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grachter Γ 214-343-9490. Bect Line + 214-343-9490. FAX: 214-341-9060)



Public Works / Engineering 16801 Westgrove • P.O. Box 144 Addison, Texas 75001 Telephone: (214) 450-2871 • Fax: (214) 931-6643

TO <u>RICHARD KING</u> <u>FAX: 214-341-9060</u>

LETTER OF TRANSMITTAL

| DATE 2 | 112/01 | JOB NO. | |
|-----------|-----------|-----------------------------|--|
| ATTENTION | RICHAR | D KING | |
| RE: | BETUNE | "" LURNETOR | |
| | DRIVE RED | 'Y SURVEYOR RHIRE MELTES | |
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GENTLEMAN:

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| COPIES | DATE | NO. | DESCRIPTION | | |
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| | | | TABLES & DRIVE TYPICAL DATA | | |
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REMARKS _

COPY TO ______

1 Killsert SIGNED:

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If enclosures are not as noted, please notify us at once.

TABLE <u>21</u> (9)

Desirable Minimum Driveway Spacing

| | Street Type | Speed Range, MPH | Minimum Spacing Between Driveways, Feet ^{a,b} |
|-----------|----------------|------------------------|--|
| BETTUNE- | Arterial | 35-40 | 200 |
| SURVETOR- | Collector | 30-35 | 150 |
| | Local | 25-30 | 100 |

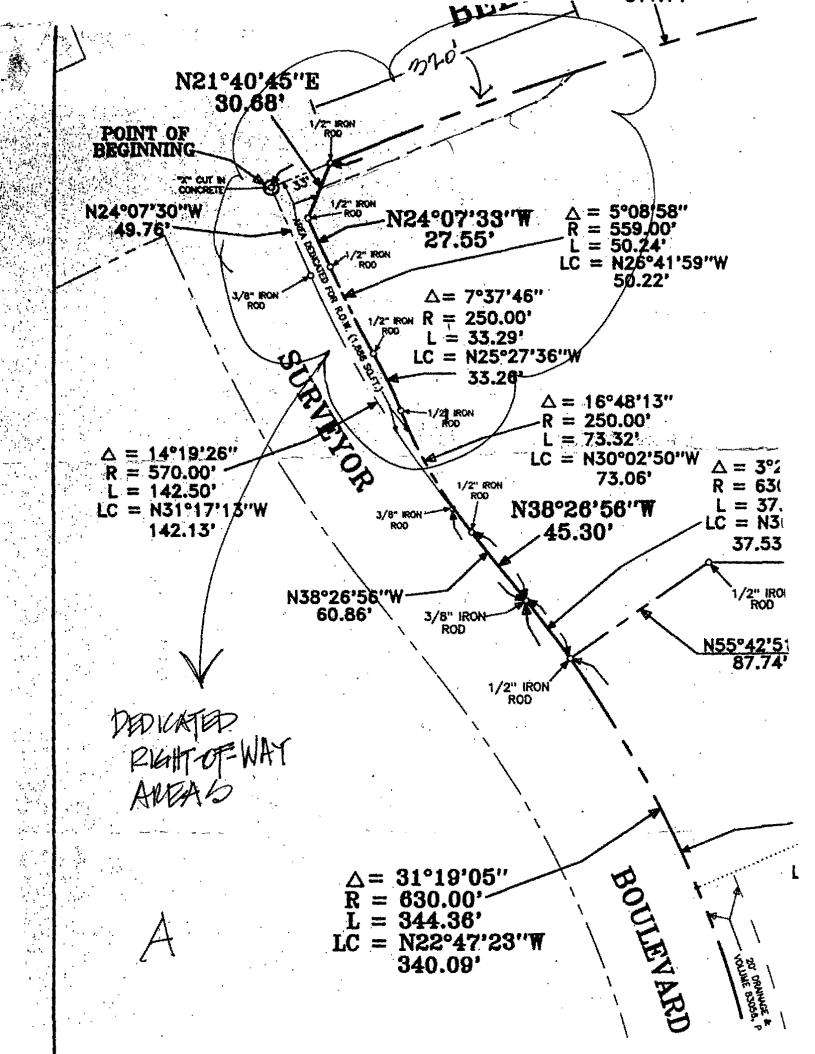
^aThese spacings are measured from driveway throat to driveway throat as shown by "J" in Figure 2.

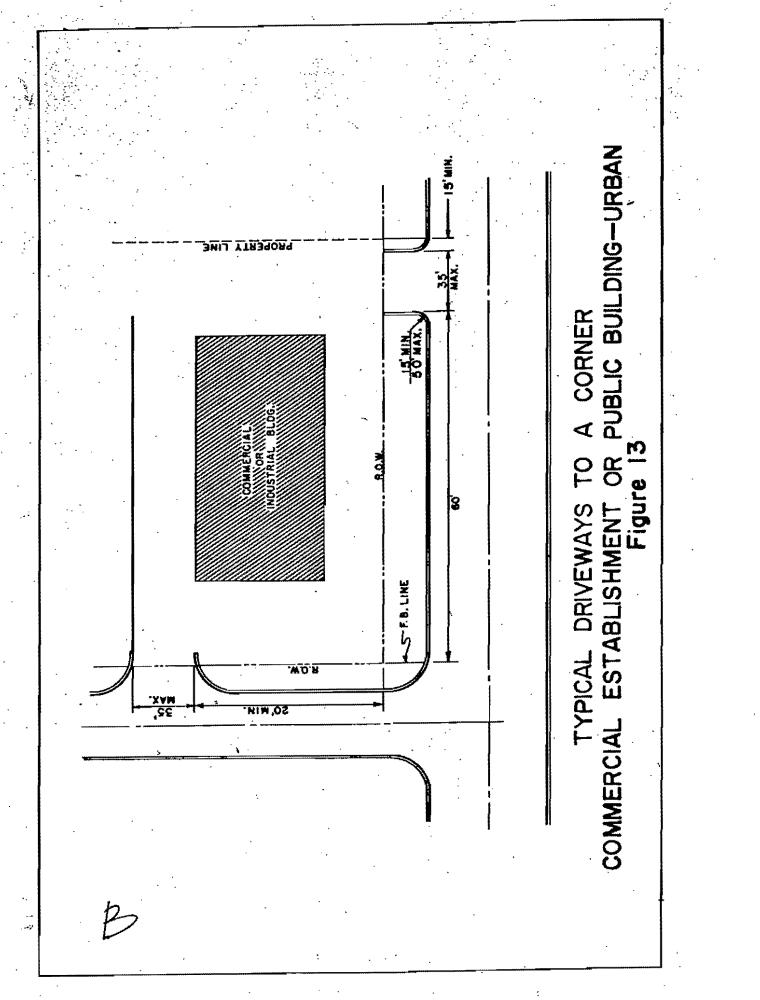
^DThe minimum spacings can be halved if the adjacent driveways

are operated as a one-way pair.

The above minimum driveway spacings (Table 21) will prevent conflicting movements at adjacent driveways from overlapping. It is recognized that at some sites these minimum values may be too restrictive, and if justified by an engineering study of site conditions, lesser spacings may be used. But, under no conditions should commercial and industrial driveway spacings be reduced below the critical minimum values stated in item "J" of Figure 2 for the various street classifications.

In addition, as depicted below, it is desirable to space driveways to reduce the potential conflicts that could arise between vehicles entering the street system from a driveway and those on the street itself. These spacings were determined using a running speed of 30 mph for the through lanes, 15 mph for a turning speed from intersecting streets, and an acceleration of 3.0 ft/sec^2 . The higher limit represents the spacing that allows the second vehicle to proceed without reducing speed and the lower limit would require the second vehicle to alter speed or direction.





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| · · | s . t i t . | 1.1.1 | | ų. 1. | · · · |
|-----------------------|-------------|----------|------------|-------------------|----------|
| · · · · | | roach-Ge | onetry (1 | (eet) | • |
| Arterial peed (mph | <u>}</u> | L | <u>, n</u> | ¥2 ⁽²⁾ | <u>_</u> |
| 45 | 120 | 250 | | 30 % | 20 · |
|) 10 | 120 | 200 | 12 | 32 | 15 |
| ., 35 | 90 | 120 | 12 | 34 | 10 |
| -, | | × | . · | | • |

 $\frac{H_3^{(3)}}{28} \quad \frac{R_2}{20} \quad \frac{L_3}{30}$ 30 15 15
30 25 0

Departure Geometry (feet)

0

.5

5

0

16.2.7

•

- (1) Excluding curb and gutter
- (2) Back-to-back of curb's total width, entrance width, N₂, divided by solid white line, lane line to back of curb as follows:
 - Wz
 Right Lane
 Left Lane

 30
 16.5
 13.5

 32
 18.5
 13.5
 - 34 20.5 13.5

(3) Total width back to back of curb; divided solid white line as follows:

| . ¥3 | | Right Lane | | Left Lane |
|-------|-----|------------|---|-----------|
| 28 | · • | 14.5 | • | 13.5 |
| · .30 | | 16.5 | • | 13.5 |

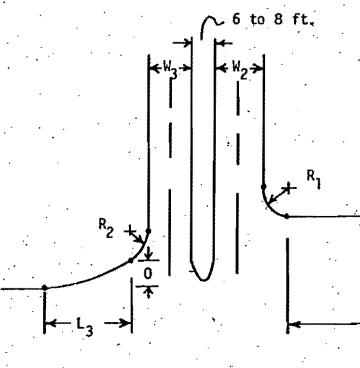


FIGURE 37. SUGGESTED DESIGN FOR DIVIDED DRIVEWAY ACCESS TO ARTERIAL STREETS (8)

Section 7 Guidelines for pirect Access prive-wars

driveway.

 Residential Driveway Access to Arterial Streets. Residential driveway access to arterial streets should not be permitted except for major multiple family "cluster" developments.

Frequency of direct access driveways: Functional Classification Urban Rura] Primary Arterial. Major generators under 2 per mile per side, specific conditions farm residences and field drives only Secondary Arterial 1 per 200 ft. of 6 per mile per side frontage Collector 2 per 100 ft. of No restriction on number frontage Local (Residential) No Control No Control

• Driveway Spacing. Driveways should be spaced a minimum of 200 feet (example for 40 mph arterial) apart to provide safe traffic operation on arterials and driveways during all periods (peak and off-peak) of the day. In some instances, where such spacing would create a hardship, but where the average spacing of driveways within 300 feet of the subject property could be maintained at 200 feet, a minimum spacing of 150 would be permissible.

Where such spacing control cannot readily be achieved within a particular parcel, joint access with an adjoining property should be sought.

If officials are satisfied that sufficient attempts to secure joint access have been made and that joint access is still not possible, and access cannot be provided via another street, driveway access to the arterial may be granted if minimum corner clearances are met. However, this access should be limited to right turns in and out (left turns in and out prohibited).

• Corner Clearance (example for 35 mph arterial). The minimum tangent curb length between a driveway and an intersection of the arterial with a cross street should be 50 feet. If the intersection is or is likely to be signalized, then traffic movements to and from any driveway within 125 feet of an inter-

section with a collector and 250 feet of an intersection with an arterial should be limited to right turns only.

• Property Clearance. The minimum distance between the property line of a parcel and the nearest edge of the nearest driveway to that property line should be 100 feet, except if the driveway provides joint access to more than one parcel. A joint access driveway may be located on the property line.

 Sight Distance. Adequate sight distance should be available at every driveway. Any movement for which adequate sight distance is not available should not be permitted. Joint access or access to another street should be sought in such cases.

• Median Openings. If and when medians are constructed on any arterial (street) spacing between median openings should be at least 400 feet. The spacing may be reduced to 300 feet if a competent traffic study shows that a lesser spacing will still safely and efficiently accommodate left turn movements to existing and projected future development in the immediate vicinity.

• Number of Driveways. Each parcel should be permitted access through one driveway, either on the parcel or as part of joint access. Additional driveways may be needed and provided under the following conditions:

1. If the daily volume using one driveway would exceed 5,000 vehicles (both directions).

2. If traffic using one driveway would exceed the capacity of a stop sign controlled intersection during one peak street traffic hour on the peak site traffic hour.

3. A competent traffic analysis shows that traffic conditions warrant two or more driveways.

In all cases, minimum spacings and clearances should be provided. For major traffic generators, it may be more appropriate to signalize certain driveway intersections than to provide more non-signalized driveways. Any driveway signals should be located to provide proper spacing of signals.

• Driveway Layout. The configuration or arrangement of driveways along a street can influence traffic operations at individual driveways. For this reason, driveways should be located, designed, and operated to function as a coordinated system of access points. Every time a new driveway is constructed, it must be coordinated with existing driveways. It is particularly important to fully coordinate the location, design, and operation of all driveways serving the same development.

Some basic principles relating to driveway arrangement and layout to consider in developing a coordinated system of access points are discussed below:

1. Motorists are very hesitant to violate normal one-way traffic operation (enter on the right and exit on the left), if it is implied by the driveway layout. Two driveways serving the same development located side by side, strongly imply normal one-way operation. Motorists will tend to enter using the driveway on the right (looking from the street) and exit using the driveway on the left (looking from the street). To a lesser extent, motorists even associate normal one-way operation with three driveway configurations. They will tend to use the far right driveway for entrance maneuvers, the far left driveway for exit maneuvers, and the middle driveway for both.

2. If a particular driveway configuration does not imply normal one-way operation, drivers tend to enter using the first driveway they reach. They tend to exit using this same driveway if returning to the point of origin of their trip.

3. Drivers will tend to avoid driveway maneuvers which are difficult, hazardous, or illogical. They avoid driving behind a building or through an intersection to use the "proper" driveway if access to the "improper" driveway is more convenient and use of this driveway is not physically restricted.

4. A turning maneuver may be discouraged by geometric design features (i.e., small turning radius, angled driveway, etc.), but design features alone generally do not imply to motorists that the discouraged maneuver is illegal or prohibited. Some motorists will invariably violate the intended meaning of the design if signing and/ or pavement markings are not used. Even then, some violations may occur.

• Type of Operation (Two-Way Versus One-Way). A two-way driveway can efficiently service several hundred vehicles per hour (vph) if properly designed and controlled. The capacity of a two-way driveway is essentially the same as a stop controlled "T" intersection.

If peak hour volumes exceed 100 vph, however, a divided driveway or one-way driveway pair should be considered inlieu of a two-way driveway. One-way operation is particularly desirable when there are over 40 left turns per hour at a driveway and property frontage exceeds 200 feet in length.

Driveway design must be completely coordinated with the type of driveway operation. Separate design standards should be used for two-way and oneway driveways.

• Number of Lanes per Driveway. To a great extent, the width for ingress movements will be determined by the turning requirement. Egress width will be determined by peak turning volumes. Generally, if egress left turns exceed 100 per hour, two egress lanes should be provided. Otherwise, one lane will be sufficient.

• Prohibition of Turns. Left turns should be prohibited to and/or from driveways under the following conditions:

1. Inadequate corner clearance (prohibit left turns to and from)

2. Inadequate sight distance (prohibit left turns with inadequate

sight distance)

3. Inadequate driveway spacing (prohibit left turns to and from)

4. Median opening too close to another median opening (prohibition dependent on specific location of adjacent openings)

5. Other capacity, delay, or safety conditions identified by agency making specific left turns detrimental to public interest.

Most desirably left-turn prohibitions are implemented physically with median channelization (if a median exist) or driveway channelization. Signing should also be installed as necessary. In cases where multiple driveways with limited turns are needed, right-turns may need to be prohibited. Effective channelization should be provided for such driveways.

 Left-Turn Lanes. When the peak hour, left turn warrant is met, left turns lanes with the appropriate storage length should be provided.

 Right-Turn Lanes. A right-turn deceleration lane should be installed at each driveway with an average daily volume of at least 1,000 vehicles and an average peak-hour inbound right-turn of at least 40 vehicles.

Where several successive driveways meet the above warrant or where driveway spacing is not adequate to avoid encroachment of the right-turn lane or another driveway, a continuous right-turn lane should be used. Continuous right-turn lane should also be provided when 20 percent of the directional volume on an arterial (35 mph or higher) makes right turns.

• Parking. Curb parking should be prohibited on all arterial streets (in some cases, localized parking conditions make this impossible).

• Frontage Roads (if applicable). All driveway access along arterials with existing or planned frontage roads should be provided to (future) oneway frontage roads. To gain <u>temporary</u> direct access to the arterial (prior to improvement of that facility and construction of the frontage road system), the petitioner should construct the section of frontage road adjacent to his/ her property. This frontage road section should be located where planned.

Any right-of-way within 100 feet of the arterial centerline not previously dedicated should be dedicated prior to issuance of a temporary direct access (to the arterial) driveway permit.

When driveway access is provided from frontage roads, driveway spacing and property clearance and minimum lot width requirements may be reduced by one-third. However, minimum driveway spacing for temporary direct access to the arterial should be adjacent to insure safe traffic operation at the design speed.

• Special Land-Use Requirements. Certain land uses will require driveways which do not totally conform to the guidelines stated previously. Examples of these land uses are service stations and drive-in banks. Applications for permits for "non-conforming" driveways for any such land use should be accompanied by a traffic engineering study technically justifying the nonconforming features. However, in no case should variations in corner clearances be permitted, since they are critical to safe, efficient intersection operation.

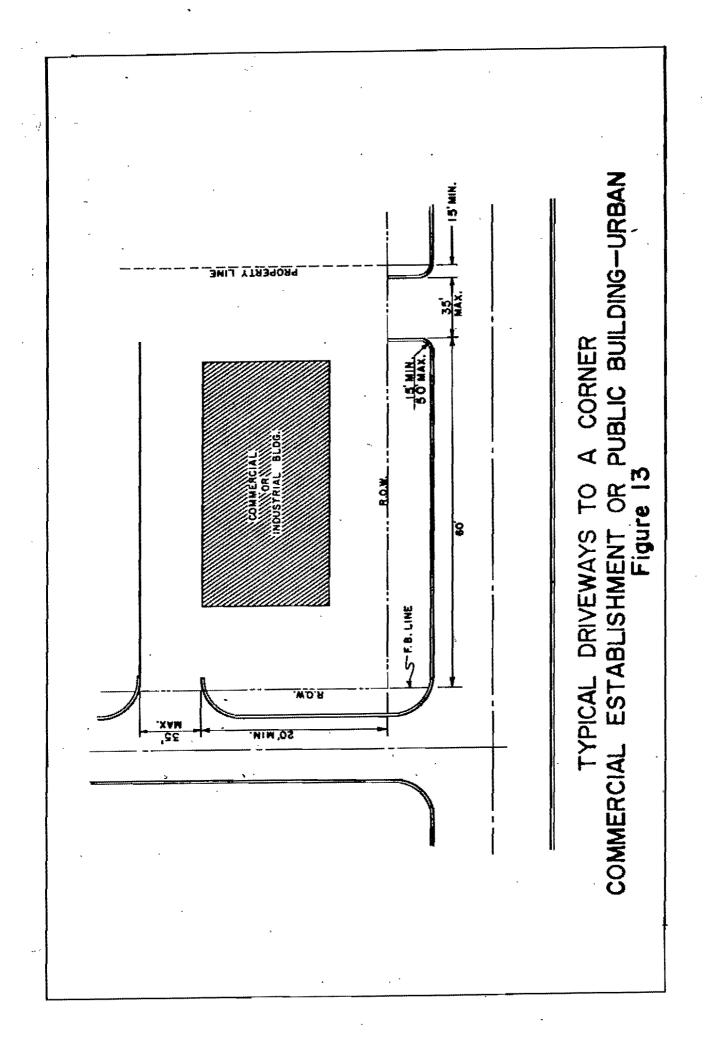
In summary, there are numerous cost-effective ways to preserve or increase roadway capacity, and safety. These are listed in Table <u>19</u>. Many of these methods can be used to improve existing conditions as well as for new driveways. (4)

TABLE <u>19</u>

Cost-Effective Techniques to Preserve Roadway Capacity and Safety

140.00

| lechnique | Coanent |
|--|---|
| apacity | |
| rovide two driveway exit lanes rather than one. rovide additional driveway. rovide left-turn lane on arterial. rovide left-turn deceleration lane. rovide continuous right-turn lane. rovide direct access only from frontage roads. ignalize driveway intersection. | Driveway ADT over 1,000. Driveway ADT over 5,000. Varies. 35 mph, 40 or more peak-hour right turns. 35 mph, 20 percent right turns during day. 40 mph, 20,000 ADT, short frontages. See Narmal on Uniform Draffie Control Daviece |
| alety | |
| rohibit parking on arterial streets. rovide adequate driveway entrance width. rovide two driveways with limited turns rather than two standard driveways. rovide two one-way driveways rather than one two-way driveway. | All locations. Provide minimum 15 mph turning speed. "T" drive-ay intersections, 40 left turns in one direction, 200-foot minimum frontage. Same as above. |
| ennelize driveways or install median to prohibit selected movements. | Less than 100 prohibited turns per day, high driveway densities or driveway close to intersection. |
| nsure adropate sight distance | See AASHTO Folicy on Geometric Design of Rura Righways. |
| pacity and Salety | |
| nimum driveway spacing. nimum corner clearance. | See Table 2. 50 feet or distance based on Table 3, which- ever is greater. |
| nimum property clearance. avide access from collector street in lieu of access from arterial street. nsolidation of access and connections between adjacent properties. | One-half values in Table 2. Corner residential parcels, corner parcels requiring more than one driveway. Frontages too short to permit minimum spacing. |
| ovide adequate internal circulation and parking space. | Always. |
| | |
| | |



| | Approach-Geometry (feet) | | | | |
|---------------------------|--------------------------|-----------|--------------|----------------|-----------|
| Arterial . Speed (mph) | <u>.</u> | <u>L2</u> | <u>H</u> ()) | . <u>4</u> (2) | <u>Rj</u> |
| 45 | 120 | 250 | . 12 | 30 | 20 |
| · 40 | 120 | 200 | 12 | 32 | 15 |
| 35 | 90 | 120 | · 12 | 34 | 10 |
| - | , | | | | • |

(1) Excluding curb and gutter

R₂

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(2) Back-to-back of curb's total width, entrance width, My, divided by solid white line, lane line to back of curb as follows:

| . ¥, | Right Lane | Left Lane |
|------|------------|-----------|
| 30 - | 16.5 | 13.5 |
| 32 | 18.5 | 13.5 |
| 34 | 20.5 | 13.5 |

to 8 ft.

R₁

War

(3) Total width back to back of curb; divided solid white line as follows:

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. 28

.30

: 30

Departure Genetry (feet)

,10

15

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5

5

0

R₂

20

15

25

| į | <u>¥3</u> | | Right Lane | Left Lane |
|---|-----------|-----|------------|-----------|
| | 28 | | - 14.5 | 13,5 |
| ţ | 30 | ; ` | 16.5 | 13.5 |
| | | | • | |

· · · · ·



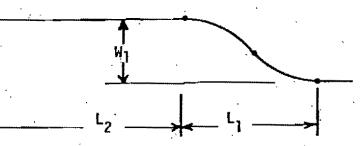


FIGURE 37. SUGGESTED DESIGN FOR DIVIDED DRIVEWAY ACCESS TO ARTERIAL STREETS (3)

TABLE 21 (9)

| Street Type | Speed Range, MPH | Minimum Spacing Between Driyeways, Feet ^{a,D} |
|----------------|------------------------|--|
| Arterial | 35-40 | 200 |
| Collector | 30-35 | 150 |
| Lócal | 25-30 | 100 |

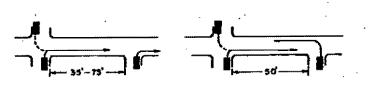
Desirable Minimum Driveway Spacing

^aThese spacings are measured from driveway throat to driveway throat as shown by "J" in Figure 2.

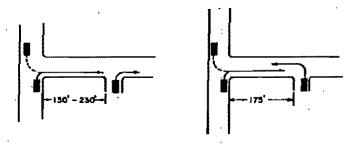
^bThe minimum spacings can be halved if the adjacent driveways are operated as a one-way pair.

The above minimum driveway spacings (Table <u>21</u>) will prevent conflicting movements at adjacent driveways from overlapping. It is recognized that at some sites these minimum values may be too restrictive, and if justified by an engineering study of site conditions, lesser spacings may be used. But, under no conditions should commercial and industrial driveway spacings be reduced below the critical minimum values stated in item "J" of Figure <u>2</u> for the various street classifications.

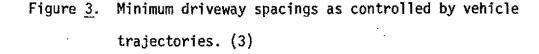
In addition, as depicted below, it is desirable to space driveways to reduce the potential conflicts that could arise between vehicles entering the street system from a driveway and those on the street itself. These spacings were determined using a running speed of 30 mph for the through lanes, 15 mph for a turning speed from intersecting streets, and an acceleration of 3.0 ft/sec^2 . The higher limit represents the spacing that allows the second vehicle to proceed without reducing speed and the lower limit would require the second vehicle to alter speed or direction.



Specings of Intermedicia Orlywycy and Stop Street



Spacings at Intersection With Through Street



The spacing between a driveway and an adjacent street intersection (corner clearance) should be regulated to enhance traffic safety and operations in the intersection area. However, similar to driveway spacing, corner clearance control is sometimes difficult due to the existence of corner properties with insufficient frontage lengths to enable good driveway location. Again, subdivision standards should recognize corner clearance requirements.

Corner clearance requirements for commercial and industrial properties or collector - local streets and the various intersection combinations for arterial streets are shown in Table 22 and Table 23.

> TABLE <u>22</u> (9) Collector and Local Street Miminum Corner Clearance

> > Minimum Corner Clearance, Feet^a

| Street Type | Desirab]e Minimum ^a | Critica] Minimum ^D |
|----------------|-----------------------------------|----------------------------------|
| Collector | 100 | 50 |
| Local , | 100 | 50 |

TABLE 23 (9)

Arterial Minimum Corner Clearances

| | Mimimum Corner | Clearance, Feet ^a |
|----------------------|----------------------|------------------------------|
| Intersection Type | Desirable Minimum | Critical Minimum |
| ×Arterial-Arterial | 300 | 250 |
| Arterial-Collector | 200 | 125 |
| Arterial-Local | 100 | 50 |

^aCorner clearance is measured from the intersection curb to the near driveway curb.

^bDesirable minimum corner clearance is the clearance needed to assure that conflicting driveway and intersection movements do not overlap.

Another significant reason to ensure adequate corner clearance is the effects on the street system created by driveway blockage. As shown below, driveways located too close to an intersection can be blocked by vehicle queues on the street system causing delay to the driveway user in making an exiting maneuver. The opposite is also true when vehicles desire to enter a driveway that is blocked by a vehicle queue. In this case, the potential exists for delay to occur on two different streets as shown below.

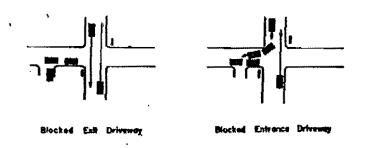


Figure 4. Blocking of near corner driveways. (3)

Based on the principles of queueing theory, Table <u>24</u> depicts the probability of a driveway located on a street near a stop intersection becoming blocked by a vehicle queue. These probabilities consider variables such as corner clearance, intersection service rate, and volumes in the lane adjacent to the driveway. As indicated by Table <u>24</u>, as traffic flow in the lane adjacent to the driveway approaches the capacity of the intersection, the probability that the intersection will be blocked increases rapidly for very limited corner clearances. Therefore, corner clearances of at least one vehicle (roadway space of 25 feet plus 10 feet between stop bar and curb extension on the cross street) would be desirable for all classes of roadway. The most desirable, as indicated by Table <u>24</u>, would be a roadway space of about three vehicles or about 85 feet which would avoid blocking for most traffic use levels.

TABLE 24

Probability of Driveway Blocking

| VOLUMB IN LANE ADJACENT TO DRIVEWAY (VPH) | INTERSECTION SERVICE RATE (VPH) | PROBABILITY THAT DRIVEWAY IS BLOCKED (%), BY CORNER CLEARANCE (NO. OF VEHICLES) | | | | | | |
|---|---------------------------------------|--|-----|-----|-----|----|----|----|
| | | 0 | ĩ | 2 | 3 | 4 | 5 | 6 |
| 50 | 100 | 50 | 25 | 13 | 6 | 3 | 2 | I |
| | 200 | 25 | 6 | 2 | | | | |
| | 300 | 17 | 3 | 1 | | | | |
| | 400 | 13 | 2 | | | | | |
| | 500 | 10 | - 1 | | | | | |
| 100 | 200 | 50 | 25 | 13 | 6 | 3 | 2 | 1 |
| | 300 | 33 | 11 | - 4 | · 1 | | | |
| | 400 | 25 | 6 | 2 | | | | |
| | 500 | 20 | 4 | 1 | | | | |
| | 600 | 17 | 3 | 1 | | | | |
| 150 | 200 | 75 | 56 | 42 | 32 | 24 | 18 | 13 |
| | 300 | 50 | 25 | 13 | 6 | 3 | 2 | 1 |
| | 400 | 38 | ID | 5 | 2 | 1 | | |
| | 500 | 30 | 9 | 3 | 1 | | | |
| | 600 | 25 | 6 | 2 | | | | : |

(near corner on stop street) (3)

February 23, 2001

Mike/Jim:

3

Carmen recently showed me a file final plat of Belt Line-Surveyor Village Addition, dated January, 1998. As shown on the attachment, this plat included acquisition of rightof-way for the future construction of a right turn lane on Surveyor Blvd., at Belt Line Road. Also, attached is a copy of the CIP listing for proposed right-turn lane improvements at this intersection. While we are considering the re-allocation of funds for certain projects, would it be a good time to initiate a contract for design and subsequent construction of this right-turn lane? The original project estimate was \$50,000. A "rough" updated estimate was generated to include surveying, design, and construction of a new right-turn lane(including removal and replacement of pavement & signal and utility line improvements). The cost is closer to \$55,000. Thanks.

C.

Steve C.

Surveyor Blvd. @ Belt Line Road Northbound Right Turn Lane from Surveyor to Belt Line

Capital Project Summary

February 2001

| Project Summary: | This project will provide a right turn lane for northbound traffic on Surveyor Blvd. to turn east onto Belt Line Road. Necessary right-of-way dedication is complete. |
|----------------------|---|
| Funding: | The estimated project cost is \$50,000. This project is not currently funded or programmed. |
| Hurdles: | None. |
| Schedule: | Project complete 9 to 12 months after funding. |
| Design Engineers: | None at this time. |
| Contractor: | None at this time. |
| Project Manager: | Steve Chutchian |
| Project Number: | None at this time. |
| | |

