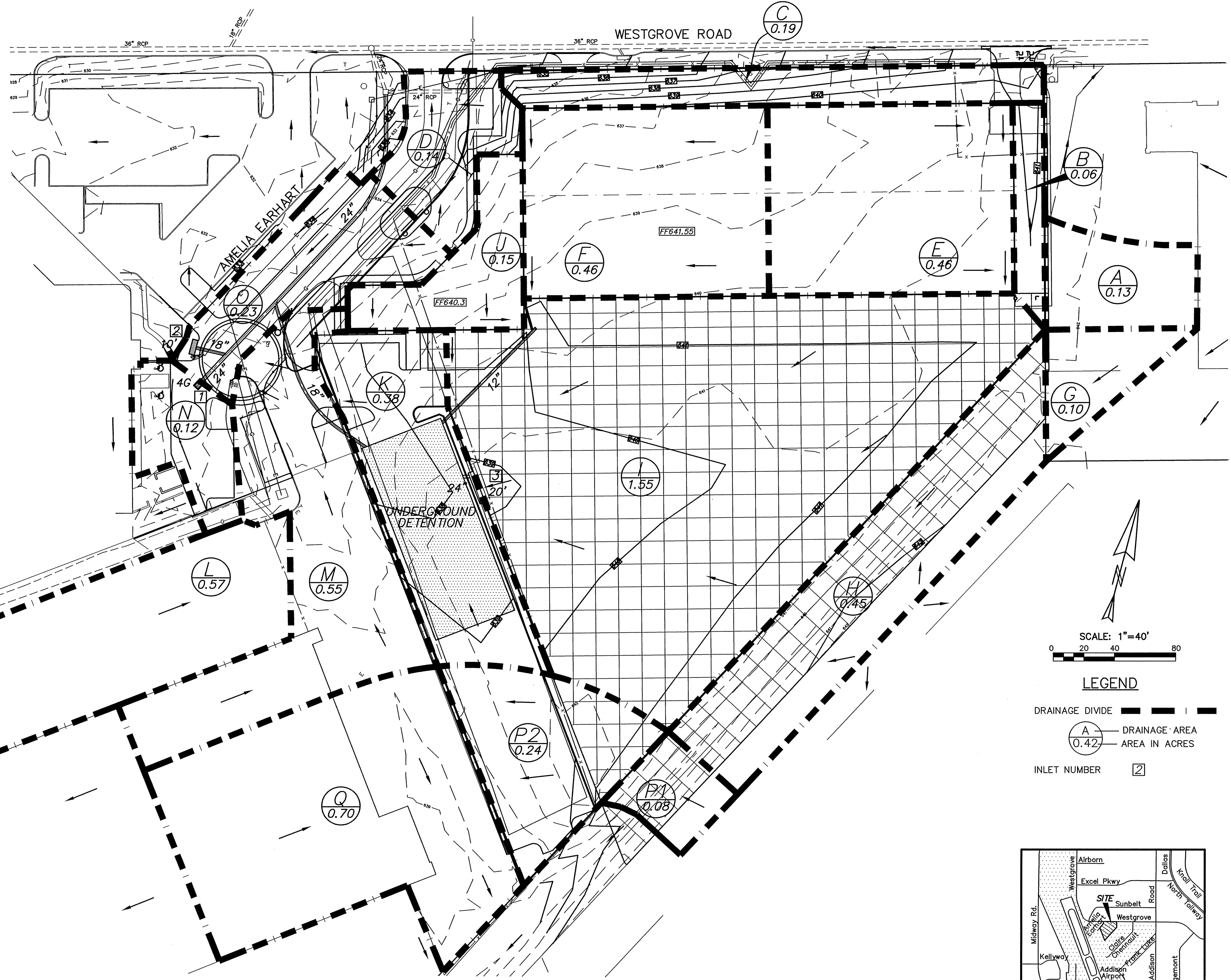
DETENTION CALCULATIONS - MILLION AIR, PHASE 2							
AREA	ACRES	C	Tc	I	CA	Q	NOTES
B	0.06	0.3	15	7.56	0.02	0.1	
C	0.19	0.3	15	7.56	0.06	0.4	
Du	0.06	0.3	15	7.56	0.02	0.1	
E	0.46	0.3	15	7.56	0.14	1.0	
F	0.46	0.3	15	7.56	0.14	1.0	
I	1.55	0.3	15	7.56	0.47	3.5	
J	0.15	0.3	15	7.56	0.05	0.3	
Qu	0.02	0.3	15	7.56	0.01	0.0	
P2u	0.06	0.3	15	7.56	0.02	0.1	
TOTAL	3.01				6.8	PRESENT CONDITIONS	
FUTURE CONDITIONS							
3.1 TOTAL UNDETAINED							
4.3 TOTAL PASS THROUGH							
8.1 ALLOWABLE DISCHARGE							

DETENTION VOLUME CALCULATIONS					
TIME/INTENSITY	CA	INFLOW	OUTFLOW	STORAGE	
10	8.74	2.85	14961	4851	10110
20	7	2.85	23965	7277	16688
30	5.79	2.85	29529	9702	19826
40	5.02	2.85	34373	12126	22245
50	4.4	2.85	37690	14553	23106
60	3.91	2.85	40159	16979	23180
70	3.62	2.85	43377	19405	23972
80	3.35	2.85	45876	21830	24046
90	3.1	2.85	47759	24256	23504
100	2.9	2.85	49642	26681	22961
110	2.7	2.85	50840	29107	21734
120	2.5	2.85	51354	31532	19822

OUTLET ORIFICE CALCULATION			
Q	8.1 Allowable Pond Discharge		
A (ft <sup>2</sup> )	1.15		
C	0.6		
g	32.2		
h	2.15 (NOTE)		
Pipe Rad. (ft)	0.60		
Pipe Dia. (in)	14.5		

INLET DESIGN CALCULATIONS												
INLET NO.	DESIGN STORM FREQ (yr)	CONCENTR. (in/hr)	AREA RUNOFF			CARRY OVER UPSTREAM	TOTAL GUTTER FLOW	SELECTED INLET LENGTH	CARRY OVER DOWNSTREAM	REMARKS		
			Q=CIA	TIME OF INTENS. RUNOFF	AREA						Q	
1	100	10	8.74	0.90	1.85	14.6	0.0	14.6	30	4 GRATE	4.3	
2	100	10	8.74	0.90	0.00	0.0	4.3	4.3	10'	CURB	0.0	
3	100	10	8.74	0.90	2.57	20.2	0.0	20.2	SAG	20'	CURB	0.0

STORM SEWER CALCULATIONS																												
RUNOFF COLLECTION POINT	DISTANCE BETWEEN POINTS	DRAINAGE AREA NO.	DRG. AREA AC.	RUNOFF COEFF. "C"	CA	STATION	TIME MIN.	DESIGN FREQ. YEARS	INTENSITY IN/HR	INCR. STORM WATER RUNOFF CFS	TOTAL STORM WATER RUNOFF CFS	SLOPE OF HYDRAULIC GRADIENT "S"	SELECT PIPE SIZE IN	VELOCITY BETWEEN POINTS FPS	HGL	HEAD LOSS CALCULATIONS					DESIGN INVERT ELEV.	REMARKS						
FROM STA.	TO STA.	FT.								Q" CFS	Q" CFS					V1 (in) ft/sec	V2 (out) ft/sec	V1 <sup>2</sup> /2G ft	V2 <sup>2</sup> /2G ft	Kj	KjV1 <sup>2</sup> /2G ft	Hj ft	HGL Elev. ft	IN ft	OUT ft			
LINE A	2+32.93	2+09.66	23.2	SEE INLET DESIGN CALCULATIONS - FROM INLET 1						10.3	10.3	0.0021	24"	3.3	633.15	633.10	N/A	3.3	0.00	0.17	1.25	0.00	0.21	633.36	630.30	630.05		
	2+09.66	1+63.15	46.5	SEE INLET DESIGN CALCULATIONS - FROM INLET 2						4.3	14.6	0.0042	24"	4.6	632.90	632.70	3.3	4.6	0.17	0.33	0.75	0.13	0.20	633.10	629.96	629.71		
	1+63.15	0+00	163.0	ALLOWED RELEASE FROM DETENTION						8.1	22.7	0.0101	24"	7.2	632.15	630.50	4.6	7.2	0.33	0.80	0.75	0.25	0.56	632.70	628.50	628.30	CONNECT TO EXIST INLET	
LINE B - DETENTION RELEASE	1+15.8	0+00	115.8	E,F,G,H,I,J	3.17	0.90	2.85	100	8.74	24.9	8.1*	0.0060	18"	4.6	633.32	632.63	N/A	4.6	0.00	0.33	0.75	0.00	0.33	633.65	629.96	629.71	END AT DETENTION	
LINE C - ROOF DRAINS	0+84.13	0+75.91	9.2	F	0.46	0.90	0.41	10	100	8.74	3.6	3.6	0.0102	12"	4.6	637.59	637.50	N/A	6.1	0.00	0.58	0.75	0.00	0.58	638.17	635.18	635.05	PARTIAL FLOW
	0+75.91	0+00	75.9	I	0.15	0.90	0.14	10	100	8.74	1.2	4.8	0.0181	12"	6.1	637.17	635.80	4.6	6.1	0.33	0.58	0.75	0.25	0.33	637.50	632.00	631.37	BEGIN AT DETENTION
LAT A-1			20.4	SEE INLET DESIGN CALCULATIONS - FROM INLET 2						4.3	4.3	0.0017	18"	2.4	632.93	632.90	4.60	2.4	0.33	0.09	1.25	0.41	0.11	633.05	630.30	630.05		



RECORD DRAWING  
 DATE: *October 26, 2012*  
 Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.



PREPARED BY  
**KADLECK & ASSOCIATES**  
 ENGINEERING PLANNING SURVEYING  
 2000 N. CENTRAL EXPY. SUITE 113  
 (972) 881-0771 PLANO, TX 75074  
 TBPE Reg. No. F-6460 TBPLS Reg. No. 100555-00

NOTE: EXISTING FEATURES AND TOPOGRAPHIC INFORMATION SHOWN WAS FURNISHED BY OWNER FROM SURVEY PREPARED BY: BRAD SPARR DATED: \_\_\_\_\_

SEE SHEETS 13 & 14 FOR DETAILS AND NOTES

**DRAINAGE AREA MAP**  
**MILLION AIR - PHASE TWO**  
**LEASE PARCEL No. 70 - WESTGROVE RD.**  
**ADDISON MUNICIPAL AIRPORT**  
**TOWN OF ADDISON, TEXAS**

DESIGN	DRAWN	DATE	SCALE
K&A	K&A	SEPT.2010	1"=40'

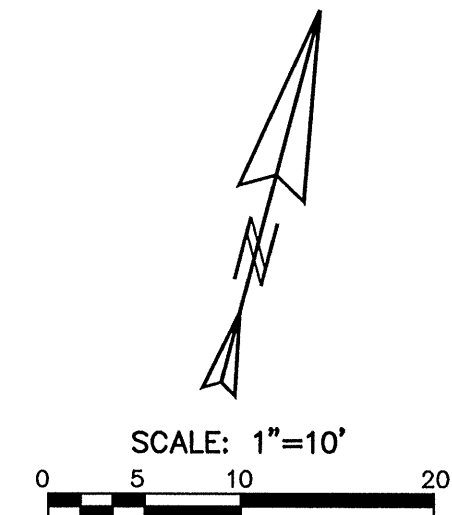
PROJECT No. 10544

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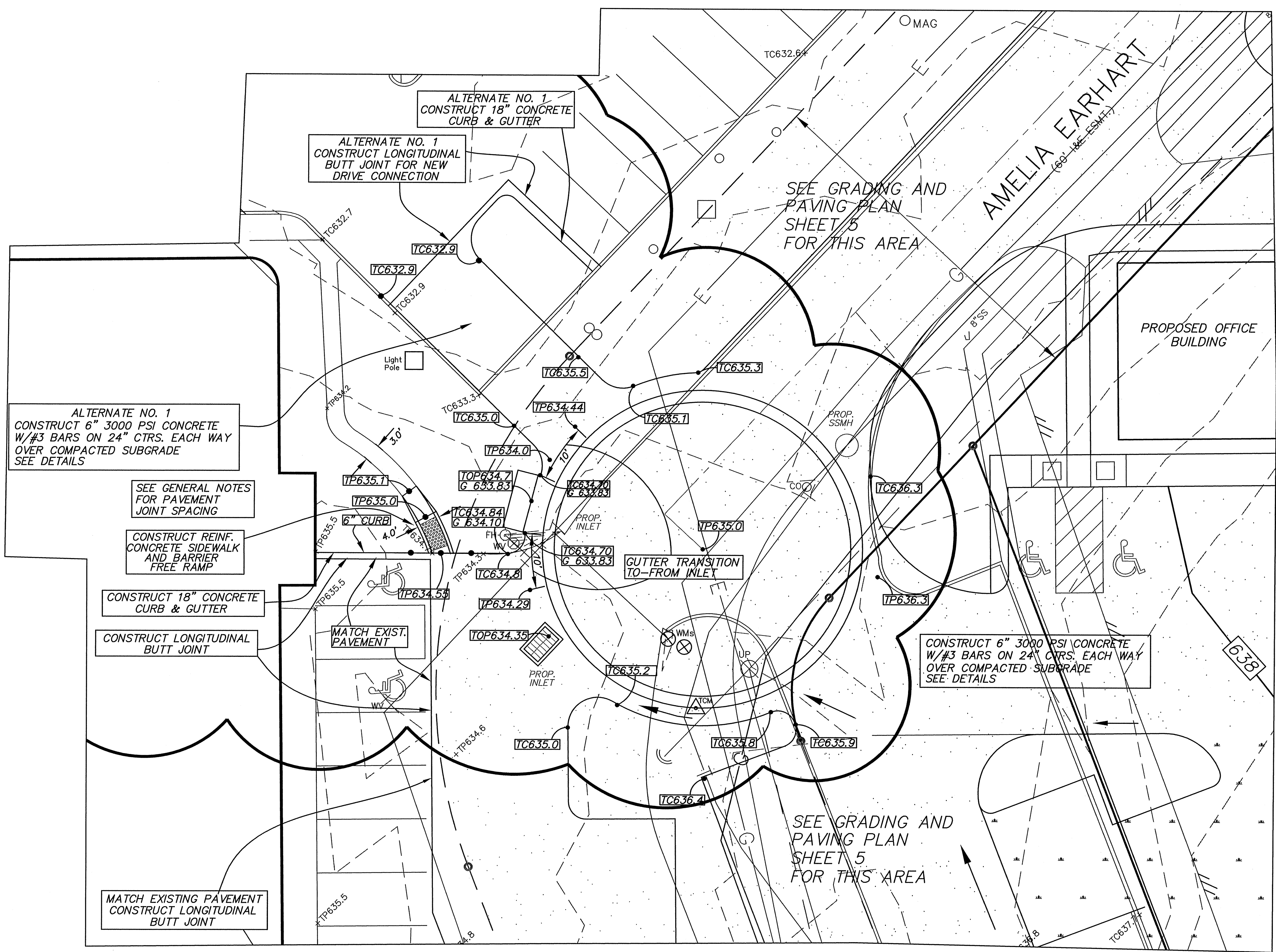
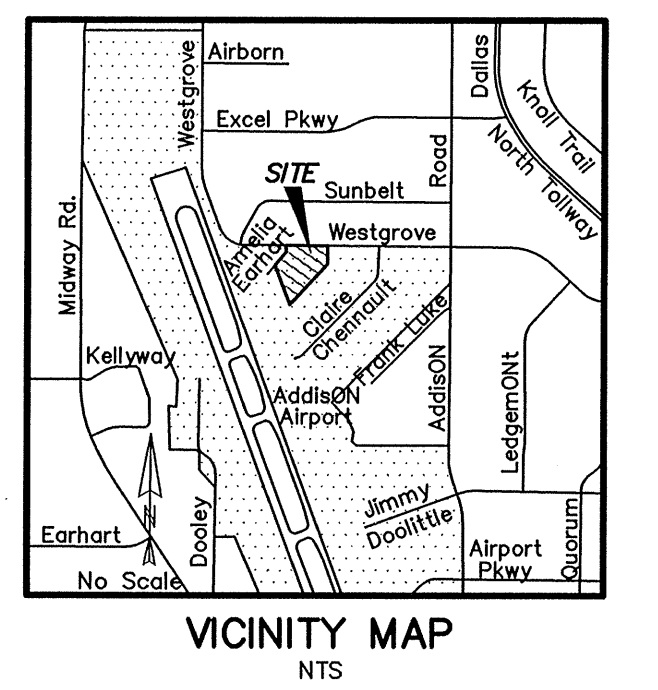








- LEGEND**
- FIR Found Iron Rod
  - UP Utility Pole
  - LP Light Pole
  - A Anchor
  - FH Fire Hydrant
  - WM Water Meter
  - WV Water Valve
  - WW Wastewater
  - SMH Storm Manhole
  - SD Storm Drain
  - RCP Reinf. CONcrete Pipe
  - EM Electric Meter
  - GM Gas Meter
  - 643.1 Ex. CONtour
  - x643.1 Ex. Spot Elevation
  - Proposed Spot Elevation
  - FF Finish Floor
  - TC Top of Curb
  - RW Retaining Wall
  - G Gutter



**PAVING LEGEND**

- HEAVY DUTY PAVEMENT**  
6" 3000 PSI CONCRETE  
W/#3 BARS ON 24" CTRS. EACH WAY  
OVER 6" COMPACTED SUBGRADE  
SEE DETAIL SHEET
- LIGHT DUTY PAVEMENT**  
5" 3000 PSI CONCRETE  
W/#3 BARS ON 24" CTRS. EACH WAY  
OVER 6" COMPACTED SUBGRADE  
SEE DETAIL SHEET
- EXISTING CONCRETE PAVEMENT**

NOTE: EXISTING FEATURES AND TOPOGRAPHIC INFORMATION SHOWN WAS FURNISHED BY OWNER FROM SURVEY PREPARED BY: BRAD SPARR DATED: \_\_\_\_\_

BENCH MARK: FOUND SQUARE CUT AT NORTHWEST CORNER OF BOX INLET, 13.5' SOUTH OF INTERSECTION OF SOUTH LINE OF WESTGROVE ROAD & EAST LINE OF AMELIA EARHART. ELEV. 632.80

6  
14

SEE TOWN OF ADDISON CONSTRUCTION DETAILS  
SEE SHEETS 13 & 14 FOR DETAILS AND NOTES

<b>GRADING AND PAVING PLAN</b>			
<b>FOR CIRCLE AREA</b>			
<b>MILLION AIR - PHASE TWO</b>			
<b>LEASE PARCEL No. 70-WESTGROVE RD.</b>			
<b>ADDISON MUNICIPAL AIRPORT</b>			
<b>TOWN OF ADDISON, TEXAS</b>			
DESIGN	DRAWN	DATE	SCALE
K&A	K&A	SEPT.2010	1"=10'
			PROJECT No. 10544

RECORD DRAWING

DATE: October 26, 2012

Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.

PREPARED BY

**KADLECK & ASSOCIATES**  
ENGINEERING PLANNING SURVEYING  
3000 N. CENTRAL EXPY. SUITE 113  
(972) 881-0771 PLANO, TX 75074  
TBPE Reg. No. F-6460 TBPLS Reg. No. 100555-00

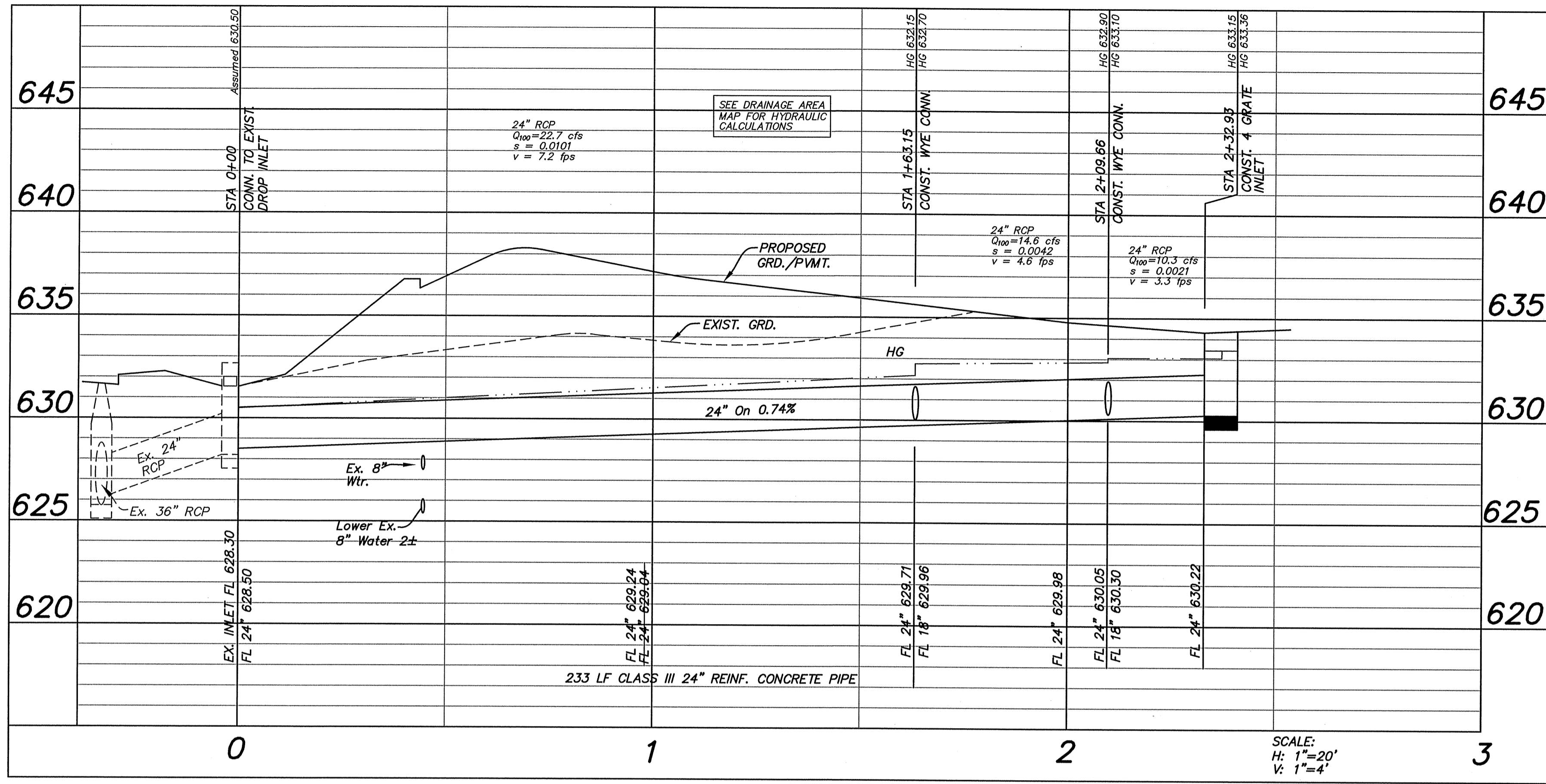




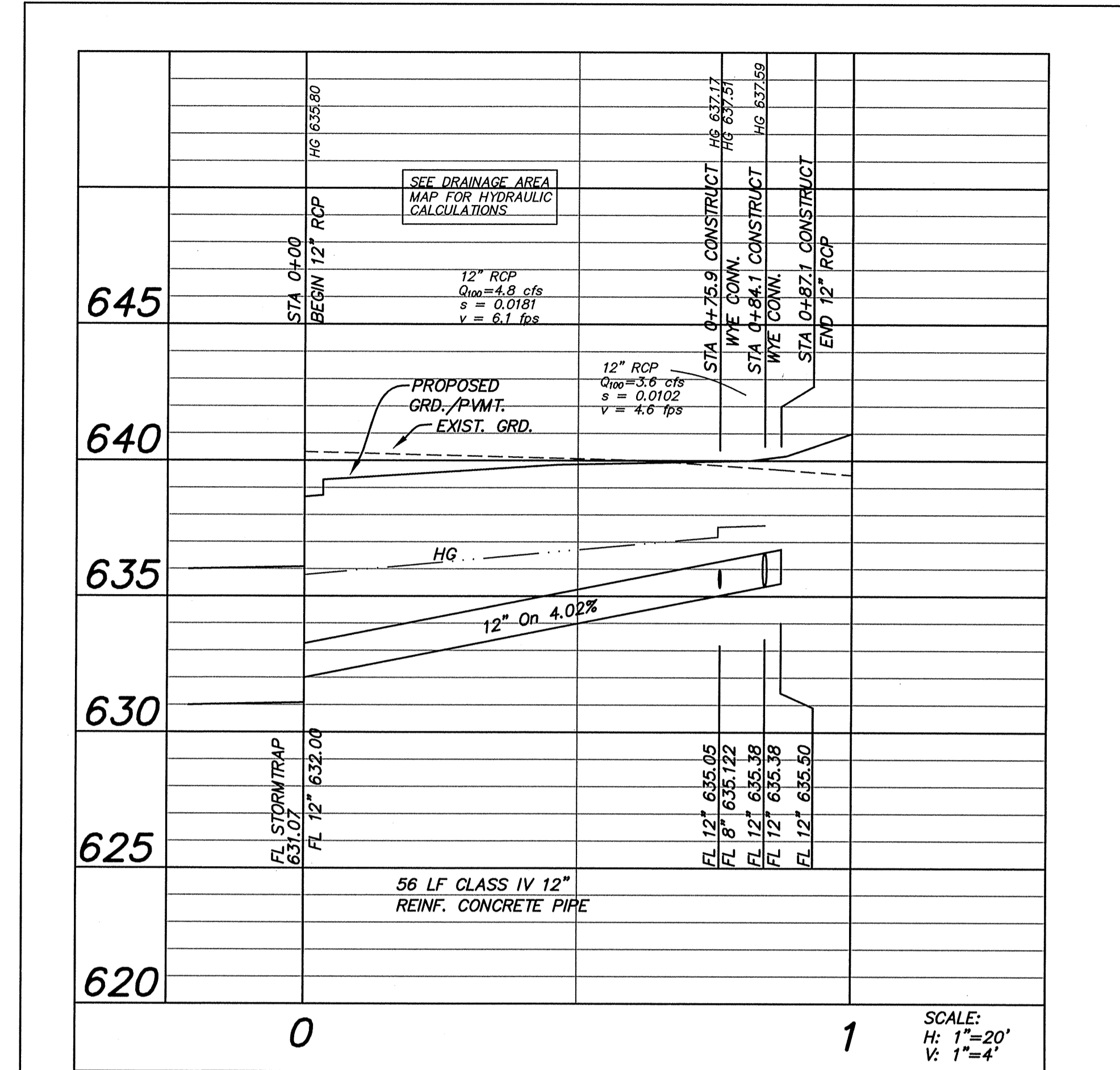




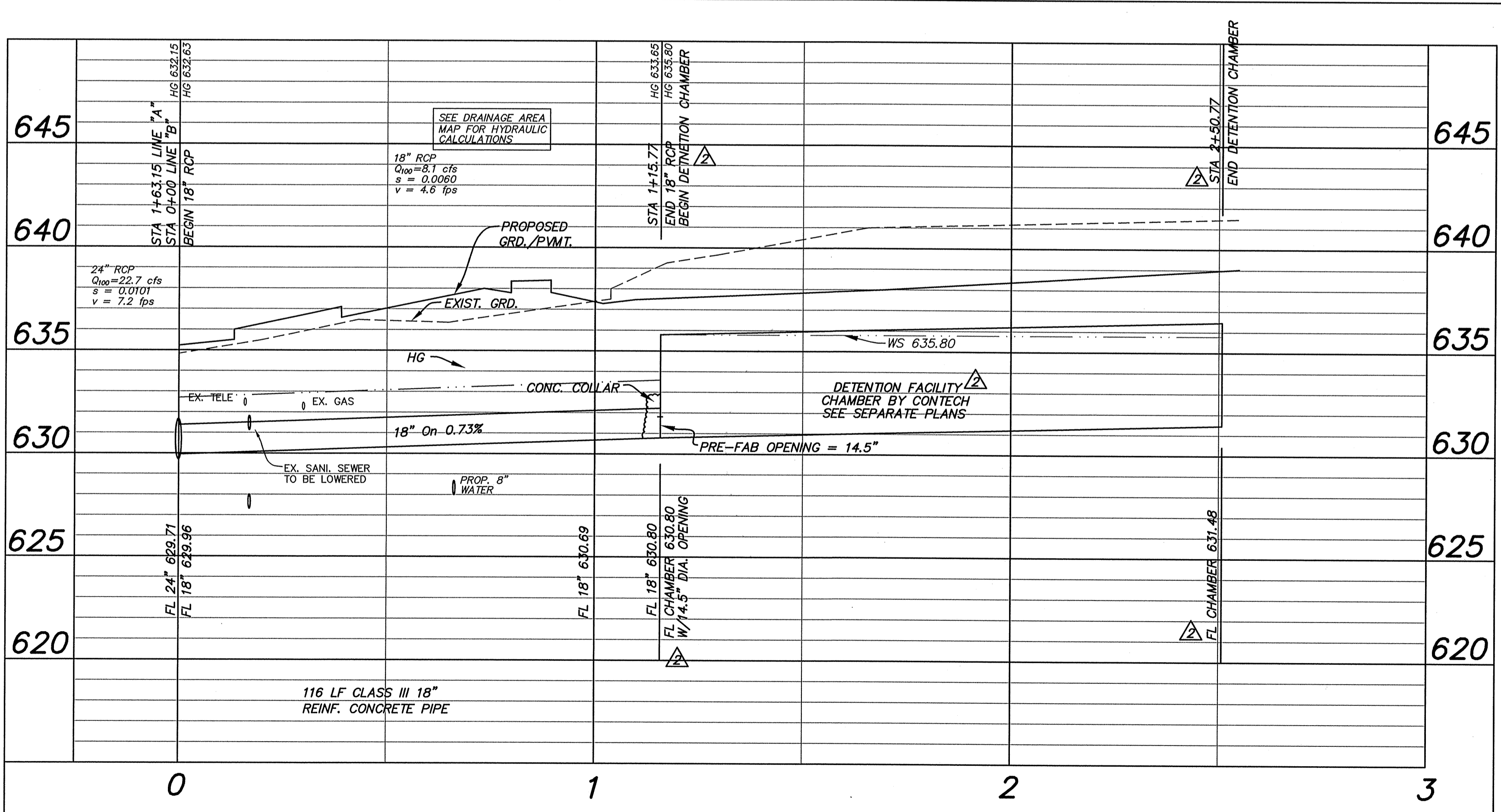




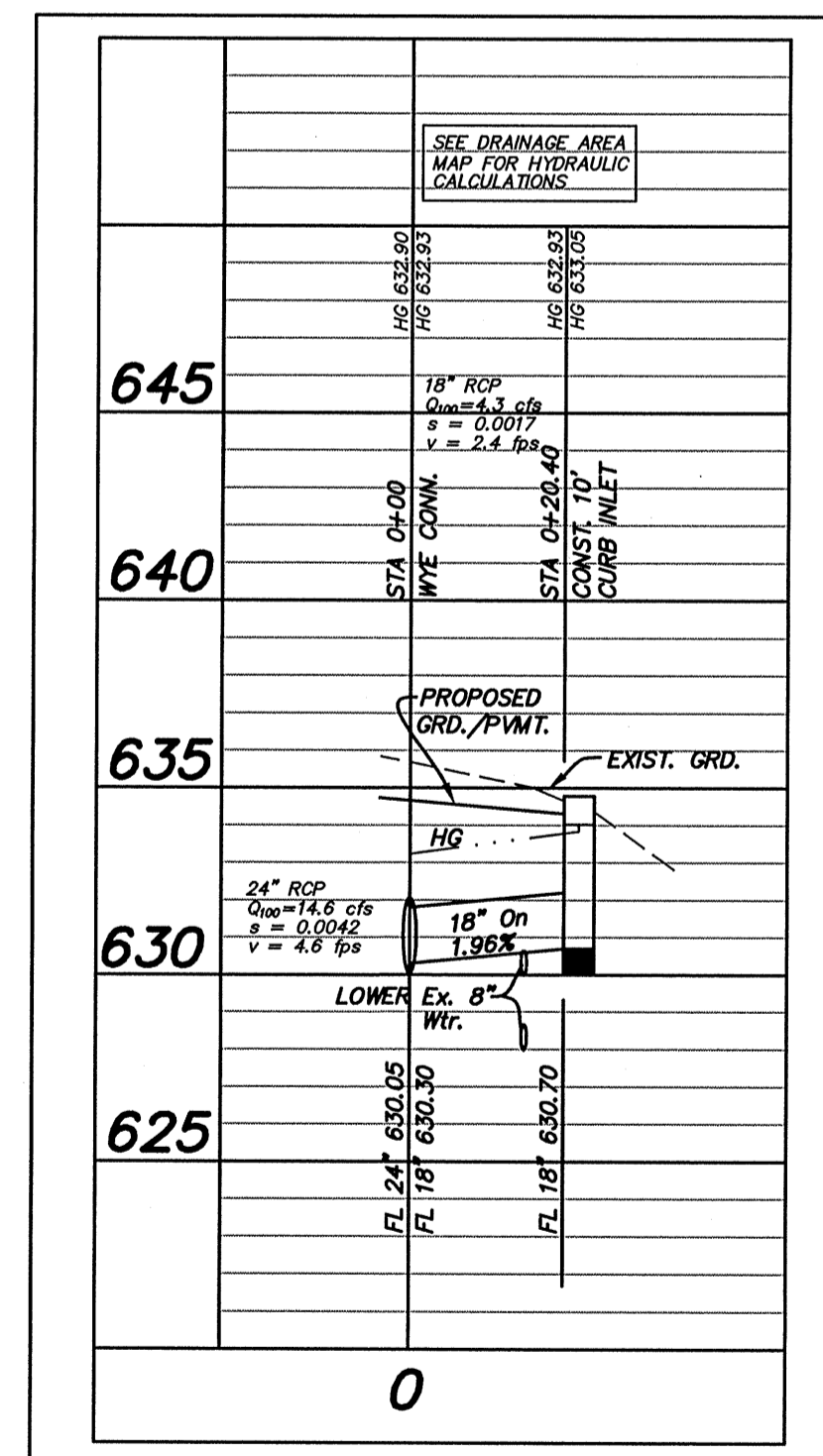
LINE "A"



LINE "C" ROOF DRAINAGE



LINE "B" DETENTION DRAIN

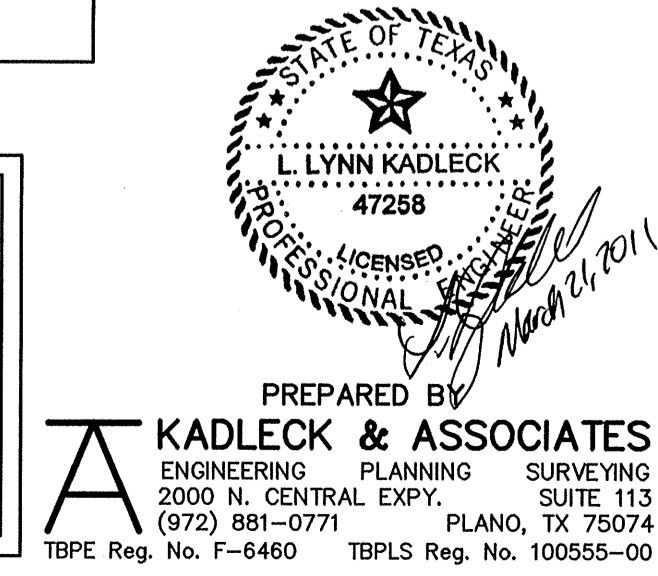


LAT. "A-1"

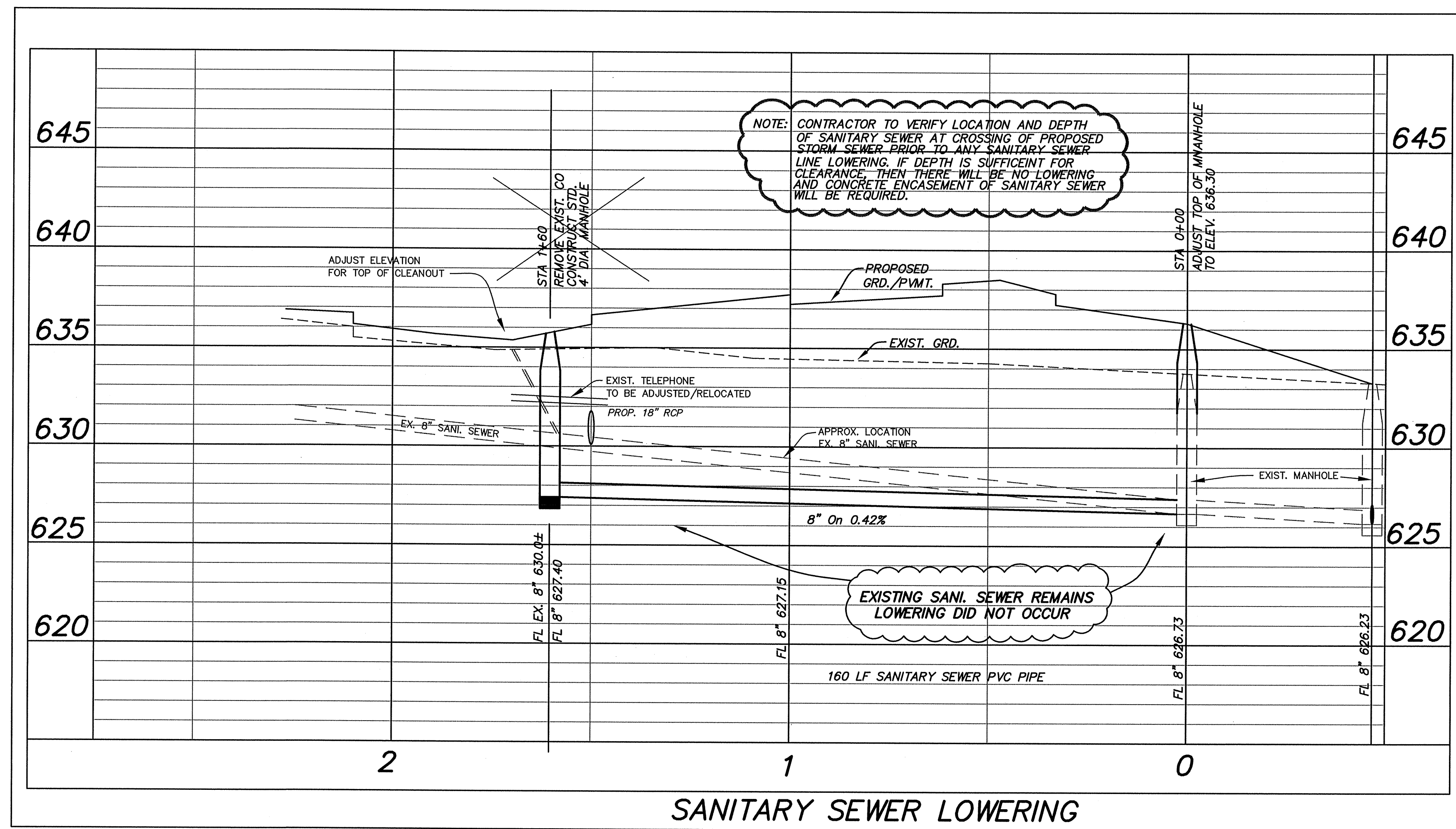
NOTE: EXISTING FEATURES AND TOPOGRAPHIC INFORMATION SHOWN WAS FURNISHED BY OWNER FROM SURVEY PREPARED BY: BRAD SPARR DATED: \_\_\_\_\_  
 BENCH MARK: FOUND SQUARE CUT AT NORTHWEST CORNER OF BOX INLET, 13.5' SOUTH OF INTERSECTION OF SOUTH LINE OF WESTGROVE ROAD & EAST LINE OF AMELIA EARHART. ELEV. 632.80

SEE SHEETS 13 & 14 FOR DETAILS AND NOTES  
 REVISED: Sept. 12, 2011; Change Detention Facility.

RECORD DRAWING  
 DATE: *October 26, 2010*  
 Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.



UTILITY PLAN			
MILLION AIR - PHASE TWO			
LEASE PARCEL No. 70-WESTGROVE RD.			
ADDISON MUNICIPAL AIRPORT			
TOWN OF ADDISON, TEXAS			
DESIGN	DRAWN	DATE	SCALE
K&A	K&A	SEPT.2010	H:1"=20' V:1"=4'
			K&A PROJECT #10544



NOTE: EXISTING FEATURES AND TOPOGRAPHIC INFORMATION SHOWN WAS FURNISHED BY OWNER FROM SURVEY PREPARED BY: BRAD SPARR DATED: \_\_\_\_\_

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FOUND SQUARE CUT AT NORTHWEST CORNER OF BOX INLET, 13.5' SOUTH OF INTERSECTION OF SOUTH LINE OF WESTGROVE ROAD & EAST LINE OF AMELIA EARHART. ELEV. 632.80

10  
14

SEE SHEETS 13 & 14 FOR DETAILS AND NOTES

RECORD DRAWING  
DATE: October 26, 2012

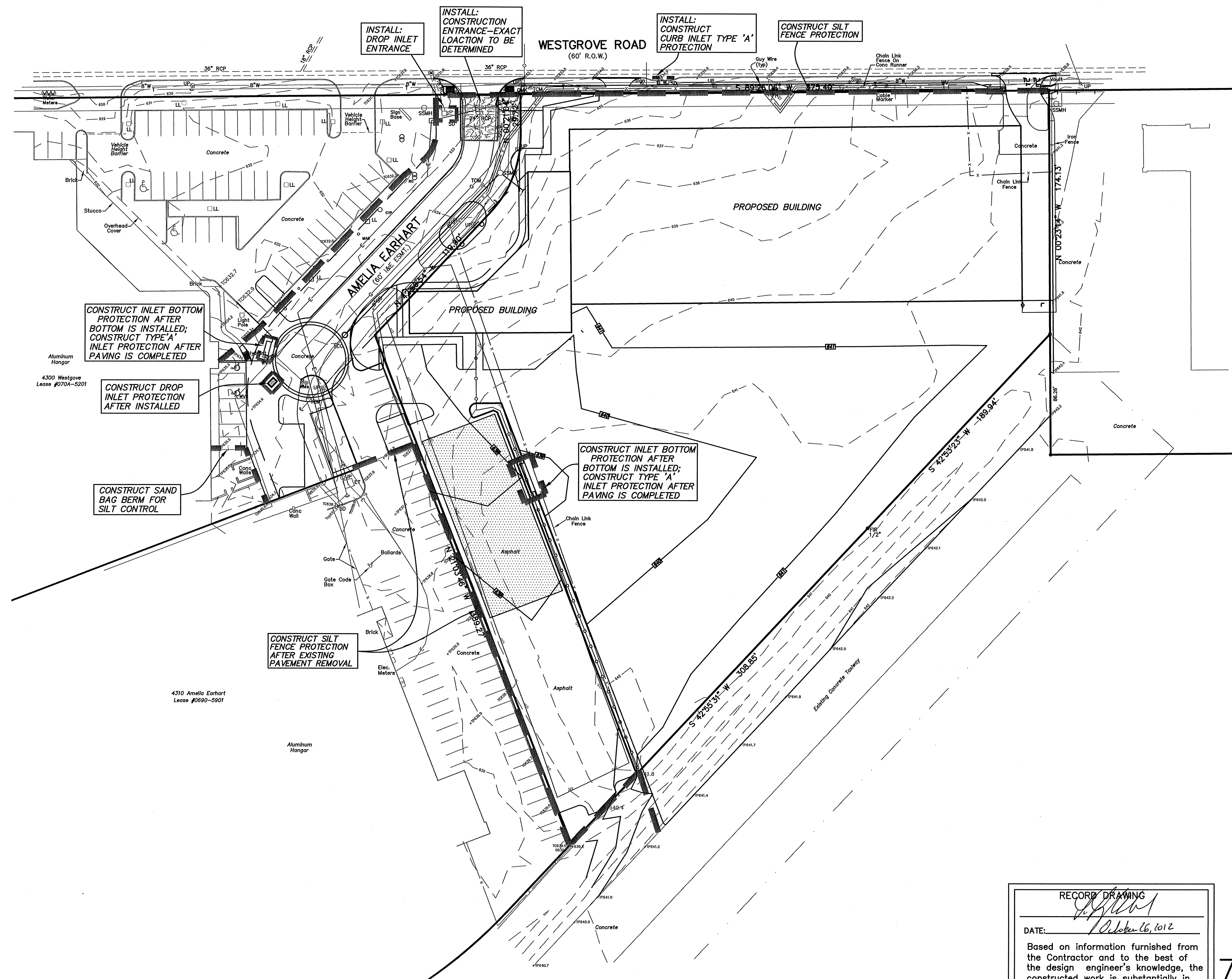
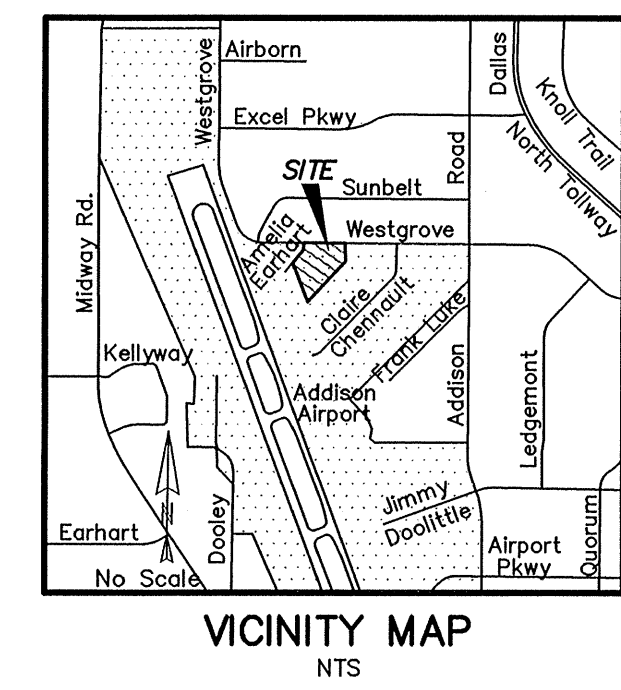
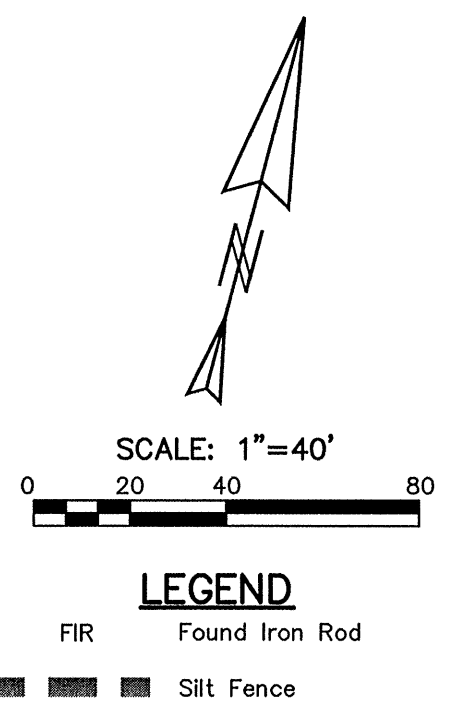
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PREPARED BY  
L. LYNN KADLECK  
47268  
L. LYNN KADLECK  
March 21, 2011

KADLECK & ASSOCIATES  
ENGINEERING PLANNING SURVEYING  
2000 N. CENTRAL EXPY. SUITE 113  
(972) 881-0771 PLANO, TX 75074  
TBPE Reg. No. F-6460 TBPLS Reg. No. 100555-00

UTILITY PLAN				
SANITARY SEWER PROFILE				
MILLION AIR - PHASE TWO				
LEASE PARCEL No. 70-WESTGROVE RD.				
ADDISON MUNICIPAL AIRPORT				
TOWN OF ADDISON, TEXAS				
DESIGN	DRAWN	DATE	SCALE	
K&A	K&A	SEPT.2010	H:1"=20' V:1"=5'	K&A PROJECT #10544





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DATED: \_\_\_\_\_

BENCH MARK:  
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ELEV. 632.80

11  
14

SEE SHEETS 12 FOR EROSION DETAILS AND NOTES  
SEE SHEETS 13 & 14 FOR DETAILS AND NOTES

RECORD DRAWING  
*[Signature]*  
DATE: *October 16, 2012*

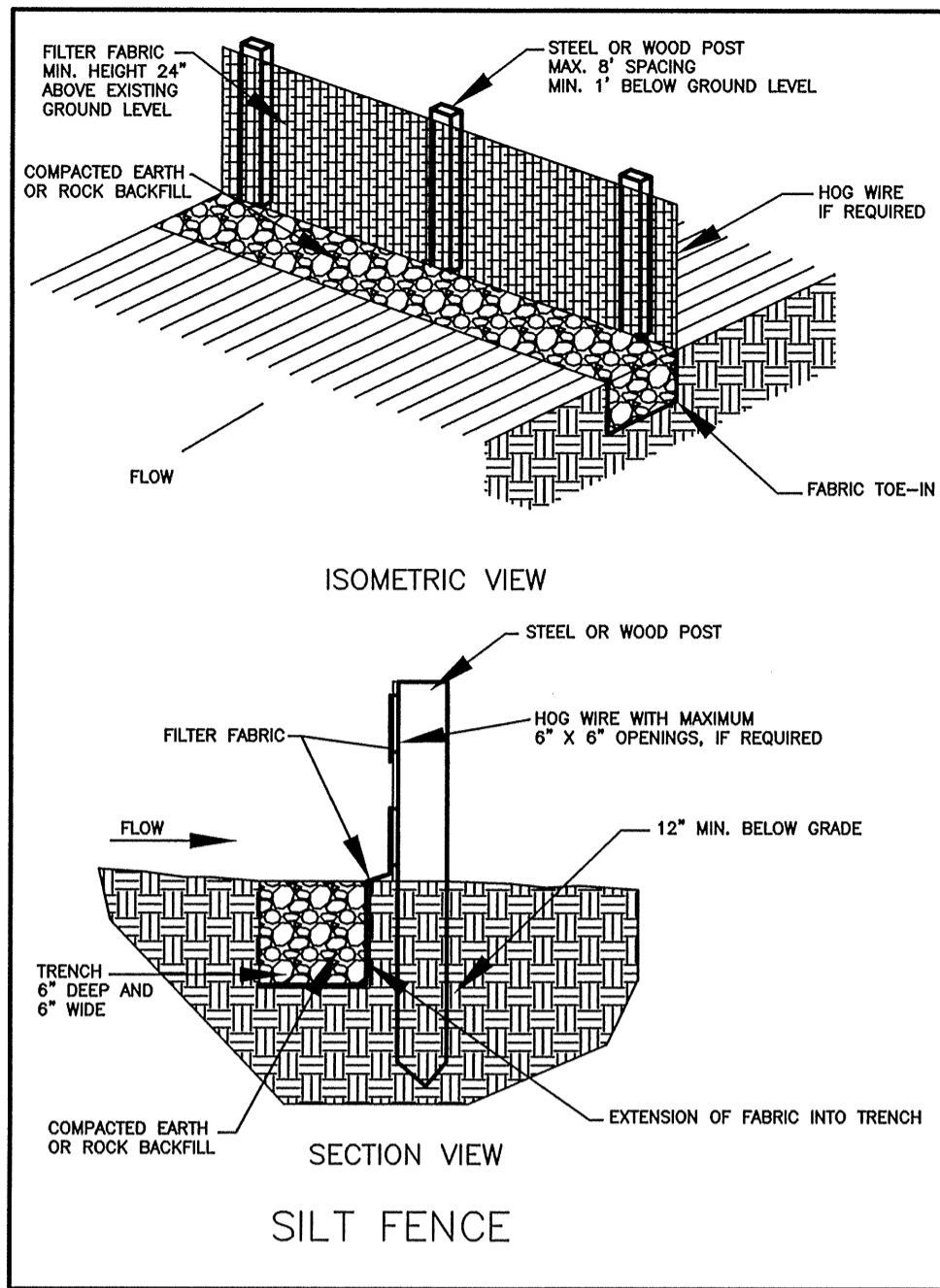
Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.



PREPARED BY  
**KADLECK & ASSOCIATES**  
ENGINEERING PLANNING SURVEYING  
2000 N. CENTRAL EXPY. SUITE 113  
(972) 881-0771 PLANO, TX 75074  
TBPE Reg. No. F-6460 TBPLS Reg. No. 100555-00

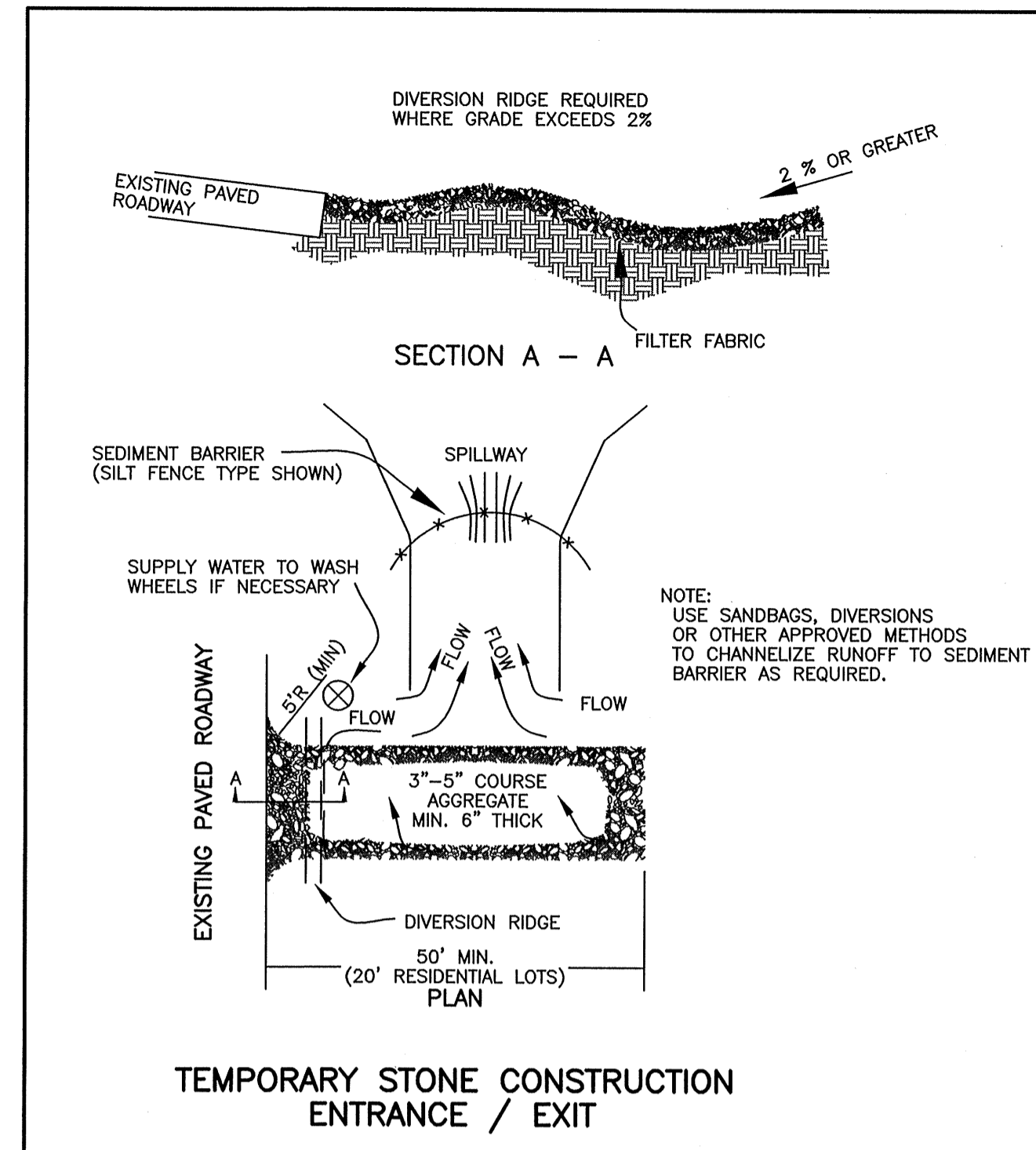
EROSION CONTROL PLAN				
MILLION AIR - PHASE TWO				
LEASE PARCEL No. 70-WESTGROVE RD.				
ADDISON MUNICIPAL AIRPORT				
TOWN OF ADDISON, TEXAS				
DESIGN	DRAWN	DATE	SCALE	
K&A	K&A	SEPT.2010	1"=40'	PROJECT No. 10544





SILT FENCE GENERAL NOTES

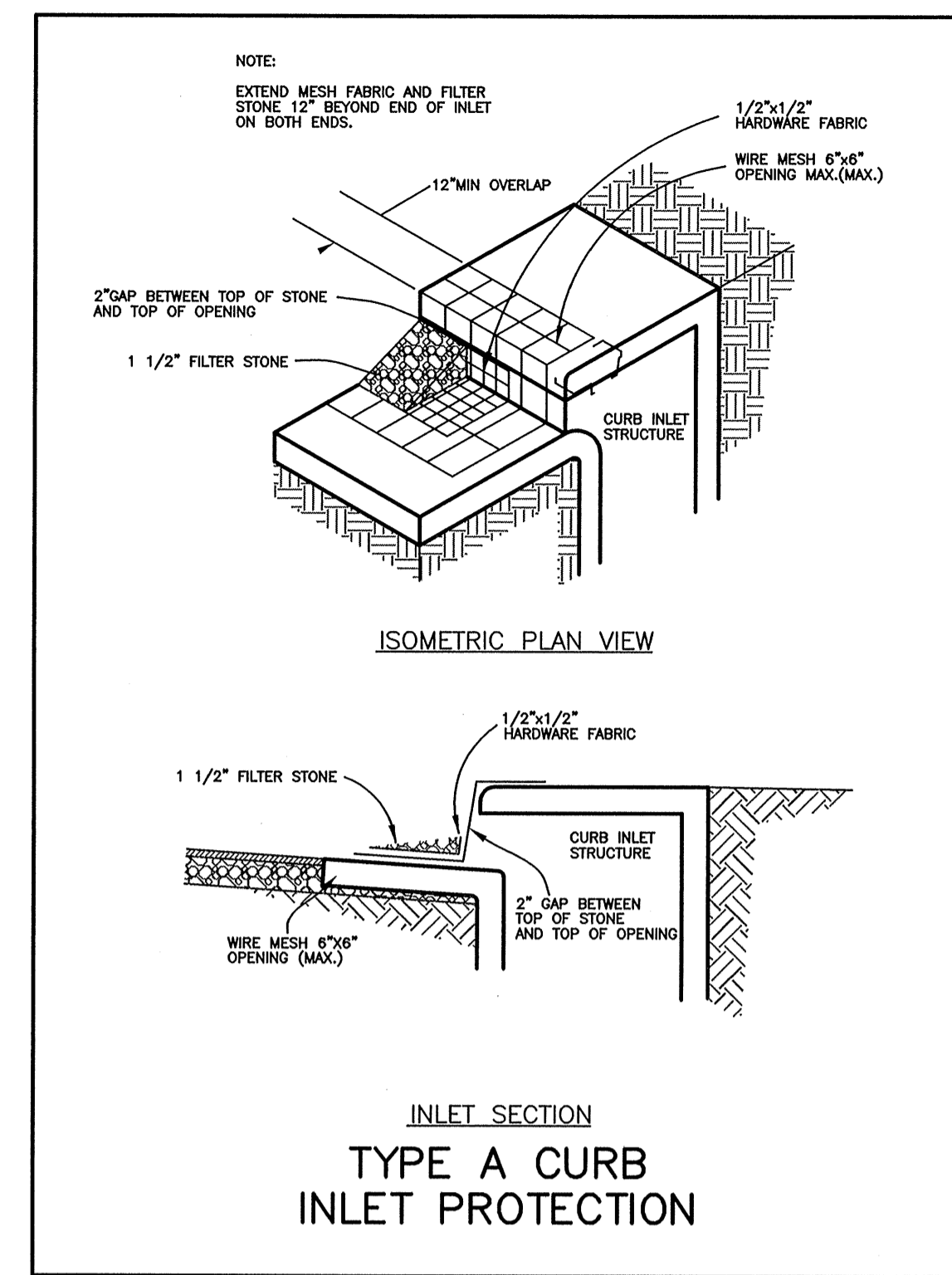
- Silt fence fabric must meet the following minimum criteria:
  - Tensile Strength, ASTM D4632 Test Method for Grab Breaking Load and Elongation of Geotextiles, 90-lbs.
  - Puncture Rating, ASTM D4833 Test Method for Index Puncture Resistance of Geotextiles, Geomembranes, and Related Products, 60-lbs.
  - Mullen Burst Rating, ASTM D3786 Standard Test for Hydraulic Bursting Strength of Textile Fabrics-Diaphragm Bursting Strength Tester Method, 280-lbs.
  - Apparent Opening Size, ASTM D4751 Test Method for Determining Apparent Opening Size of a Geotextile, U.S. Sieve No. 70 (max) to No. 100 Min)
  - Ultraviolet Resistance, ASTM D4355, Minimum 70 percent.
- Fence posts shall be galvanized steel and may be T-section or L-section, 1.3 pounds per linear foot minimum, and 4 feet in length minimum. Wood post may be used depending on anticipated length of service and provided they are 4 feet in length minimum and have a nominal cross section of 2 inches by 4 inches for pine and 2 inches by 2 inches for hardwoods. Fence posts, which support the silt fence, shall be installed on a slight angle toward the anticipated runoff source. Post must be embedded a minimum of one foot.
- Silt fence shall be supported by galvanized steel wire fence fabric as follows:
  - 4 inch by 4 inch mesh wire, @1.4/1.4, minimum 14-gauge wire fence fabric;
  - Hog wire, 12 gauge wire, small openings installed at bottom of silt fence;
  - Standard 2 inch by 2 inch chain link fence fabric; or
  - Other welded or woven steel fabric consisting of equal or smaller spacing as that listed hereon and appropriate gauge wire to provide support.
- A 6-inch wide trench is to be cut 6 inches deep at the toe of the fence to allow the fabric to be laid below the surface and backfilled with compacted earth or gravel to prevent bypass runoff under the fence. Fabric shall overlap at abutting ends a minimum of 3 feet and shall be joined such that no leakage or bypass occurs.
- Sufficient room for the operation of sediment removal equipment shall be provided between the silt fence and other obstructions in order to properly maintain the fence.
- The ends of the fence shall be turned upstream to prevent bypass of storm water.
- Silt fence should be inspected regularly (at least as often as required by the TPDES Construction General Permit, Appendix A) for buildup of excess sediment, undercutting, sags and other failures. Sediment should be removed when it reached approximately 1/2 half the height of the fence. In addition, determine the source of excess sediment and implement appropriate BMPs to control erosion. If the fabric becomes damaged or clogged, it should be repaired or replaced as necessary.
- Silt fence shall be removed when the site is completely stabilized so as not to block or impede storm flow or drainage.
- Stone overflow structures or other outlet control devices shall be installed at all low points along the fence or spaced at approximately 300 feet if there is no apparent low point.
- Filter stone for overflow structure shall be 1-1/2" washed stone containing no fines. Angular shaped stone is preferable to rounded shapes.



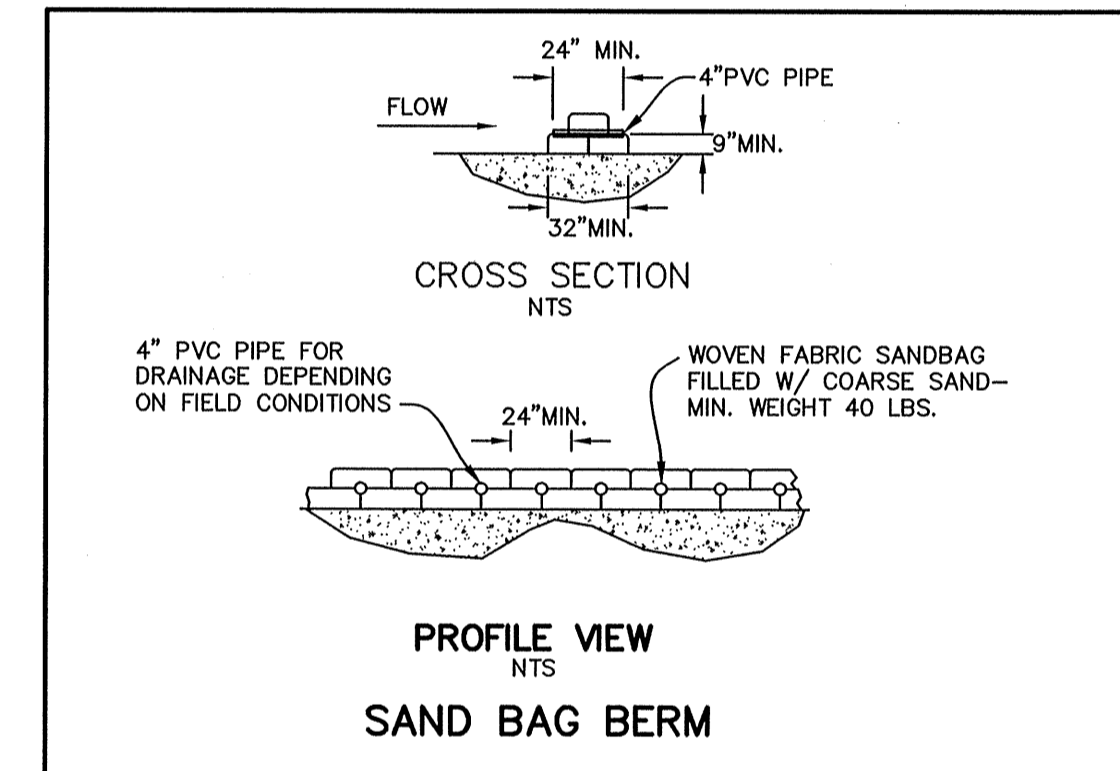
TEMPORARY STONE CONSTRUCTION ENTRANCE / EXIT

NOTES FOR STABILIZED CONSTRUCTION ENTRANCE

- The stabilized construction entrance shall consist of a pad constructed of a crushed stone, recycled concrete or other rock like material, or asphaltic material on top of geotextile filter cloth to facilitate removal of sediment and other debris from construction equipment prior to exiting the construction site. This directly addresses the problem of silt and mud deposition in roadways used for construction site access. For added effectiveness, a wash rack are can be incorporated into the design to further reduce sediment tracking.
- The stabilized construction entrance shall be constructed such that drainage across the entrance is directed to a controlled, stabilized outlet on site with provisions for proper filtration and removal of wash water.
- The entrance must be sloped away from the paved surface so that storm water is not allowed to leave the site onto roadways.
- Minimum width of entrance shall be 15 feet.
- Stone shall be placed in a layer of at least 12-inches thickness. The stone shall be a minimum of 3 to 5 inch coarse aggregates.
- Prevent shortcutting of the full length of the construction entrance by installing barriers as necessary.
- Silt fence fabric must meet the following minimum criteria:
  - Tensile Strength, ASTM D4632 Test Method for Grab Breaking Load and Elongation of Geotextiles, 90-lbs.
  - Puncture Strength, ASTM D4833 Test Method for Index Puncture Resistance of Geotextiles, Geomembranes, and Related Products, 60-lbs.
  - Mullen Burst Rating, ASTM D3786 Standard Test for Hydraulic Bursting Strength of Textile Fabrics-Diaphragm Bursting Strength Tester Method, 280-lbs.
  - Apparent Opening Size, ASTM D4751 Test Method for Determining Apparent Opening Size of a Geotextile, U.S. Sieve No. 70 (max) to No. 100 Min)
  - Ultraviolet Resistance, ASTM D4355, Minimum 70 percent.
- When necessary, vehicles must be cleaned to remove sediment prior to entrance to paved roads, streets, or parking lots. When washing is required, it shall be done on a constructed wheel wash facility that drains into an approved sediment trap or sediment basin or other sedimentation/filtration device.
- Construction entrances shall be inspected regularly (at least as often as required by TPDES Construction General Permit, Appendix A). When sediment has substantially clogged the void area between the rocks, the aggregate mat must be washed down or replaced. Periodic re-grading and top dressing with additional stone must be done to keep the efficiency of the entrance from diminishing.
- If the stabilized construction entrance is not effective removing sediment from wheels then a wheel wash should be considered.



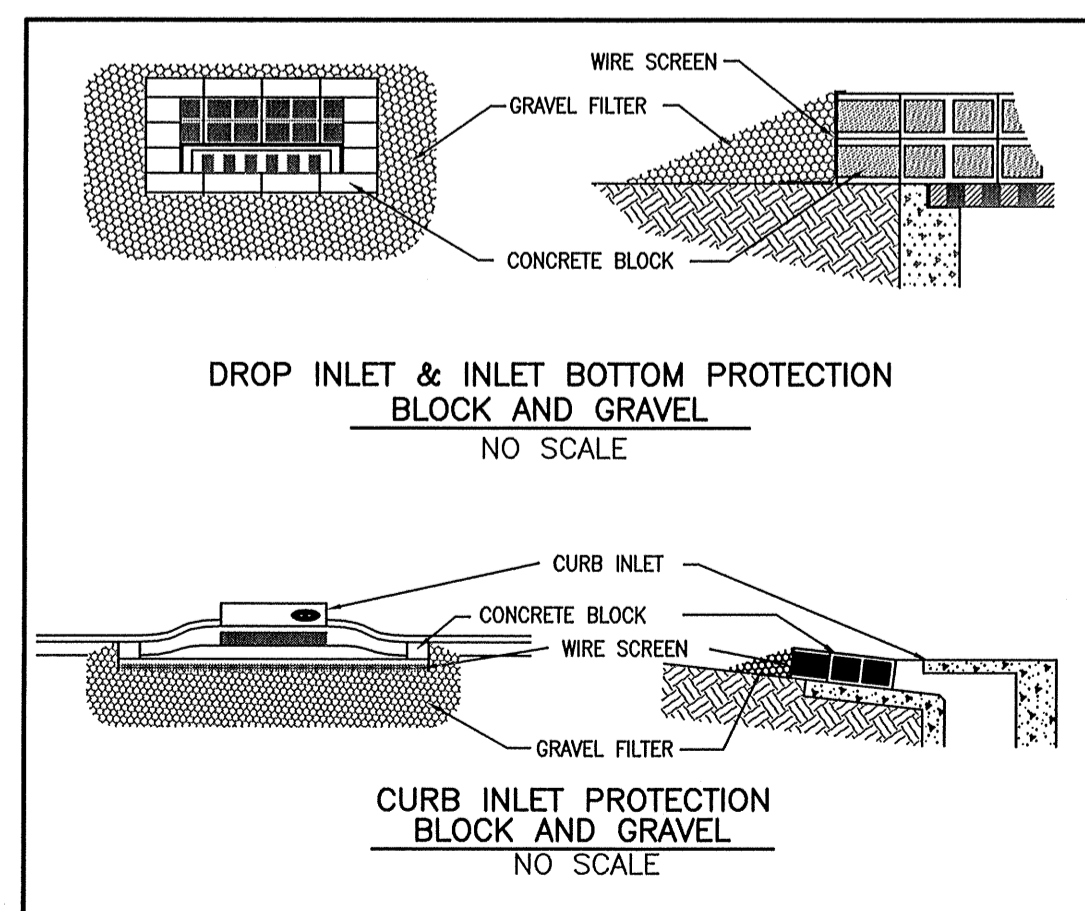
INLET SECTION TYPE A CURB INLET PROTECTION



PROFILE VIEW SAND BAG BERM

SAND BAG BERM GENERAL NOTES

- When a sandbag is filled with material, the open end of the sandbag should be stapled or tied with nylon or poly cord.
- Sandbags should be stacked in at least three rows abutting each other, and in a staggered arrangement.
- The base of the berm should have at least 3 sandbags. These can be reduced to 2 and 1 bag in the second and third rows, respectively.
- For each additional 6 inches of height, an additional sandbag must be added to each row width.
- The sandbag berm shall be inspected after each 1/2 inch rain event and shall be reshaped or replaced as needed during inspection. Repairs shall be made for washout, construction traffic damage, etc.
- When silt reaches a depth equal to 6 inches (the height of one sandbag), the silt shall be removed and disposed of at an approved site and in such a manner as to not create a siltation problem.
- When the site is completely stabilized, the berm and accumulated sediment shall be removed and disposed of in an approved manner.



DROP INLET & INLET BOTTOM PROTECTION BLOCK AND GRAVEL NO SCALE

CURB INLET PROTECTION BLOCK AND GRAVEL NO SCALE

GENERAL NOTES - EROSION CONTROL

- A Storm Water Pollution Prevention Plan shall be required for this project and shall be prepared by the Owner and/or Contractor.
- Contractor shall be responsible for erosion and sediment control including control of construction access and street cleaning if necessary. Place erosion control devices as indicated hereon prior to start of construction. Contractor to maintain erosion control devices until time for permanent paving and landscaping. Prior to placement of proposed paving, construction access shall be from a single location to be specified by the Contractor and the curb shall be protected in accordance with City requirements. Vehicles shall be properly cleaned prior to entering public streets. The Contractor shall immediately remove any construction materials tracked onto public streets.
- Inspections must be conducted at least once every fourteen (14) days and within twenty-four (24) hours of the end of a storm event of 0.5 inches or greater. As an alternative to the said inspection schedule, inspections will occur at least once every seven (7) calendar days. If this alternate schedule is used, the inspection must occur on a specific day of the week, regardless of whether or not there has been a rainfall event since the previous inspection.
- The disturbed area consists of approximately 3.5 acres of land. The Owner is The Mission Companies, P.O. Box 639, Addison, Texas 75001, contact: George Shirley (214) 906-0720. The property to be disturbed and the surrounding areas are currently paving and unimproved areas with grass. The land use is commercial. The main pollutant from the site during construction is erosion and sediment.
- The construction activities shall consist of excavation, utility line placement, storm sewer line and inlet construction, site grading, subgrade preparation and paving.
- A Notice of Intent shall be completed, submitted and posted at the project in plain view.
- After grading is completed, the contractor shall hydromulch all disturbed areas, unless specified differently in the landscape plans, and establish at least 90% grass coverage including necessary watering. The specific plant material proposed to protect fill and excavated slopes shall be as indicated on the landscape plans. Plant material must be suitable for use under local climate and soil conditions. In general, hydroseeding or sodding Bermuda grass is acceptable during the summer months (May 1 to August 30). Winter rye or fescue grass may be planted during times other than the summer months as temporary measure until such time as the permanent planting can be made.
- Prior to commencing any construction, a construction entrance and perimeter silt fence shall be installed at the location shown.
- All existing inlets shall be protected prior to start of construction. As inlets are completed, temporary sediment barriers shall be installed.
- Silt fence and inlet sediment barriers shall remain in place until vegetation has been completed.
- Disturbed areas that are seeded or sodded shall be checked periodically to see that grass coverage is properly maintained. Disturbed areas shall be watered, fertilized, and re-seeded or re-sodded, if necessary.
- Erosion control measures may only be placed in front of inlets, or in channels, drainageways or borrow ditches at risk of contractor. Contractor shall remain liable for any damage caused by the measures, including flooding damage which may occur due to blocked drainage. At the conclusion of any project, all channels, driveways and borrow ditches in the work zone shall be dredged of any sediment generated by the project or deposited as a result of erosion control measures.
- See separate soils report prepared by Reed Engineering Group for soil types and log of soil borings.

RECORD DRAWING  
 DATE: *October 26, 2012*  
 Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.

STATE OF TEXAS  
 L. LYNN KADLECK  
 47258  
 PROFESSIONAL ENGINEER  
 PREPARED BY  
**KADLECK & ASSOCIATES**  
 ENGINEERING PLANNING SURVEYING  
 2000 N. CENTRAL EXPY. SUITE 113  
 (972) 881-0771 PLANO, TX 75074  
 TBPE Reg. No. F-6460 TBPLS Reg. No. 100555-00

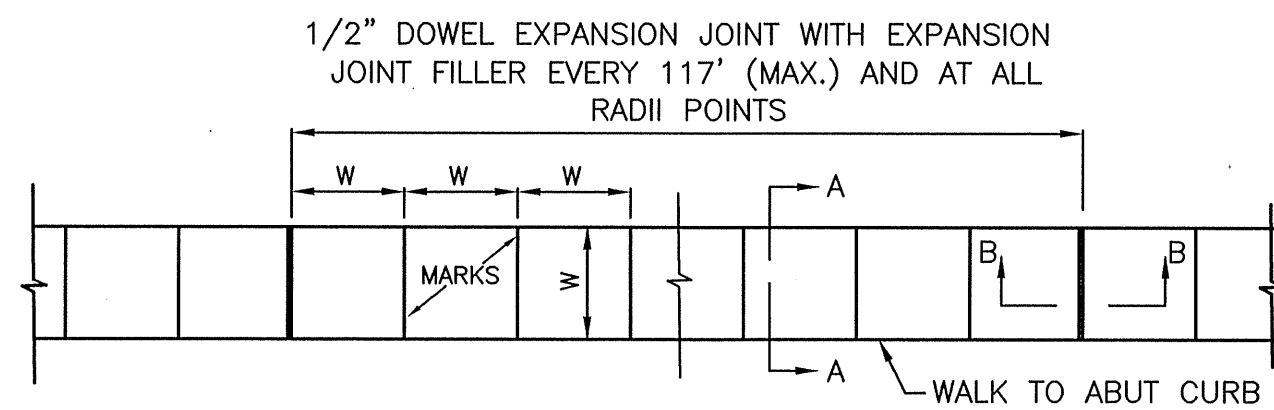
SEE SHEET 4 FOR DRAINAGE CALCULATIONS  
 SEE SHEETS 13 & 14 FOR DETAILS AND NOTES

**EROSION CONTROL DETAILS & NOTES**  
**MILLION AIR - PHASE TWO**  
**LEASE PARCEL No. 70-WESTGROVE RD.**  
**ADDISON MUNICIPAL AIRPORT**  
**TOWN OF ADDISON, TEXAS**

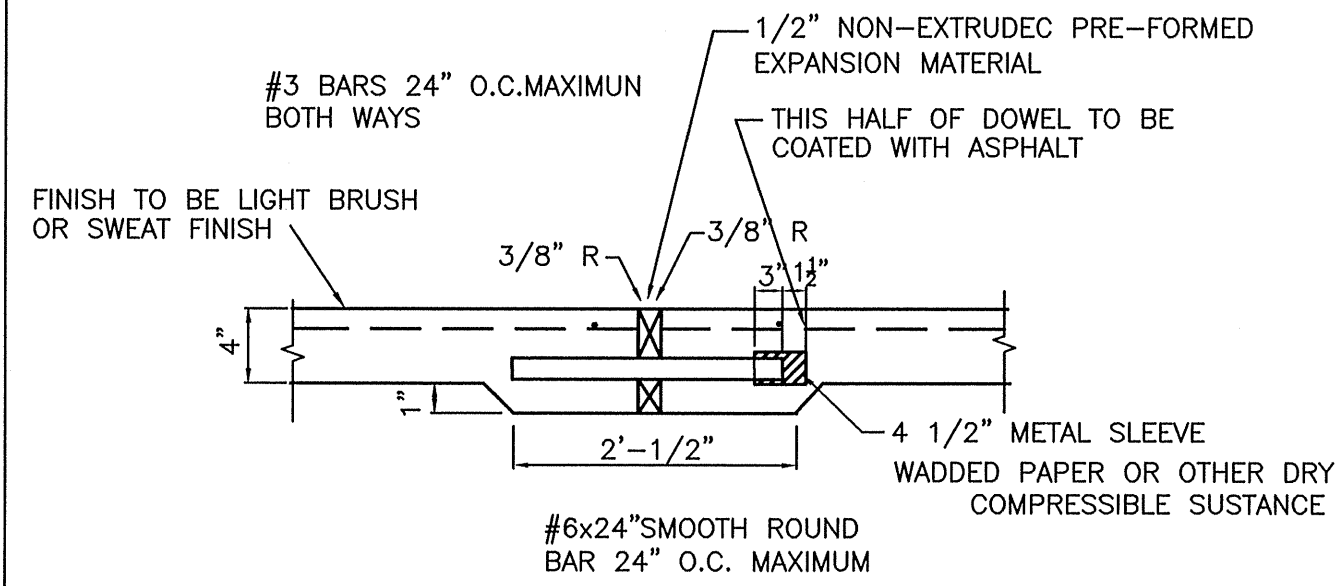
DESIGN	DRAWN	DATE	SCALE	
K&A	K&A	SEPT.2010	N/A	PROJECT No. 10544

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14



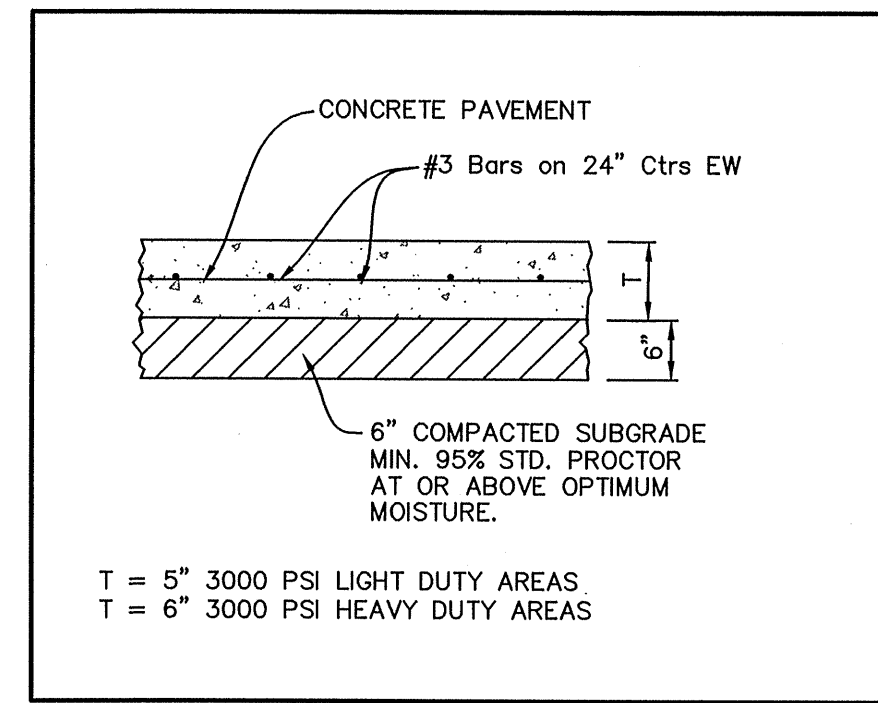


**REINFORCED CONCRETE SIDEWALK**  
NO SCALE

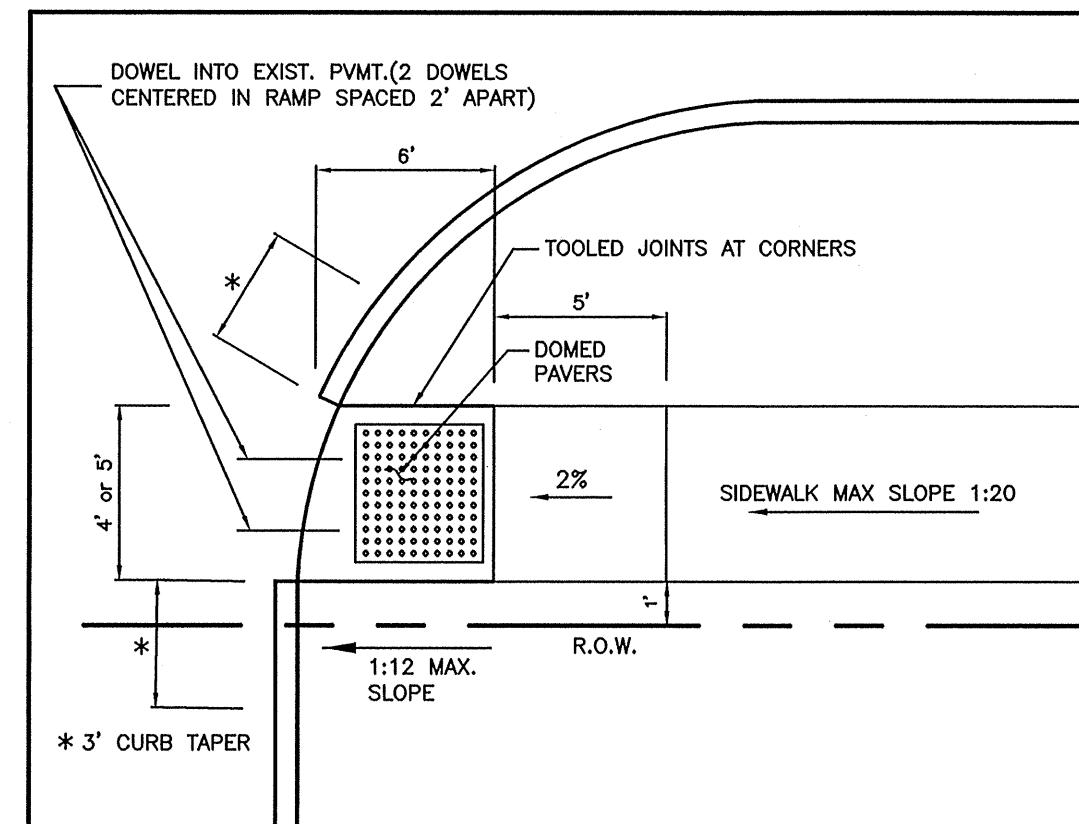


**B-B EXPANSION JOINT DETAIL**  
NO SCALE

- All honeycomb in back of curb to be trowel-plastered before pouring sidewalk.
- Lug may be formed by shaping subgrade to approximate dimensions shown.
- For sidewalks against existing curbs, keyway shall be replaced with 6" long - #3 bars drilled 4" into existing back of curb and epoxy grouted on 24" centers.
- Payment for keyway subsidiary to concrete sidewalk pay item.
- Payment for excavation, borrow, subgrade stabilization and compaction is subsidiary to concrete sidewalk pay item.
- Lime stabilization or select borrow material for subgrade is required when soil P.I. is greater than 18. Limits of subgrade stabilization are minimum required.
- Backfill for sidewalk subgrade shall be lime stabilized soil or select borrow material having a P.I. not less than 10 nor greater than 18.
- Sidewalk backfill and subgrade shall be compacted in lifts not to exceed 6 inches to 95% of ASTM D698 density with a moisture content within -2% to +4% of optimum moisture.
- 1/2" expansion joints shall be provided at street expansion joints and at a maximum 40 foot spacing in line with intermediate street saw joints.

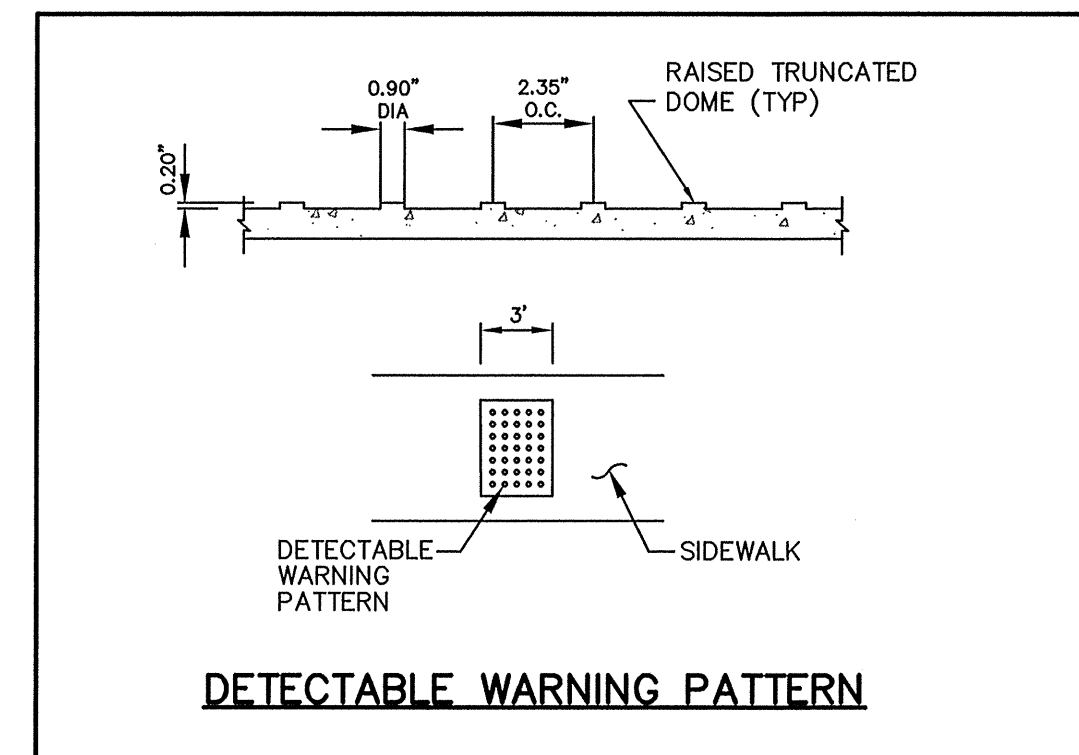


**TYPICAL CONCRETE PAVEMENT SECTION VEHICULAR PARKING**  
NO SCALE

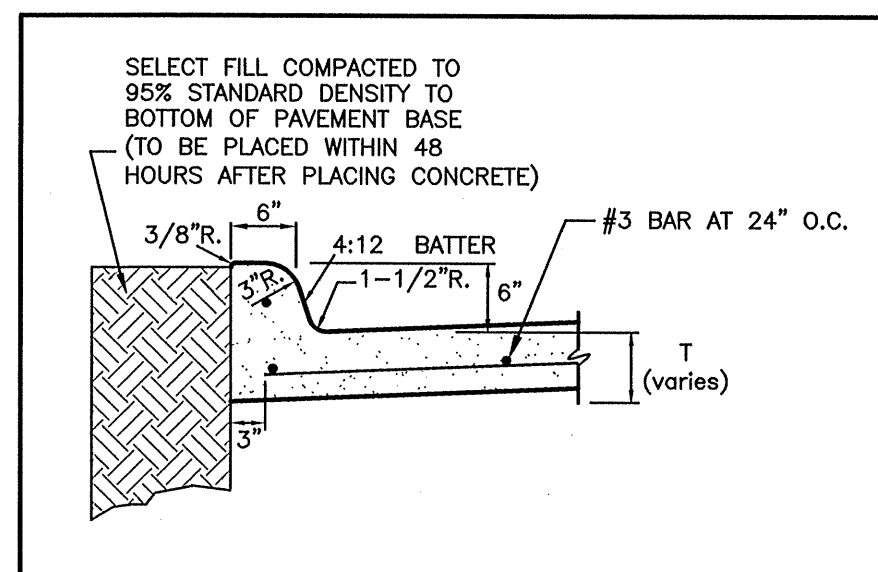


**BARRIER FREE RAMP AT COMMERCIAL DRIVES W/ CURB RETURN**  
P-18

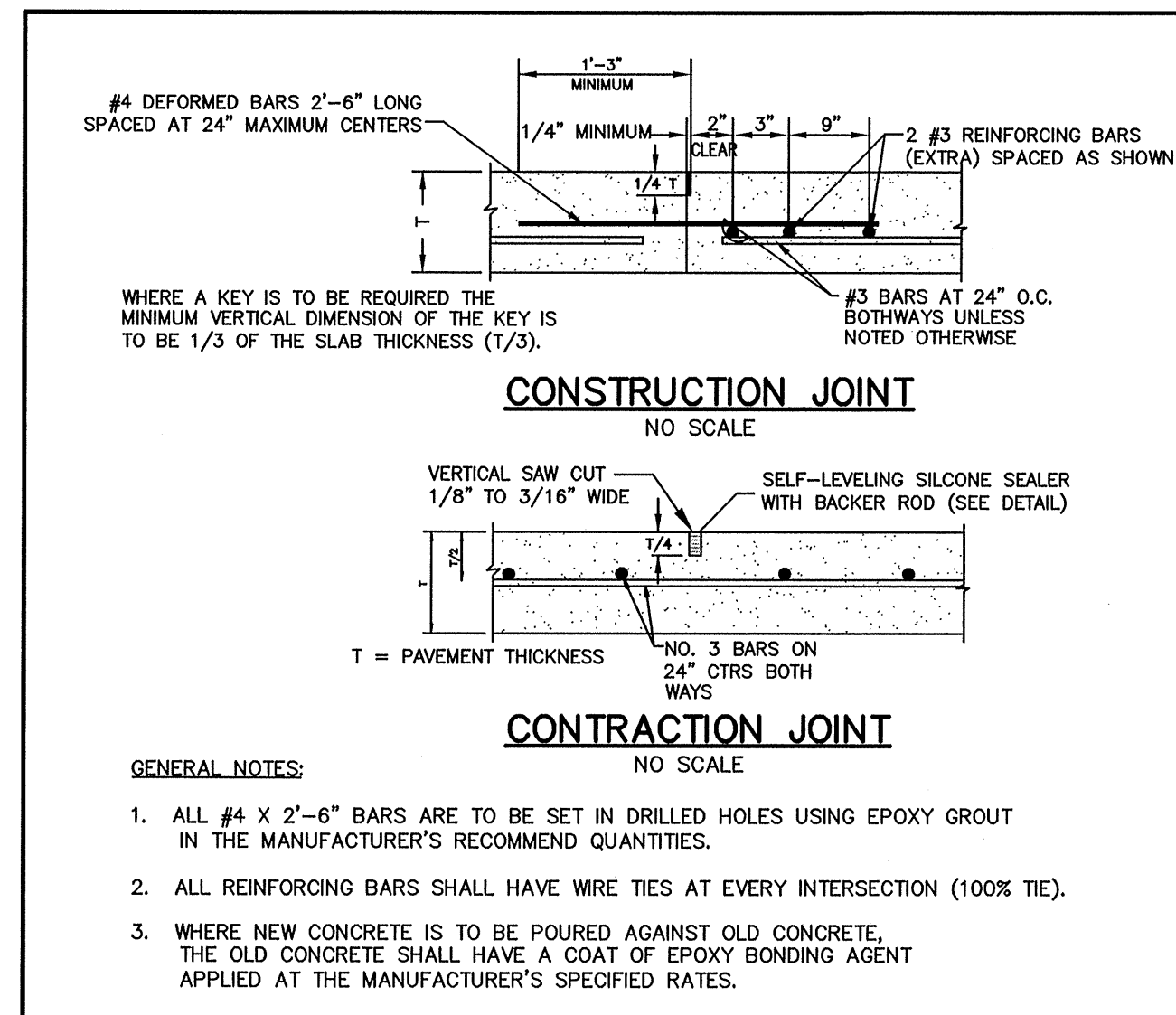
- GENERAL NOTES:
- BARRIER FREE RAMPS SHALL BE CONSTRUCTED AS EXTENSIONS OF STANDARD CONCRETE SIDEWALKS AND SEPARATE FROM THE STREET PAVEMENT, EXCEPT AS NOTED IN THESE DETAILS.
  - MAXIMUM SLOPE ON RAMP PORTION SHALL NOT EXCEED 1" PER FOOT AT ANY LOCATION. VERTICAL DISTANCE BETWEEN STREET & RAMP SHALL NOT EXCEED 1/4".
  - DESIGNS SHOWN ARE FOR 6" CURBS. FOR CURBS WITH HEIGHT GREATER THAN 6", DIMENSIONS SHALL BE INCREASED PROPORTIONATELY.
  - STREETS ON STEEP GRADE WILL REQUIRE LONGER TRANSITIONS ON UPGRADE SIDE.
  - LOCATION OF BARRIER FREE RAMP MAY BE SHIFTED TO CLEAR OBSTRUCTIONS WITH THE APPROVAL OF THE ENGINEERING DEPARTMENT.



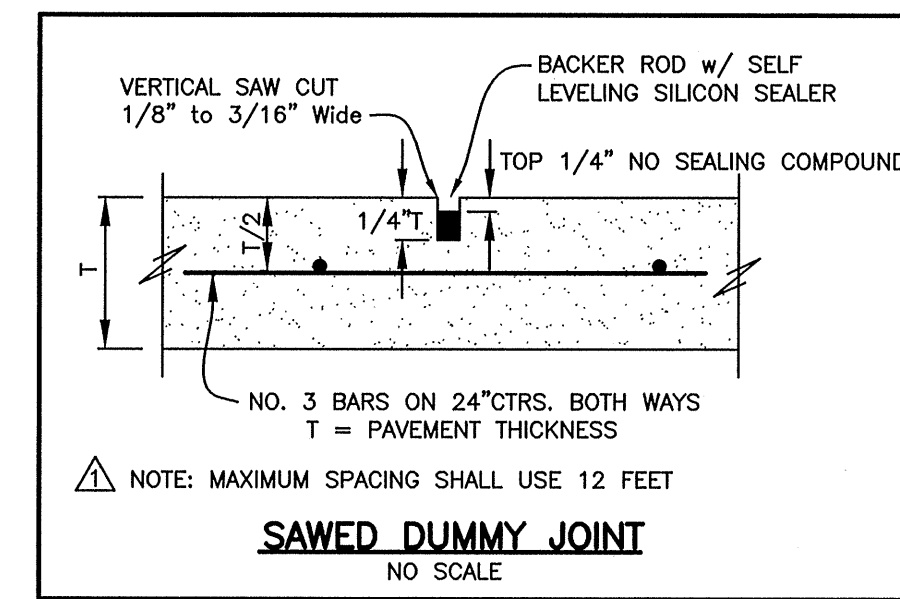
**DETECTABLE WARNING PATTERN**



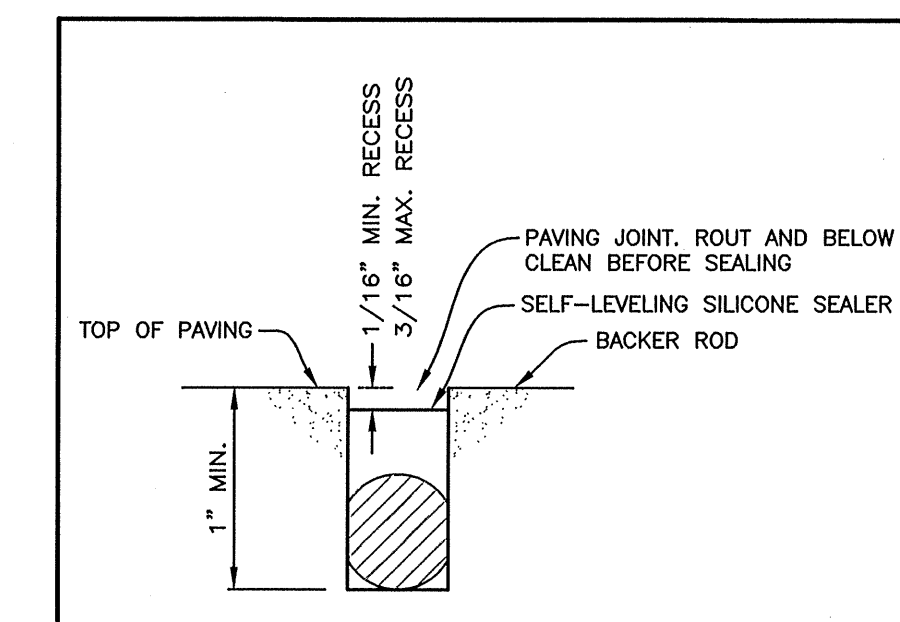
**MONOLITHIC CURB**  
NO SCALE



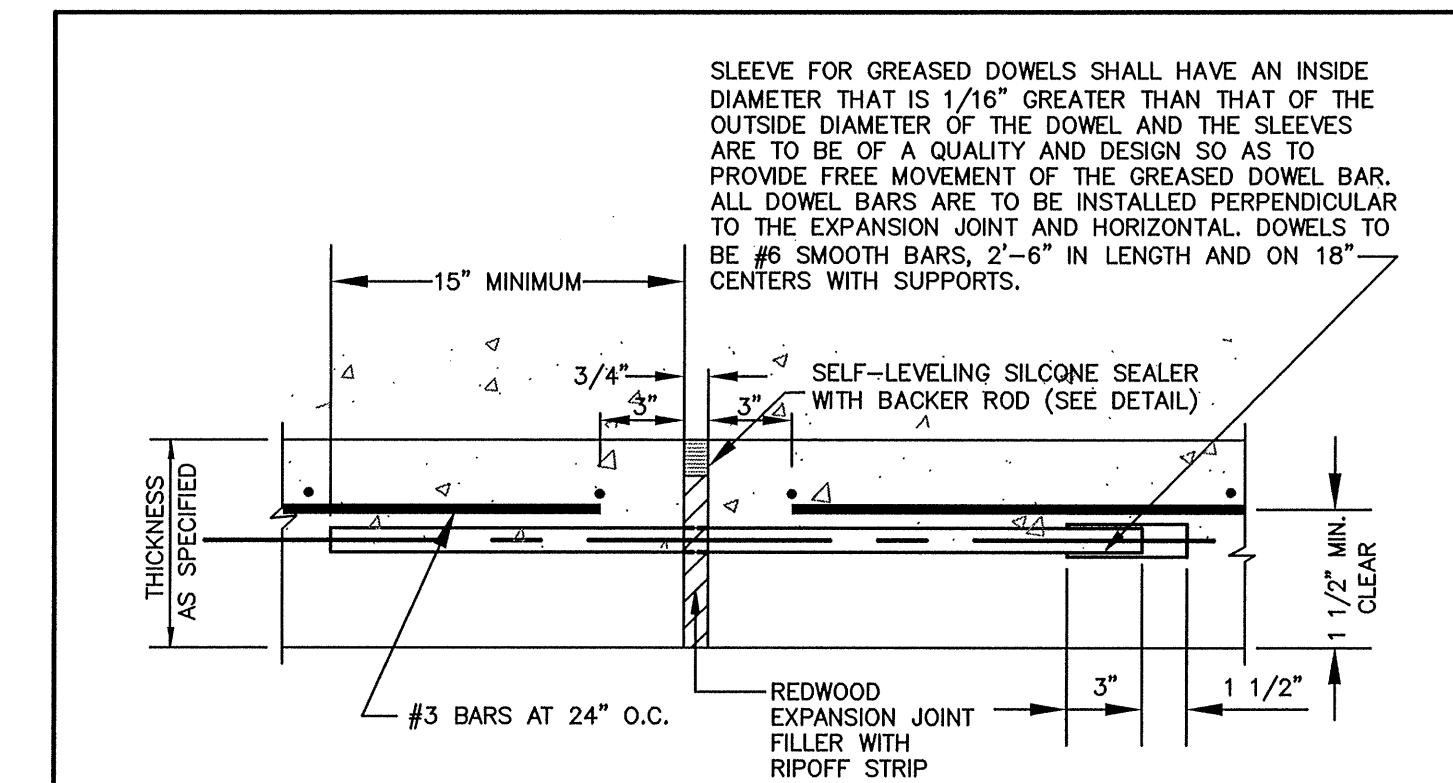
**CONSTRUCTION & CONTRACTION JOINT**  
NO SCALE



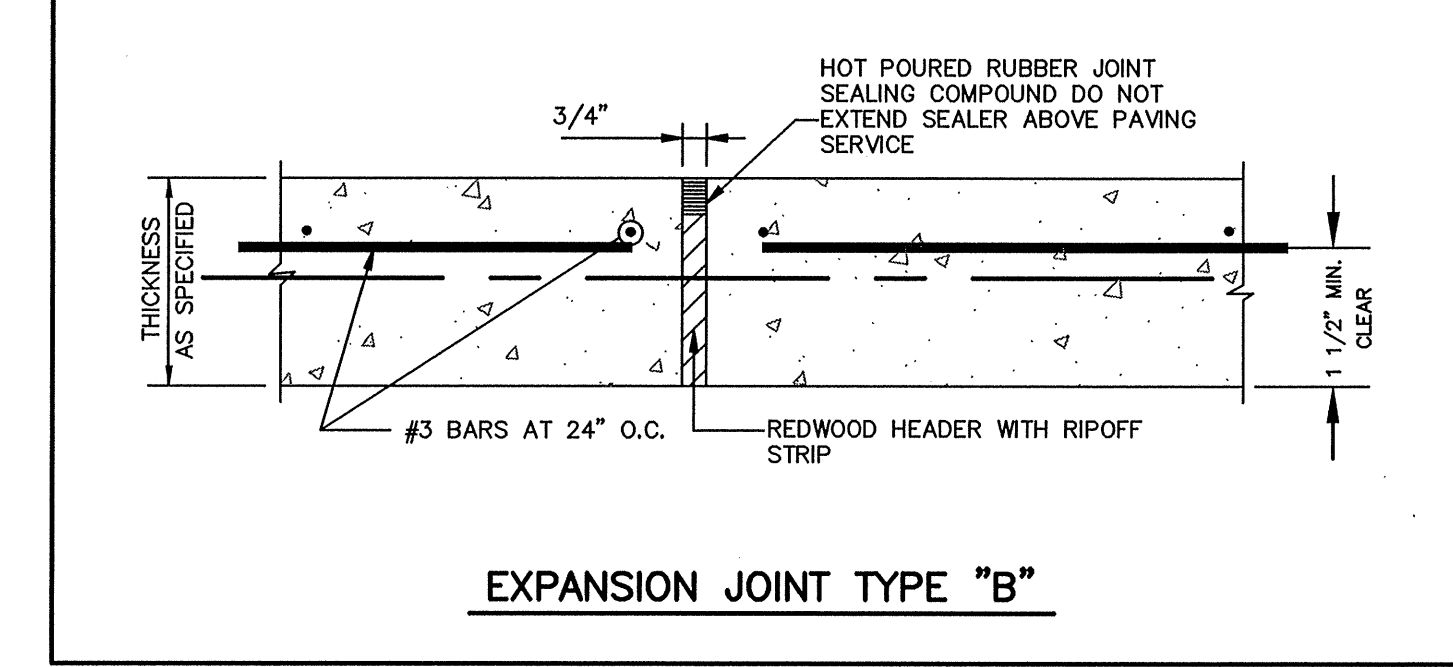
**SAWED DUMMY JOINT**  
NO SCALE



**TYPICAL JOINT SEALING DETAIL**  
NO SCALE

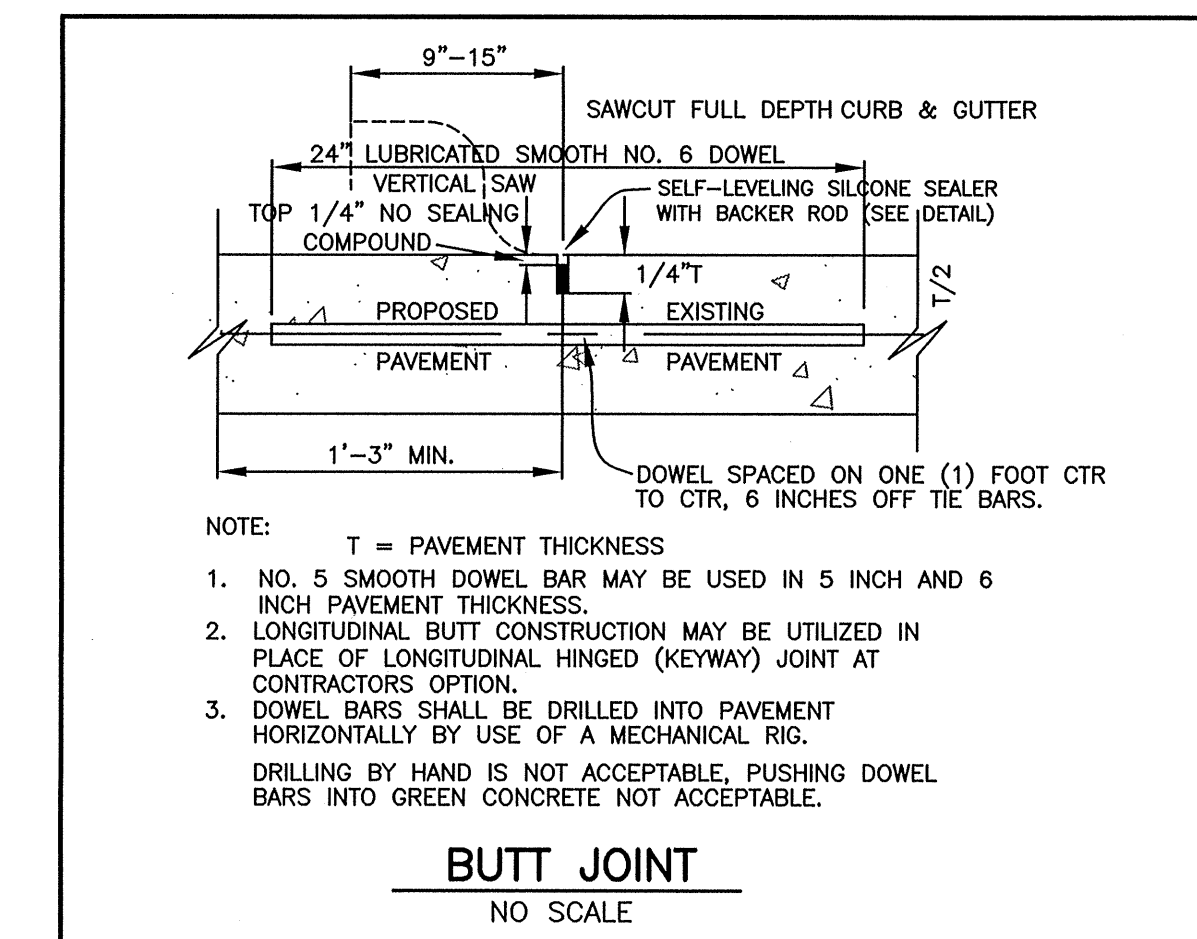


**EXPANSION JOINT TYPE "A"**



**EXPANSION JOINT TYPE "B"**

**EXPANSION JOINTS**



**BUTT JOINT**  
NO SCALE

SEE TOWN OF ADDISON STANDARD CONSTRUCTION DETAILS AND NOTES, WHICH SUPERCEDE THE DETAILS AND NOTES HEREON IF A CONFLICT EXISTS.

NOTE: THE JOINT DETAILS SHOWN HEREON ARE FOR THE DRIVE AND PARKING PAVEMENT ONLY. SEE SHEET 7 FOR RAMP PAVEMENT SECTION AND RAMP PAVEMENT JOINTS.

13  
14

RECORD DRAWING  
DATE: October 26, 2012  
Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.

STATE OF TEXAS  
L. LYNN KADLECK  
ENGINEERING PLANNING SURVEYING  
2000 N. CENTRAL EXPY. SUITE 113  
(972) 881-0771 PLANO, TX 75074  
TBPE Reg. No. F-6460 TBPLS Reg. No. 100555-00

DETAIL AND NOTES				
MILLION AIR - PHASE TWO				
LEASE PARCEL No. 70-WESTGROVE RD.				
ADDISON MUNICIPAL AIRPORT				
TOWN OF ADDISON, TEXAS				
DESIGN	DRAWN	DATE	SCALE	
K&A	K&A	SEPT.2010	1"=40'	PROJECT No. 10544



GENERAL NOTES

- All materials and workmanship shall conform to the Standard Specifications for Public Works Construction for North Central Texas (NCTCOG), latest edition, and the Town of Addison Department of Public Works and Transportation Addendum.
- During the construction of these improvements, any interpretation of the Standard Specifications for Public Works Construction for North Central Texas, and any matter which requires the approval of the owner, must be approved by the Director of Public Works and Transportation or his designee before any construction involving that decision commences. Assumptions about what these decisions might be which are made during the bidding phase will have no bearing on the decision.
- Streets, alleys, sidewalks, driveways and storm drainage facilities in the public right of way shall be constructed in conformance with the Town of Addison Standard Construction Details.
- Contractor shall be responsible for removal and offsite disposal of removed pavement and other construction debris.
- In preparation of the plans and specifications, the engineer has endeavored to indicate the location of existing underground utilities. It is not guaranteed that all major lines, structures or services have been shown or are accurate on the plans. Prior to beginning of construction, the Contractor shall contact all utility companies for verification of existence and location of utility lines.
- The Contractor shall protect all structures, pipelines, walks, drives, trees, shrubbery, lawns and other improvements during the progress of the work and shall remove from the project and dispose of all debris and unused materials.
- The Contractor shall obtain all necessary permits and approvals for his work before beginning construction, and shall maintain a copy of these at the project site. The Contractor shall be responsible for payment of permit fees.
- The Contractor shall be responsible for complying with state laws and federal regulations relating to trench safety. The Contractor shall furnish to the City an acceptable trench safety plan signed and sealed by a Professional Engineer qualified to do such work and registered in the State of Texas.
- Contractor shall maintain traffic flow throughout construction on existing public streets and, if needed, shall submit a detour plan to the City of Coppell for approval prior to start of construction. Contractor shall submit a traffic control plan consistent with the T.M.U.T.C.D.
- Contractor shall seed or sod all disturbed areas upon completion of paving, unless specified differently in the landscape plans.

GRADING AND PAVING CONSTRUCTION NOTES:

- All materials, construction, testing and workmanship shall conform to the Town of Addison Specifications and Standard Construction Details and the Standard Specifications for Public Works Construction for North Central Texas, latest edition, except as noted herein and approved by the Town of Addison.
- All earthwork shall conform to the Geotechnical Report prepared by Reed Engineering Group, Inc. Site preparation and grading of areas within public right of way shall be in accordance with the NCTCOG Specifications as amended by Town of Addison. Site preparation outside public rights of way shall be in accordance with the said Reed Geotechnical Report, but at a minimum shall include: vegetation and organic soils (upper two to four inches) should be stripped and removed at the start of earth work construction. The exposed soils shall be scarified to a minimum depth of six inches and recompacted to a density of between 95 and 100 percent of maximum Standard Proctor Density, ASTM D-698. The moisture content of the compacted soil shall be at +1 to +5% above optimum moisture. Onsite soils used in fill areas outside of the building and pavement shall be compacted in maximum six inch loose lifts to a minimum of 95 percent to an maximum of 100 percent of the maximum Standard Proctor Density, ASTM D-698 at +1 to +5% above optimum moisture.
- The subgrade areas under the proposed ramp shall be as specified by Reed Engineering Group in a letter dated March 15, 2011, which is the top 6 inches as flexible base TXDOT Item 247, Grade 2, Type D or better, compacted to a minimum 95 percent of Modified Proctor (ASTM D1557) at or above optimum moisture, over 6 inch subgrade, scarified and recompacted to a minimum of 95 percent Standard Proctor, ASTM D 698, at or above optimum moisture.
- The proposed ramp pavement shall be in as specified by Reed Engineering Group in a letter dated March 15, 2011, for Option No. 2, which requires 10 inch, 4,000 psi concrete with #4 bars on 24 inch centers each way with jointing as specified on the Ramp Pavement Jointing Plan.
- The onsite pavement areas outside the public right of way, fire lane easements and the proposed ramp shall be in accordance with the said Reed Geotechnical Report, but at a minimum shall be 5 inch, 3000 psi concrete in parking areas, and 6 inch, 3000 psi concrete in traffic areas. The pavement shall be reinforced with #3 bars on 24 inch centers each way (at a minimum). This shall be over a subgrade of 6 inches, scarified and recompacted to a minimum 95 percent Standard Proctor, ASTM D 698, at or above optimum moisture.
- Pavement sections, outside the proposed ramp area, shall be sawed cut at an approximate spacing of 2.5 to 3 times the pavement thickness (for example, for 6 inch pavement, the spacing shall be 15 feet to 18 feet), the actual joint pattern shall be designed to avoid irregular shapes. Refer to "Joint Design for Concrete Highway and Street Pavements" published by the Portland Cement Association for jointing techniques. Sawed joints shall be cut into new concrete within 4 hours after placement, or as soon as practical, but in no case later than 6 hours after placement.
- Finished pavement within Town R.O.W. or public easement shall meet certain quality standards for surface of the concrete including the durability, texture, riding surface and appearance. The surface must be durable, firm, dense and well bonded to the aggregate to maintain an appearance and texture which is satisfactory to the Town. Concrete pavement having a poor surface which has spalled (exposed aggregate) due to poor quality paste, high water-cement ration, over-vibration, improper curing, extreme weather or any other reason, or does not have a satisfactory riding surface shall be removed and replaced at the Contractor's expense. It is extremely important that the pavement have a good rideable surface, free from undulations and rough joints. The Public Works department shall determine the acceptability of the pavement.
- Broom finishing shall be performed so that the corrugation produced in the surface shall be uniform in appearance and not more than 1/8-inch in depth. Brooming shall be completed before the concrete is in such condition that the surface will be torn or unduly roughened by the operation. The finished surface shall be free from rough and porous areas, irregularities and depressions resulting from improper handling of the broom.
- Cracks formed in concrete pavement shall be repaired or removed by the Contractor at the Town's discretion.
- Fire lanes shall be striped in accordance with the Town of Addison requirements. Fire lane striping shall not be placed on curb.
- Traffic Barricades will be required for all construction within the Public R.O.W. Barricades shall conform to the installation identified in the Texas Manual of Uniform Traffic Control Devices, as currently amended. The Contractor shall have the Traffic Control Plan at the time of pre-construction meeting.
- See Town of Addison Standard Construction Details for Additional General Notes.
- Dumpster pad pavement to be minimum of 7 inch thickness.
- Contractor to match existing grades at property lines and not alter or disturb runoff patterns on adjacent property.

ADDITIONAL GENERAL NOTES FOR SIDEWALKS

The Contractor shall ensure all sidewalks in pedestrian and access ways and adjacent to buildings shall comply with the requirements of the American Disabilities Act (ADA). If the Contractor determines there is a discrepancy between information shown on the plans and the requirements of ADA, he shall notify the owner and engineer immediately.

WATER & SANITARY SEWER CONSTRUCTION NOTES:

- All materials, construction, testing and workmanship shall conform to the Town of Addison Standard Construction Details and Specifications for Public Works Construction.
- All Water Mains 6" and larger will be C900 DR14 Polyvinyl Chloride.
- Water mains eight inches (8") to twenty-four inches (24") in diameter shall be AWWA C905 / DR18 PVC, mechanical joint or a joint of the type which provides a recession in the bell for the employment of a single rubber gasket to be placed before the insertion of the succeeding spigot. Joint material for PVC shall Conform to ASTM F477.
- All water Line pipe shall be imbedded per Town of Addison standards.
- MINIMUM COVER:  

PIPE DIAMETER	COVER	PIPE DIAMETER	COVER
Less Than 6"	42"	8"	48"
6"	42"	Greater than 8"	54-60"
- All water lines shall have a 150 psi Hydrostatic test for a four hour period. Test shall be witnessed by the Engineering Inspector. All Fire mains shall have a 200 psi Hydrostatic test for a two hour period.
- All water mains must be chlorinated by the contractor and water samples then must be taken and provided to the Town of Addison.
- All water line valves, main line taps, fittings and fire hydrants shall be placed in a concrete saddle and thrust blocks.
- All valve boxes shall be adjustable Cast Iron type and shall be placed on a concrete base. A reinforced concrete pad of 3'-0"x3'-0"x6" shall be poured around the valve box in areas outside of the pavement.
- Fire hydrants shall be placed to conform with the requirements of the State Board of Insurance and Fire Prevention and Engineering Bureau of Texas. The fire hydrant shall be set perpendicular, and to the proper depth, and shall be carefully and substantially blocked against firm trench walls using 2000 psi concrete.
- Sanitary sewer mains shall be polyvinyl chloride pipe conforming to the specifications of ASTM D 3034, SDR 35, or equal. Joints and fittings shall be compression rubber gasket joints.
- Sanitary sewer pipe shall be placed on a 4" layer of crushed stone. The trench shall be back filled per Town of Addison standards and consolidated to a minimum of 90% standard proctor density.
- Testing for sanitary sewer system shall be per the North Central Texas Council of Governments Standard Specifications.
- All water and sanitary sewer service locations shall be marked on the nearest curb face with a "W" & "S" respectively.
- All plumbing installed outside of R.O.W. or Easement shall be installed by a Licensed Plumber and inspected by Building Inspection.
- Fire sprinkler line shall be sized and installed by a State Licensed Contractor.
- The Contractor shall tie detectable green metallic tape to the end of sewer service or buried dead end and shall leave a minimum of 36" of tape exposed after backfill is complete.
- The Contractor shall be responsible for providing "As-Built" plans to the design engineer of record showing the location of water & sanitary sewer services.
- Contractor shall have a Trench Safety Plan.
- See Town of Addison Standard Construction Details for Additional General Notes.

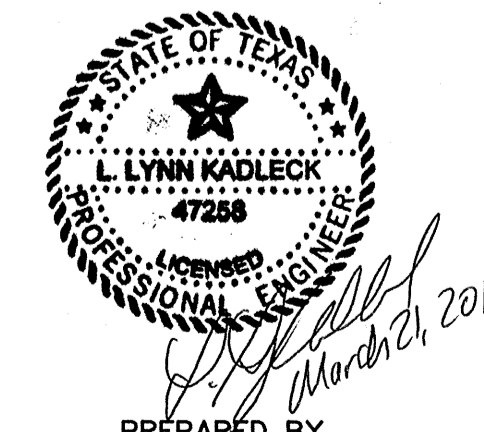
STORM SEWER CONSTRUCTION NOTES:

- All materials, construction, testing and workmanship shall conform to the Town of Addison Specifications and the Standard Specifications for Public Works Construction for North Central Texas, latest edition, except as noted herein and approved by the City.
- All public storm sewer pipe shall be a minimum of 18" and shall be Class III or Class IV RCP based upon depth of cover. Class III RCP shall be used where the depth of cover, from ground line to top of pipe, is less than or equal to 10 feet. Class IV RCP shall be used where the depth of cover is greater than 10 feet. All storm sewer pipe shall be laid on a minimum of 4" of Grade No. 4 crushed stone (1" maximum diameter). The initial backfill shall consist of the same crushed stone to a minimum of the spring line of the pipe. The remainder backfill must be clean and free of rocks & lumps of earth larger than 4" and of vegetation.
- The joints shall be constructed and jointed together in a manner that eliminates spill through of backfill. Approved joint materials are concrete collars; plastic asphalt joint compound (cold applied); rubber gaskets; and preformed plastic gaskets (cold applied).
- The tops of all storm drainage inlets, manholes and junction structures shall have a round manhole cover with locking device.
- All drainage structures shall have a minimum compressive strength of 3600 psi at 28 days.
- All precast box culverts, drainage structures and RCP will require a certification from the manufacturer that the product meets the design requirements and 28 day compressive strength.
- All storm sewer systems with radii less than 100' shall use 4' long joints with beveled ends (radius pipe).
- See Town of Addison Standard Construction Details for all General Notes.

SEE TOWN OF ADDISON STANDARD CONSTRUCTION DETAILS AND NOTES, WHICH SUPERCEDE THE DETAILS AND NOTES HEREON IF A CONFLICT EXISTS.

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RECORD DRAWING  
 DATE: October 26, 2012  
 Based on information furnished from the Contractor and to the best of the design engineer's knowledge, the constructed work is substantially in accordance with this plan.



PREPARED BY  
**KADLECK & ASSOCIATES**  
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 (972) 881-0771 PLANO, TX 75074  
 TBPE Reg. No. F-6480 TBPLS Reg. No. 100555-00

DETAIL AND NOTES				
MILLION AIR - PHASE TWO				
LEASE PARCEL No. 70-WESTGROVE RD.				
ADDISON MUNICIPAL AIRPORT				
TOWN OF ADDISON, TEXAS				
DESIGN	DRAWN	DATE	SCALE	
K&A	K&A	SEPT.2010	1"=40'	PROJECT No. 10544