COMMISSIONER DISTRICT NO.

KENNETH

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MAYFIELD

DATE

05/04/2010

COMMISSIONER DISTRICT

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WILEY

PRICE

COMMISSIONER DISTRICT
MIKE CANTRELL

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COMMISSIONER DISTRICT

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COUNTY JUDGE JAMES FOSTER

<u>DFFICIAL</u>S

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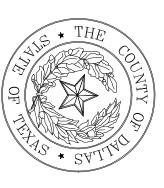
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COVER SHEET AND LOCATION MAP

2002-102

MCIP PROJECT 10305

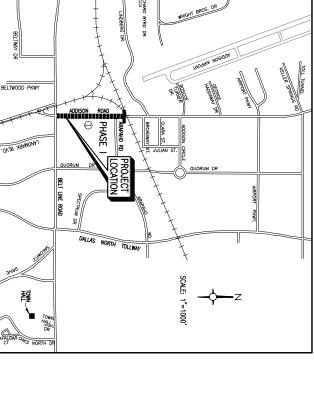
 \mathbb{F} \mathbb{R} \mathbb{R} \mathbb{R} BELT LINE \Box ARAPAHO \mathbb{Z}



EQUATION EXCEPTION RAILROAD

LENGTH OF PROJECT 1500 FT.

CONSTRUCTION TYPE PAVING AND DRAINAGE





DATE: HESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.

APPROVALS

ASSIST. DIRECTOR OF PUBLIC WORKS, ENG. & CONST. -- ALBERTA BLAIR ROBINSON, P.E.

-02

DESIGN SPEED: 40 MPH

① ADDISON RD - BELT LINE RD TO ARAPAHO RD DUNCANVILI DE SOTO PROJECT LOCATION GLENN HEIGHTS ELLIS COUNTY NCASTER PALLAS ROWLETT MESQUITE

TOWN OF ADDISON

DIRECTOR OF PUBLIC WORKS

COUNTY OF DALLAS

RECOMMENDED

PROJECT MANAGER

RECOMMENDED FOR APPROVAL: .

DIRECTOR OF PUBLIC WORKS - DON HOLZWARTH, P.E.

N

SHEET NO.

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CONSTRUCTION PLANS & PROFILES PAVEMENT REMOVAL PLANS COORDINATE LAYOUT PLANS

TYPICAL PAVING SECTIONS

STRIP MAPS

COVER SHEET AND LOCATION MAP SHEET INDEX/GENERAL NOTES

SHEET DESCRIPTION

16 13 **7** - 15 - 21 8410%17

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CROSS SECTIONS AT DRIVEWAYS
DETAIL SHEETS

CONTOUR PLANS PAVING MARKING SIGN PLAN

PLANS

STORM SEWER PLANS STORM SEWER PROFILES

STORM SYSTEM CALCULATIONS

DRAINAGE AREA MAPS

PHASING PLAN

STORM WATER POLLUTION PREVENTION STORM WATER POLLUTION PREVENTION LANDSCAPE PLANS

PLAN PLAN

DETAIL & NOTES

ELECTRICAL PLANS IRRIGATION PLANS

SIGNAL ELECTRICAL DETAILS

UMMARY OF ABBREVIATIONS AND DEFINITIONS

 $\label{lem:hambers} H:\Projects\Addison\2002102\PHASE1\ShEET\Sheets-As-Built\2002102C01.dwg$

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Р.Д.	M.B.G.F.	M.B.	L.S.T.	L.S.	Ę	F	.P.F. OR I.R.F.	HDWL.	GUT.	G.V.	G.M.	G.L.M.	F.L.	F.M.	F.H.	FF.	ESMT.	E.P.	ENC.	ELEC.	뜬	D.I.P.	CULV.	CONN	C.O.	£.	C.M.P.	C.M.A.P.	ا ح	R -R	R.C.	ABBREVIATION	
POINT OF CURVATURE	METAL BEAM GUARD FENCE	MAILBOX	LANDSCAPING TIMBERS	SANITARY SEWAGE LIFT STATION	LIGHT POLE	LANDSCAPING LIGHT	IRON PIN (ROD) FOUND	HEADWALL	GUTTER	GAS VALVE	GAS METER	GAS LINE MARKER	FLOW LINE	SANITARY SEWER FORCE MAIN	FIRE HYDRANT	FACE TO FACE	EASEMENT	EDGE OF PAVEMENT	ENCASEMENT	BURIED ELECTRICAL POWER LINE	ELEVATION	DUCTILE IRON PIPE	CULVERT	CONNECTION	CLEANOUT	CAST IRON	묡	CORRUGATED METAL ARCH PIPE	CENTERLINE	BACK TO BACK	BACK OF CLIRR	DEFINITION	GENERAL TOPOGRAPHY & HORIZONTAL/ VERTICAL CONTROL
W.M. W.V.	₩ V.C.	U.P.M.	U.G.C.M.	T.V.B.	T.V.	TRANS.	T0 F.	T.S.B.	T.M.H.	TEL.	T.C.	T.B.	S.V.B.	ST. SEW.	S.S.M.H.	S.S.	г с	Ľ.	SAN. SEW. OR S.S.	R.O.W.	R.C.C.P.	R.C.P.	R.C.	7 0	P.V.I.	P.V.C.	P.T.	P.R.C.	P.P.	- 0-	P	ABBREVIATION	RIZONTAL/ VERTIC
WATER METER WATER VALVE	VERTICAL CURVE	UNDERGROUND PIPELINE MARKER	UNDERGROUND CABLE MARKER	CABLE TELEVISION BOX	BURIED TELEVISION CABLE	ELECTRICAL TRANSFORMER	TO FACE (OF CURB)	TRAFFIC SIGNAL BOX	TELEPHONE CABLE MANHOLE	BURIED TELEPHONE CABLE	TOP OF CURB	TELEPHONE CABLE BOX	LAWN SPRINKLER VALVE BOX	STORM SEWER	SANITARY SEWER MANHOLE	SANITARY SEWER	SURVEY LINE	LAWN SPRINKLER HEAD	SANITARY SEWER	RIGHT OF WAY	REINF. CONCRETE CYLINDER PIPE	REINFORCED CONCRETE PIPE	REINFORCED CONCRETE	RADIUS	POINT OF VERTICAL INTERSECTION	PLASTIC PIPE	POINT OF TANGENCY	POINT OF REVERSE CURVATURE	POWER POLE	PROPERTY LINE	POINT OF INTERSECTION	DEFINITION	AL CONTROL
	WIL.	SYC.	S.G.	R.O.	R.B.	PIST.	PHOT.	PER.	PEC.	NAN	MUL.	MM.	MSQ.	MAP.	MAG.	L0C.	Lo.	LIG.	JAP.	.	HCK.	H.B.	CYP	C.W.	C.P.	C.M.	C.E.	CDR.	C.B.	BI	R D	ABBREVIATION	П
	WILLOW TREE	SYCAMORE TREE	SWEET GUM TREE	RED OAK TREE	REDBUD TREE	CHINESE PISTACHIO TREE	RED TIPPED PHOTINIA BUSH	PERSIMMON TREE	PECAN TREE	NANDINA BUSH	MULBERRY TREE	MIMOSA TREE	MESQUITE TREE	MAPLE TREE	MAGNOLIA TREE	LOCUST TREE	LIVE OAK TREE	LIGUSTRUM BUSH	JAPONICA TREE	HAWTHORN TREE	HICKORY TREE	HACKBERRY TREE	CYPRESS TREE	COTTONWOOD TREE	CHINESE PISTACHIO TREE	CRAPE MYRTLE TREE	CEDAR ELM TREE	CEDAR TREE	CHINABERRY TREE	BLACK THORN TRFF	BOIS D'ARC TREE	DEFINITION	TREES & BUSHES

GENERAL NOTES

1. ALL WORK AND MATERIALS SHALL BE IN ACCORDANCE WITH THE SPECIFICA SPECIFICATIONS. TIONS, AS OUTLINED IN N.C.T.C.O.G. AND TOWN OF ADDISON

- 2. PROPOSED MAIN LANES ALL PROPOSED 10-INCH CONCRETE REINFORCEMENT SHALL BE #4 BARS DEFORMED ON 18" CENTER EACH MAX: REINFORCED CONCRETE PAVEMENT SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 4000 P.S.I. MINIMUM 6.5 SACK MIX, MAXIMUM W/C 5.5 N.C.T.C.O.G. AGGREGATE 2—3 [ITEM 2.1.1.(C)(4)] COURSE. MAXIMUM SLUMP SLIP FORM 2 INCHES, SLUMP HAND POURED 4 INCHES.
- 3. ALL CONNECTIONS OF PROPOSED CONCRETE PAVEMENT TO EXSTING PAVEMENT SHALL INCLUDE A LONGITUDINAL BUTT JOINT WITH #5 OR #6 BAR DOWELS AS THE LOAD TRANSFER DEVICE SEE DETAIL THIS SET OF PLANS.
- 4. REASONABLE EFFORT HAS BEEN MADE TO SHOW THE LOCATION OF ALL KNOWN UNDERGROUND UTILITIES AND SERVICE LINES, HOWEVER, THE OWNER ASSUMES NO RESPONSIBILITY FOR FAILURE TO SHOW ANY OR ALL EXISTING SUBSURFACE UTILITIES OR UTILITY LINES, OR TO SHOW THEM IN THER EXACT LOCATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING UTILITIES, SERVICE LINES OR THE LIKE, WHICH ARE CROSSED OR EXPOSED BY THE CONSTRUCTION OPERATION.
- 5. THE CONTRACTOR SHALL FIELD VERFY DEPTH AND HORIZONTAL ALGNMENT OF ALL EXSTING UTILITIES (GAS LINES, MATER LINES, COMMUNICATION LINES AND SERVICE LINES) PRIOR TO CONSTRUCTION. CONFLICTS WITH PROPOSED IMPROVEMENTS SHALL BE PROVIDED IN WRITING TO THE TOWN OF ADDISON.
- 6. THE CONTRACTOR SHALL PROTECT ALL UNDERGROUND SPRINKLER SYSTEMS. ALL DAMAGE CAUSED BY CONSTRUCTION SHALL BE REPAIRED WITHIN 48 HOURS BY AN IRRIGATOR LICENSED IN THE STATE OF TEXAS. ALL COSTS SHALL BE BORNE BY THE CONTRACTOR. SEE LANDSCAPE PLANS FOR PROPOSED IRRIGATION.
- 7. UTILITY RELOCATION'S AND ADJUSTMENTS WATER METER AND WATER METER BOXES TO BE RELOCATED OR ADJUSTED SHALL BE REPUACED WITH NEW METER BOXES AND SET AT THE FINNSHED GRADE ELENATION AND WATER SERVICES RECONNECTED. ALL WATER METERS AND METER BOXES IN PROPOSED BACK OF CURB AND PROPOSED SIDEMALK. EXISTING METER BOXES REMOVED SHALL BE THE PROPERTY OF THE TOWN. EXISTING WATER VALVE BOXES WITH COVERS SHALL BE REMOVED AND RESET AT THE FINISHED GRADE ELENATION. ALL EXISTING STORM SEVERE OR SANTHARY SEVER MANHOLE FRAME AND COVER TO BE ADJUSTED TO MATCH FINISHED GRADE SHALL USE PRE CAST CONCRETE GRADE RINGS AND NON SHRINK GROUT. ANY OF THESE TEAM THAT FACE DAMAGED BY CONSTRUCTION ACTION SHALL BE REPUACED WITH TEAMS OF EQUAL OR SUPERIOR GRADE. THE SOLE EXPENSE OF THE CONTRACTOR. ALL WATER SHALL BE REPUACED WITH TEAMS OF EQUAL OR SUPERIOR GRADE. ALL WATER SERVICE LINKES SHALL BE REPUACED WITH THE SOLE EXPENSE OF THE CONTRACTOR. ALL WATER SERVICE LINKES SHALL BE REPUACED WITH NEW 4 INCH SDR 35 PVC PHEE OR MATCH PIPE SIZE IF LARGER THAN 4 INCH ESD.
- 8. THERE SHALL BE AT LEAST 6-INCHES CLEAR DISTANCE FROM THE TOP OF THE ADJUSTED METER BOXES AND THE THE MATER METER. CONTRACTOR SHALL NOTIFY THE TOWN AND ALL OWNERS WITH METERS TO BE ADJUSTED 48 HOURS IN ADVANCE OF ANY PLANNED CONSTRUCTION, SERVICE SHALL NOT BE DISRUPTED FOR MORE THAN 4 HOURS WITHOUT THE PERMISSION FROM THE CITY ENGINEER. CONTRACTOR SHALL TAKE MEASURES NECESSARY TO RESTORE WATER SERVICE WITHIN THE 4-HOURS.
- 9. THE CONTRACTOR SHALL GIVE THE TOWN, BUSINESSES AND RESIDENTS AT CONSTRUCTION AND AT LEAST 48 HOURS FOR ANY ANTICIPATED WATER OR S LEAST 72 HOURS PRIOR NOTICE TO BEGINNING OF EWER SERVICE DISRUPTION.
- CONCRETE MIX DESIGN SHALL BE SUBMITTED FOR REVIEW A MINIMUM OF 14 DAYS PRIOR TO THE POUR.
- '1. NO SEPARATE PAY ITEM FOR SUB GRADE PREPARATION UNDER DRIVE OR APPROPRIATE ITEMS OF THE BID SCHEDULE. PARKING AREAS AND ALL COST SHALL BE INCLUDED IN THE
- 12. ALL PAVEMENT REMOVAL AND REPLACEMENT SHALL BE FULL DEPTH SAW CUT ALONG NEAT LINES AS SHOWN IN THE PLANS. PROPOSED CONCRETE PAVEMENT SHALL BE CONSTRUCTED WITH LONGITUDINAL CONSTRUCTION JOINTS AT ALL CONNECTIONS TO EXISTING CONCRETE PAVEMENT. DAMAGED OR SPALDED EDGES WILL BE RECUT FULL LENGTH.
- 13. THE CONTRACTOR SHALL REMOVE AND PROPERLY DISPOSE OF ALL CONSTRUCTION OF DRIVES AND PARKING AREAS. THERE SHALL BE NO IN THE APPROPRIATE ITEMS IN THE BID SCHEDULE. EXISTING CONCRETE AND H.M.A.C. PAVEMENT AS REQUIRED FOR SEPARATE PAYMENT FOR THIS WORK, ALL COSTS SHALL BE INCLUDED
- 14. THE CONTRACTOR SHALL RESTORE ALL AREAS DISTURBED BY THE CONSTRUCTION TO THE ORIGINAL CONDITION OR BETTER. RESTORED AREAS INCLUDE BUT NOT LIMITED TO TRENCH BACK FILL, SIDE SLOPES, FENCES, IRRIGATION SYSTEMS, DRIVEWAYS, PRIVATE YARDS, SIGNS AND ROADWAYS.
- 15. THE CONTRACTIOR SHALL PROTECT ALL TREES, STRUCTURES, SIONS, MAILBOXES AND OTHER ITEMS THAT ARE NEAR AND/OR ADJACENT TO A TO CONSTRUCTION. PRIMATE SIGNS AND MAILBOXES TO BE REPUBLIED SHALL BE REPLACED OR RELIGIONAL AND ACCORDINATION OF THE OWNER AND THE TOWN AND BE WITHIN THE CONDEL DESTAIL BE TOWN ORDINANCES. ALL OTHER SIGNS REMOVED DURING CONSTRUCTION SHALL BE TEMPORARY MOUNTED AND PLACED AT PROPER LOCATIONS TO DIRECT TRAFFIC FLOW IF APPLIES.
- 16. THE CONTRACTOR SHALL MAINTAIN TWO WAY TRAFFIC DURING ROADWAY CONSTRUCTION AND MAINTAIN ACCESS TO ALL BUSINESSES BY USING EXISTING, PROPOSED OR TEMPORARY H.M.A.C. PAVEMENT. THE CONTRACTOR SHALL POST SIGNS AT DRIVEWAYS ALONG THE CONSTRUCTION SITE TO DIRECT TRAFFIC INTO BUSINESSES. THE SIGNS SHALL BE 2 FT. BY 3 FT. WITH BLUE BACKGROUND AND WHITE LETTERS. LETTERS SHALL HAVE THE NAME OF THE BUSINESS AT THE DRIVEWAY TO BE ACCESSED.
- 17. CONTRACTOR SHALL PROVIDE PROPER BARRICADES, SIGNS, FLAG MEN AND OTHER TRAFFIC CONTROL DEVICES FOR THIS AREA.
- 18. CONTRACTOR SHALL STAKE SIDEWALK LAYOUT IN THE RIELD FOR TOWN I AS DIRECTED BY THE TOWN. DETERMINATION WILL BE BASED ON AVOIDING LANDSCAPING. REVIEW. CONTRACTOR SHALL MODIFY ALIGNMENT OF SIDEMALK DISRUPTION OF TREE ROOT STRUCTURE OF EXISTING
- 19. CONTRACTOR SHALL ARRANGE AND PAY FOR SWBT, AT&T, ATMOS GAS GRADE OF ROADWAY. COMPANY TO ADJUST EXISTING MANHOLES TO FINISHED

his record drawing is a compilation of the seeled revening drawing for this project; modified by addendo, the orders and information furnished by the contractor, and information for the record drawings that was all parties of the contract or other pot associated with the seign engineer cannot be verified for accuracy or tereass. This original sealed drawings are an file at the offices of Birkhotf, Hendricks & Corter, L.L.P. THESE DOCUMENTS ARE FI BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.





DESIGNED BY: J.W.B.

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ADDISON, TEXAS

These plans and related specifications were prepared for construction of this specific project only. Reuse of these specific project only. Reuse without documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.

SHEET INDEX/GENERAL NOTES PHASE I

BIRKHOFF, HENDRICKS & CONWAY L. L. CONSULTING ENGINEERS ۳.

PROJECT: 2002-102 SHEET NO.

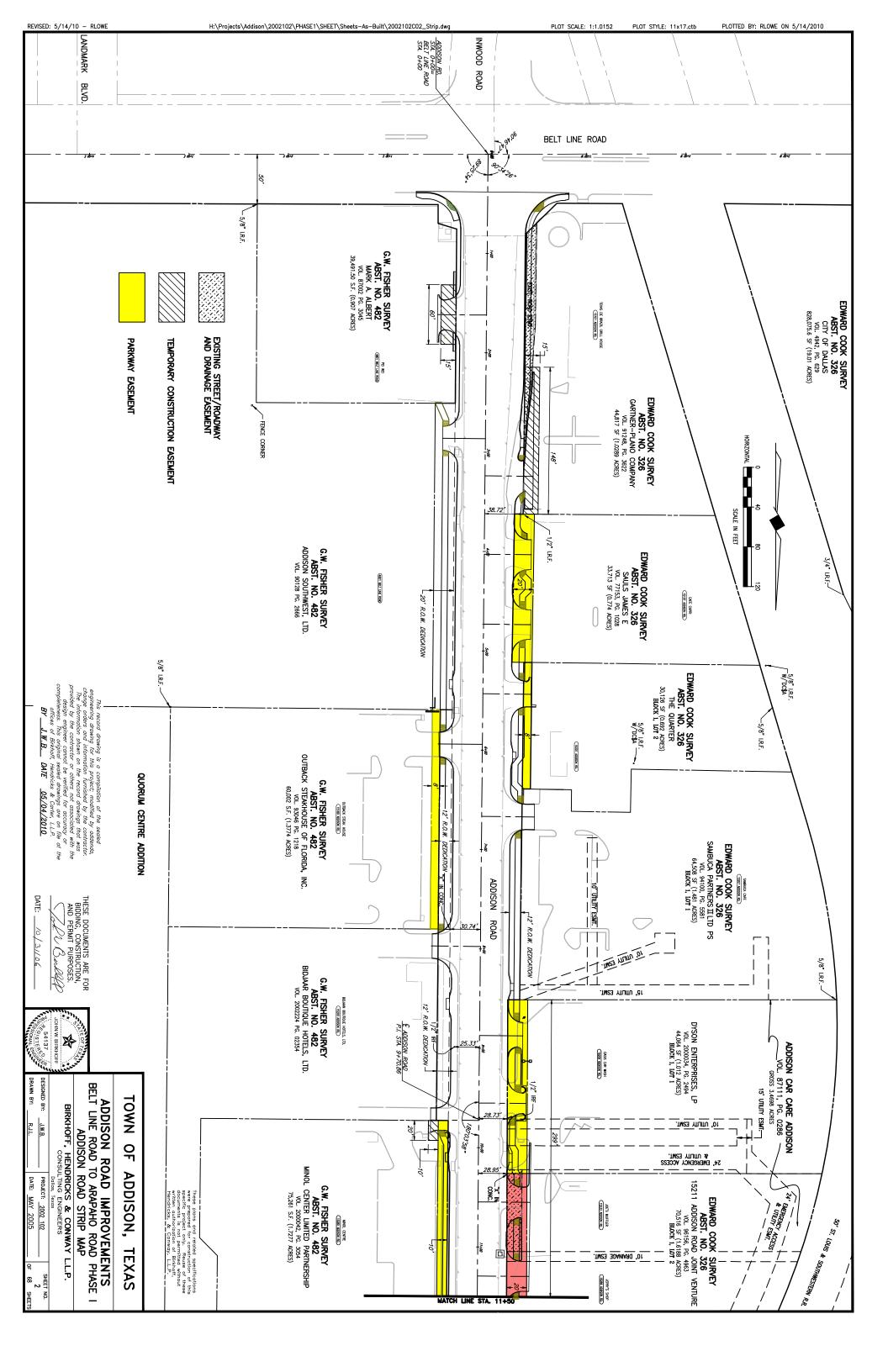
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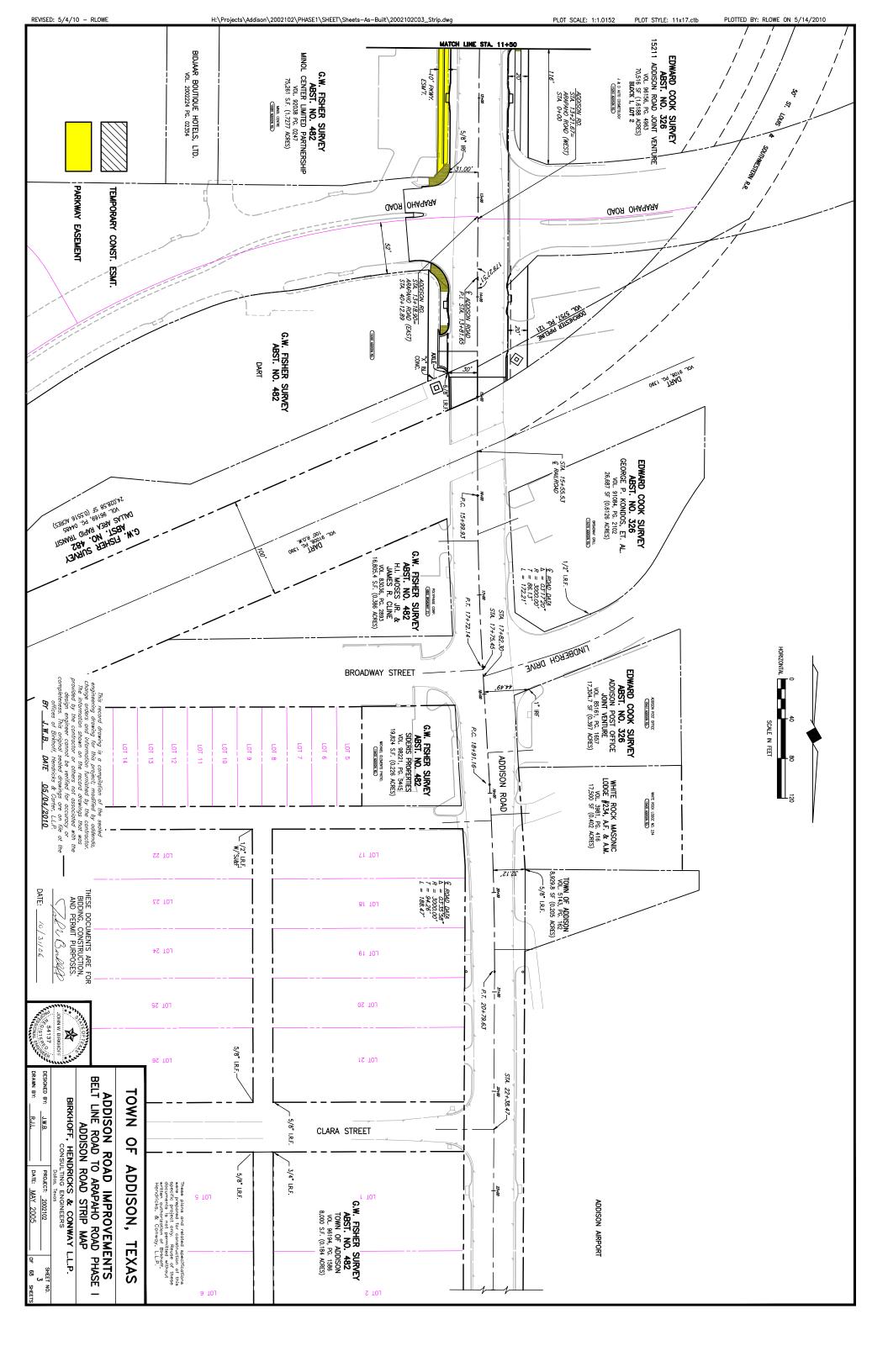
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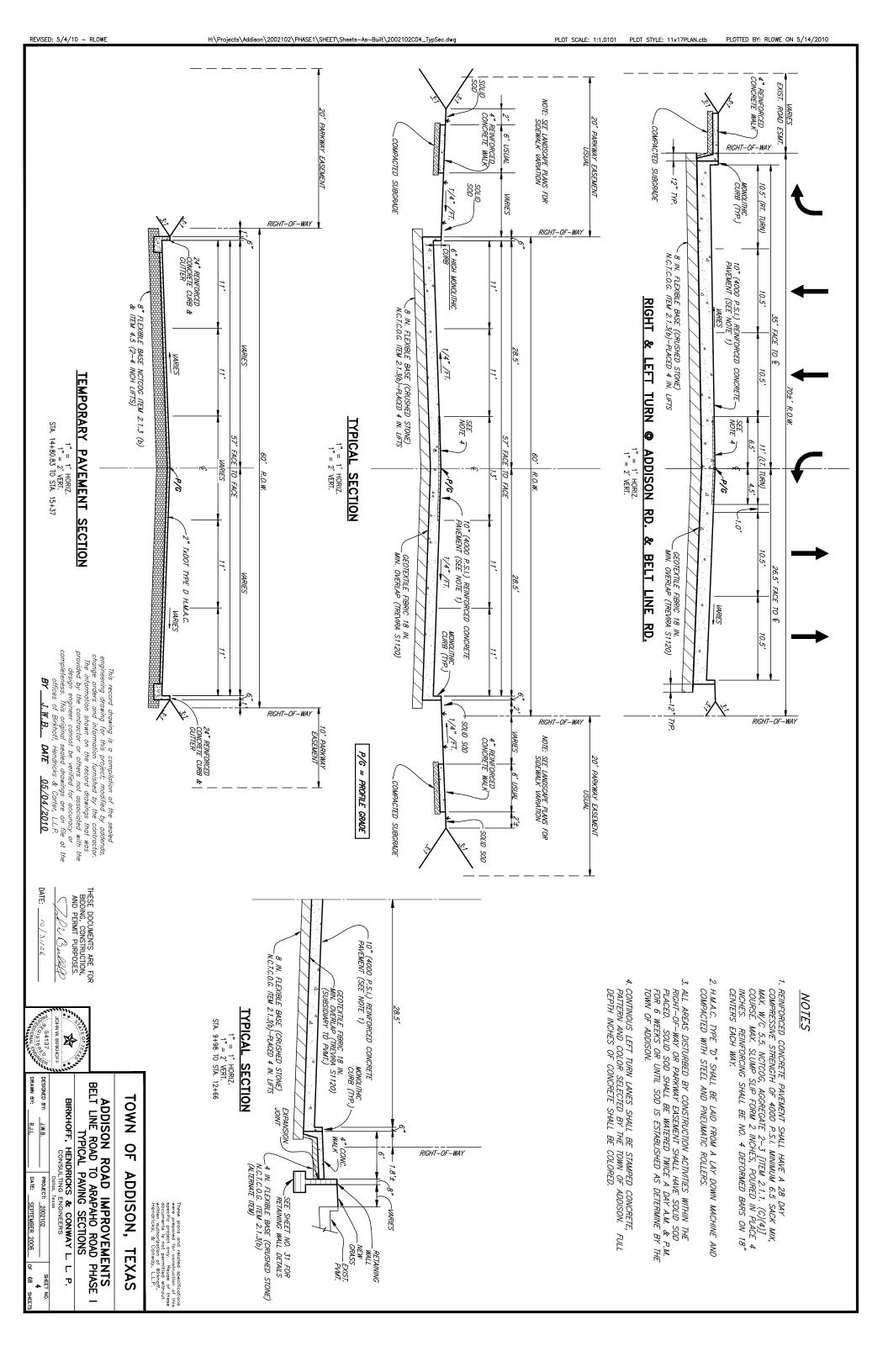
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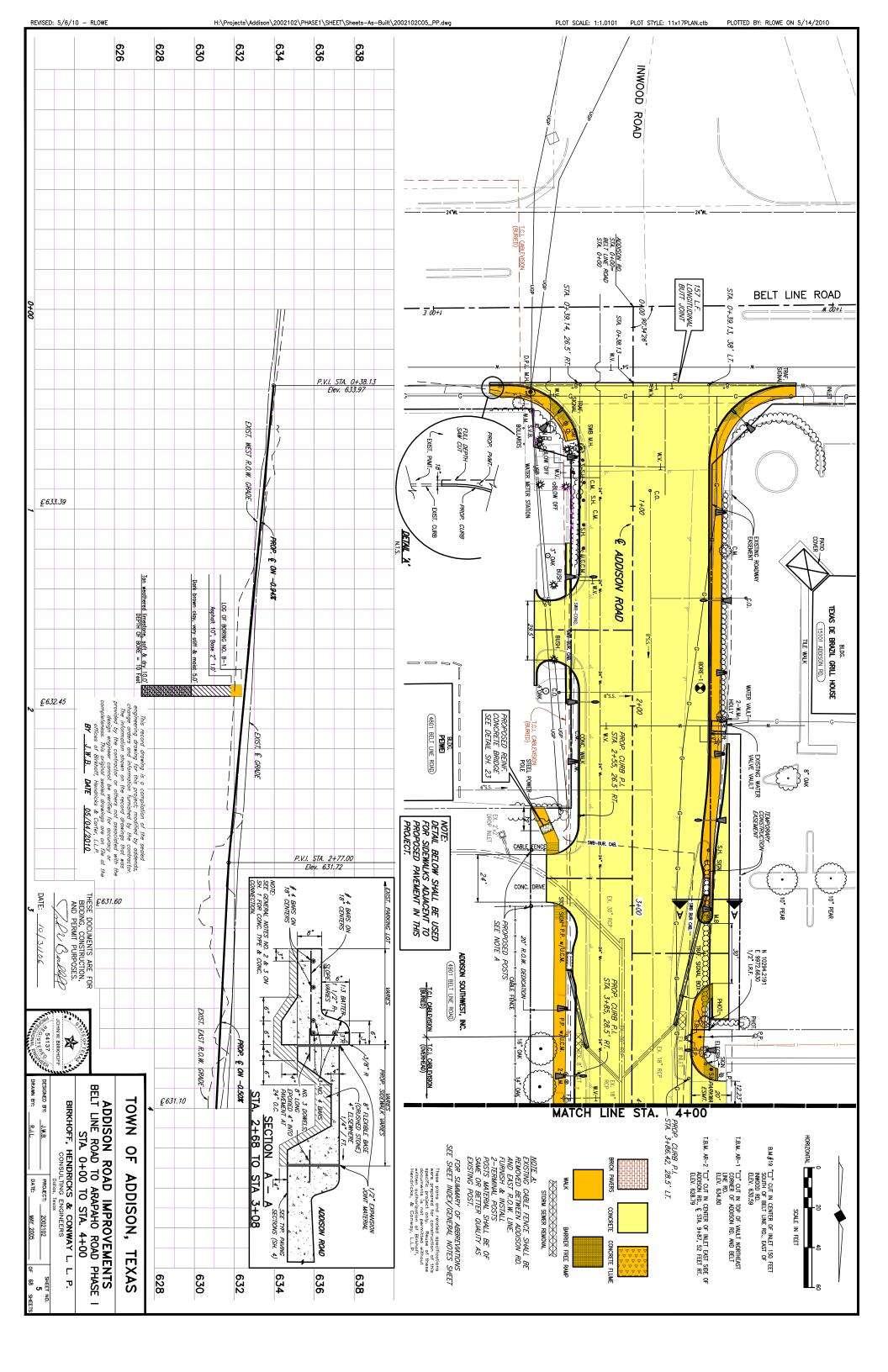
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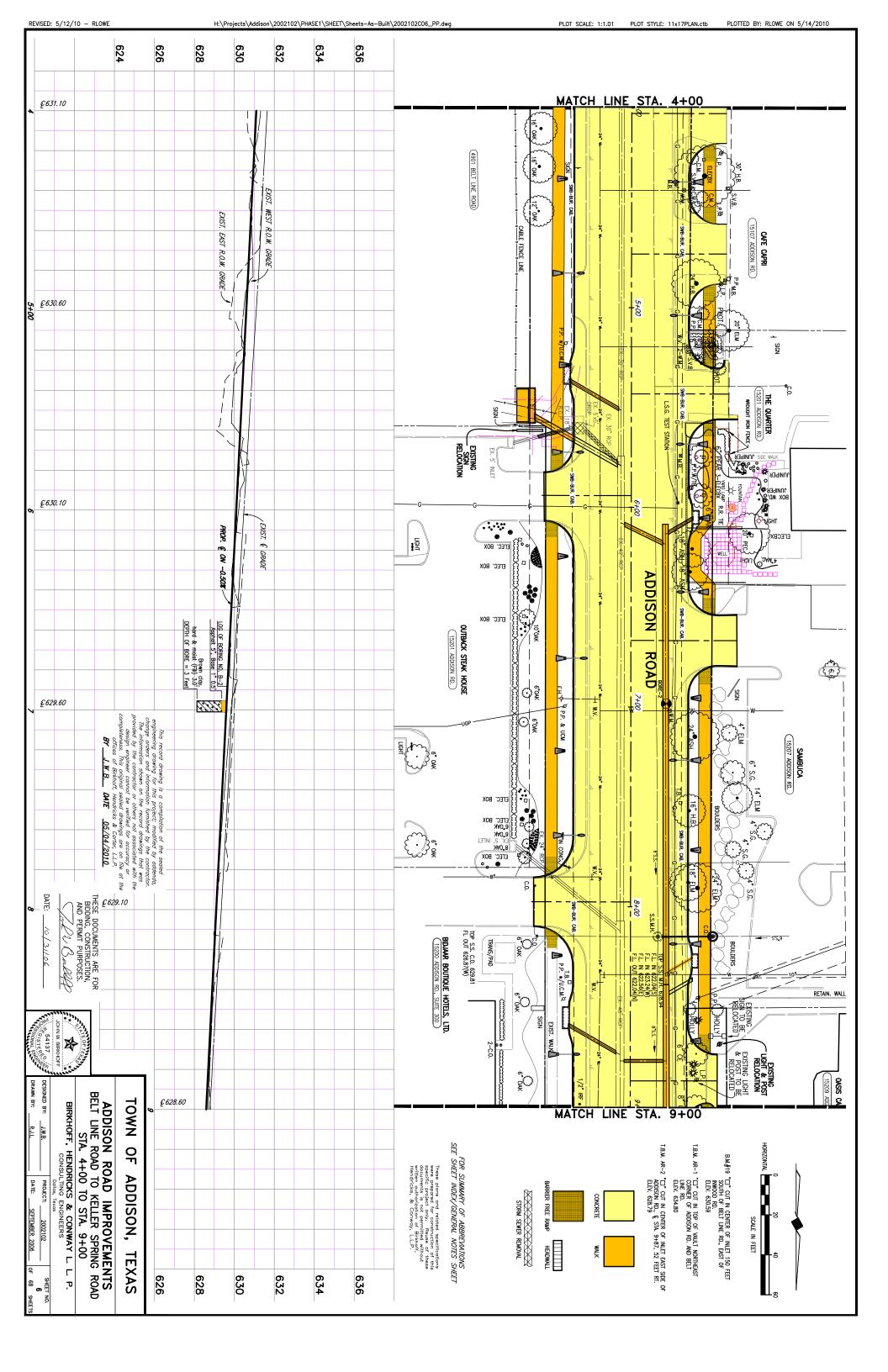
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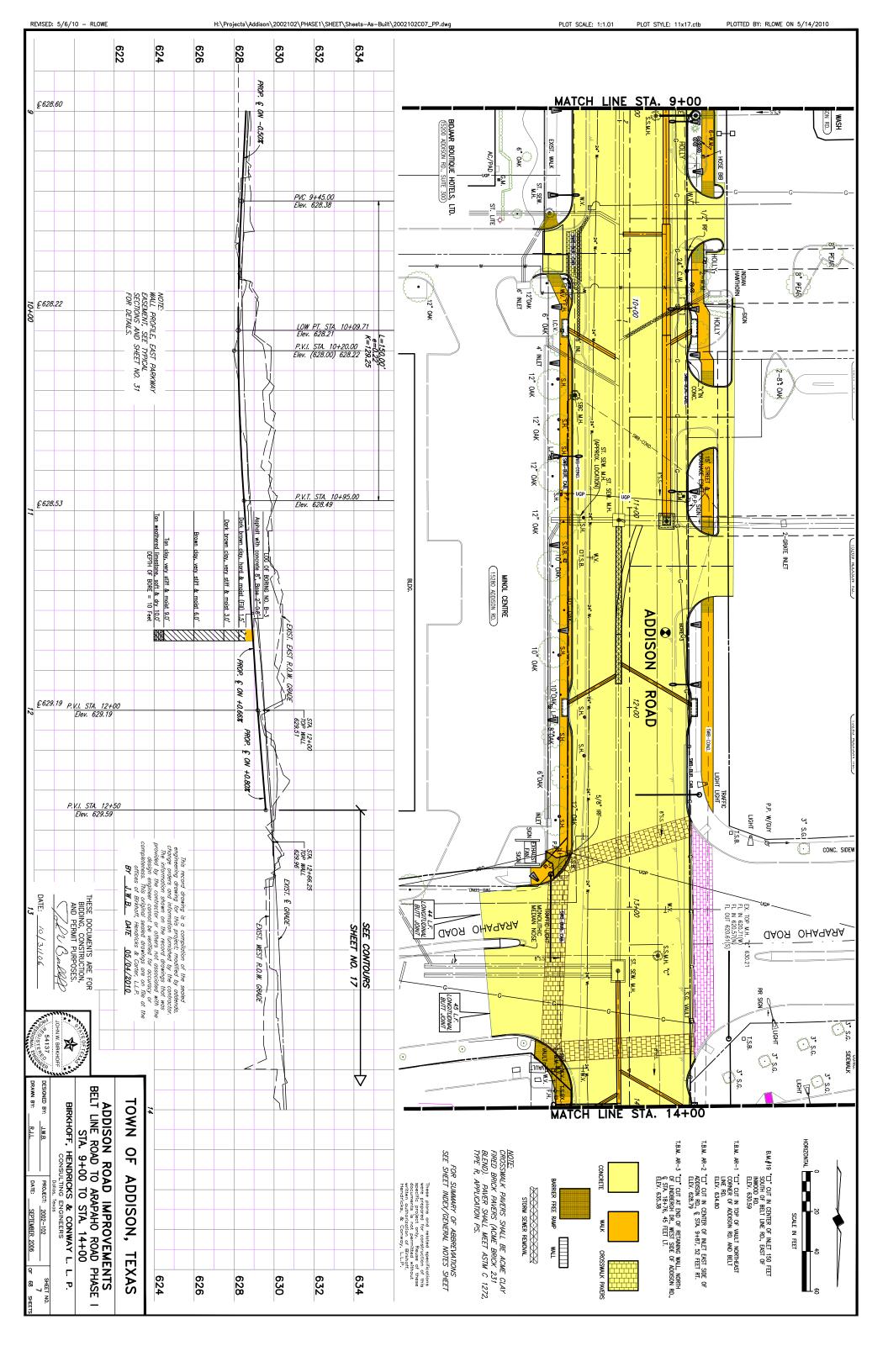


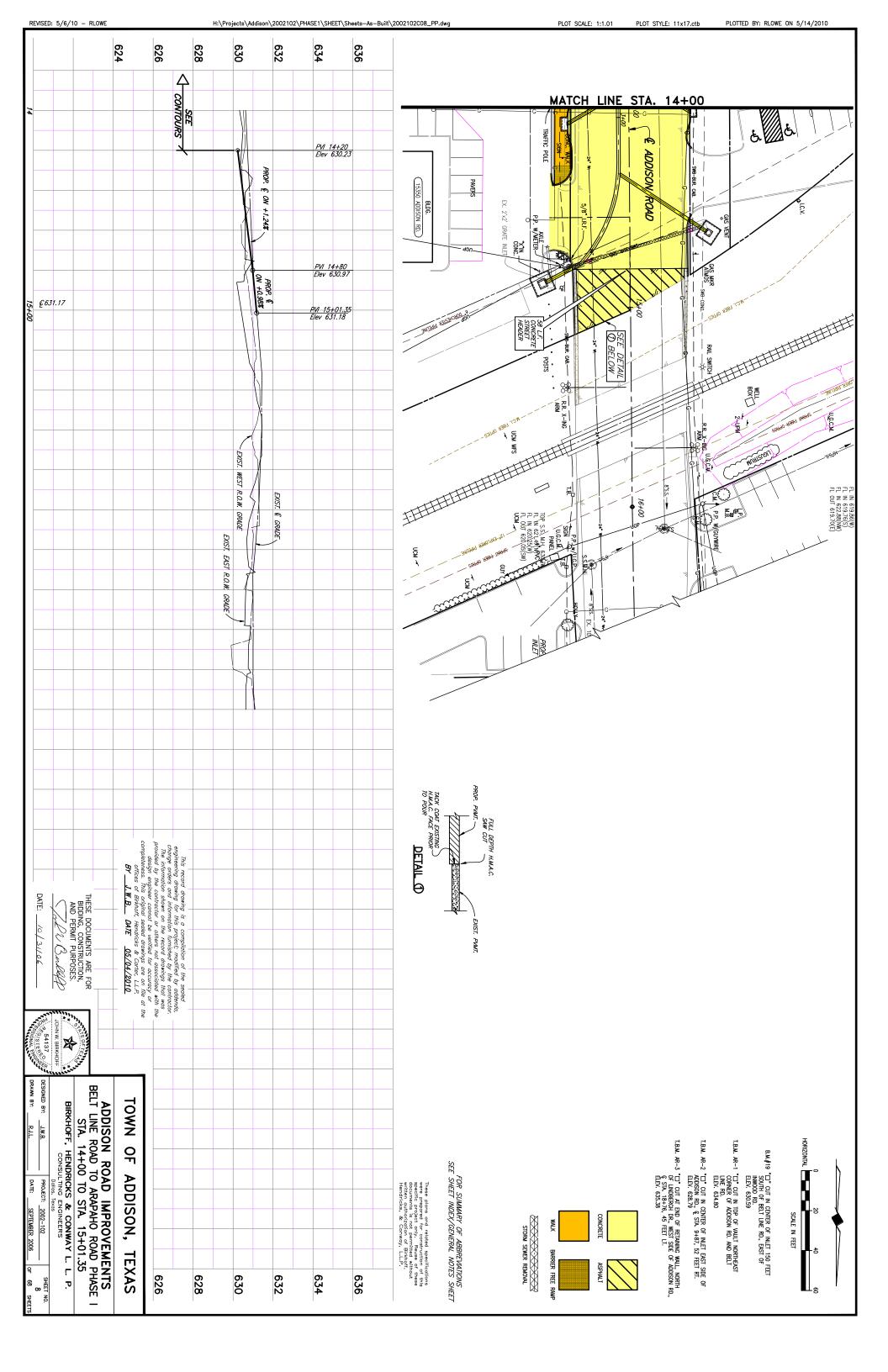


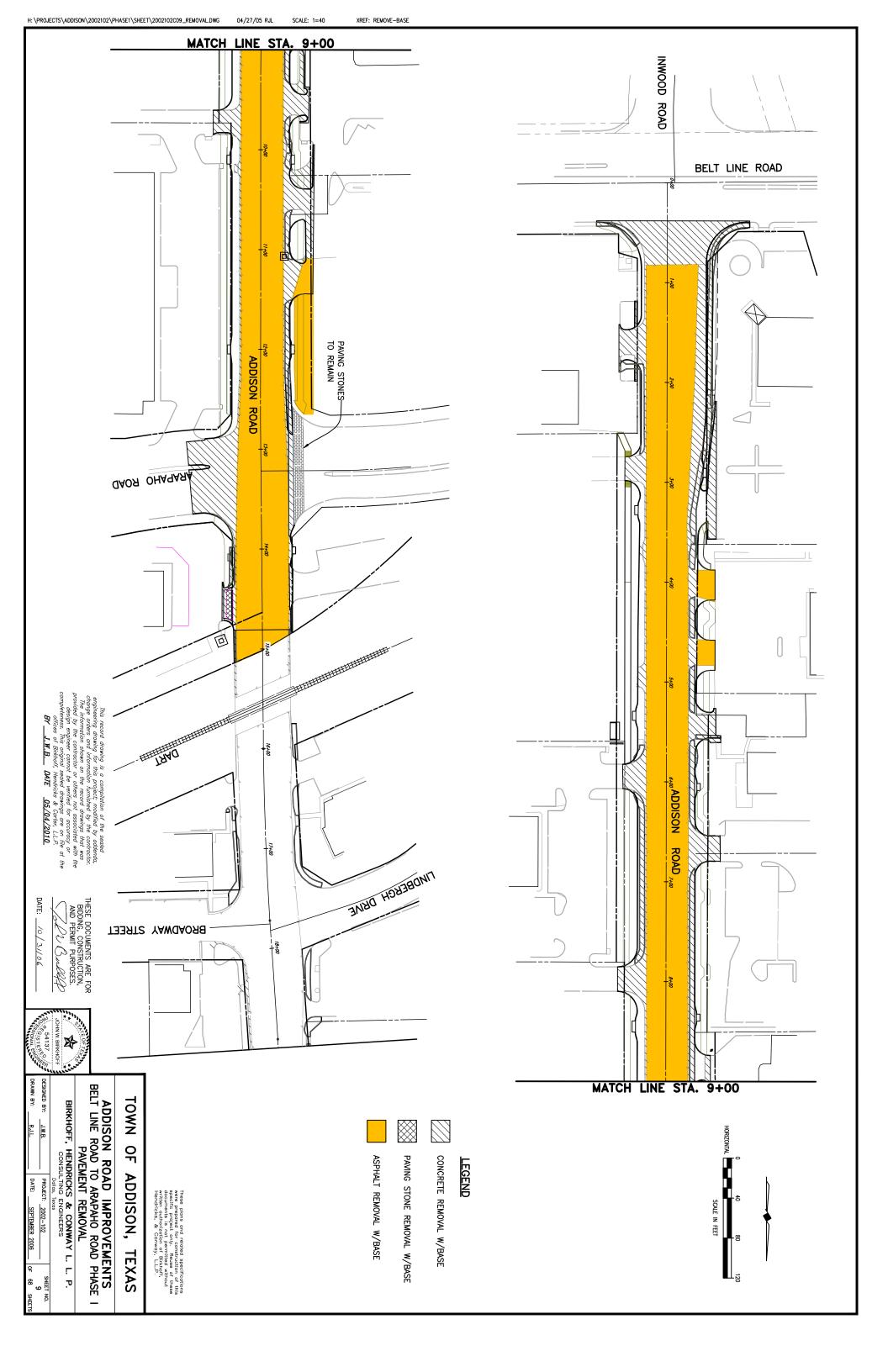


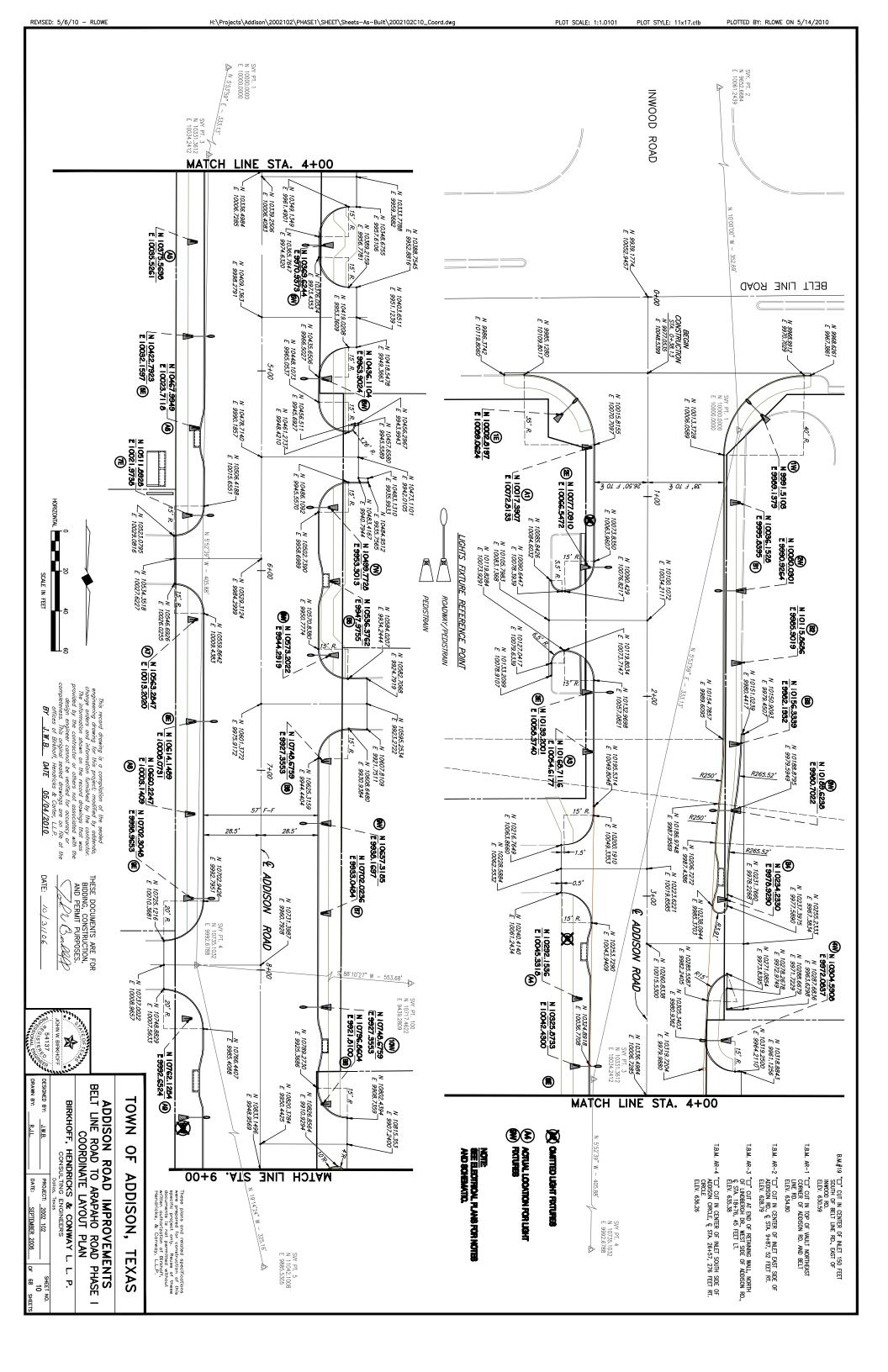


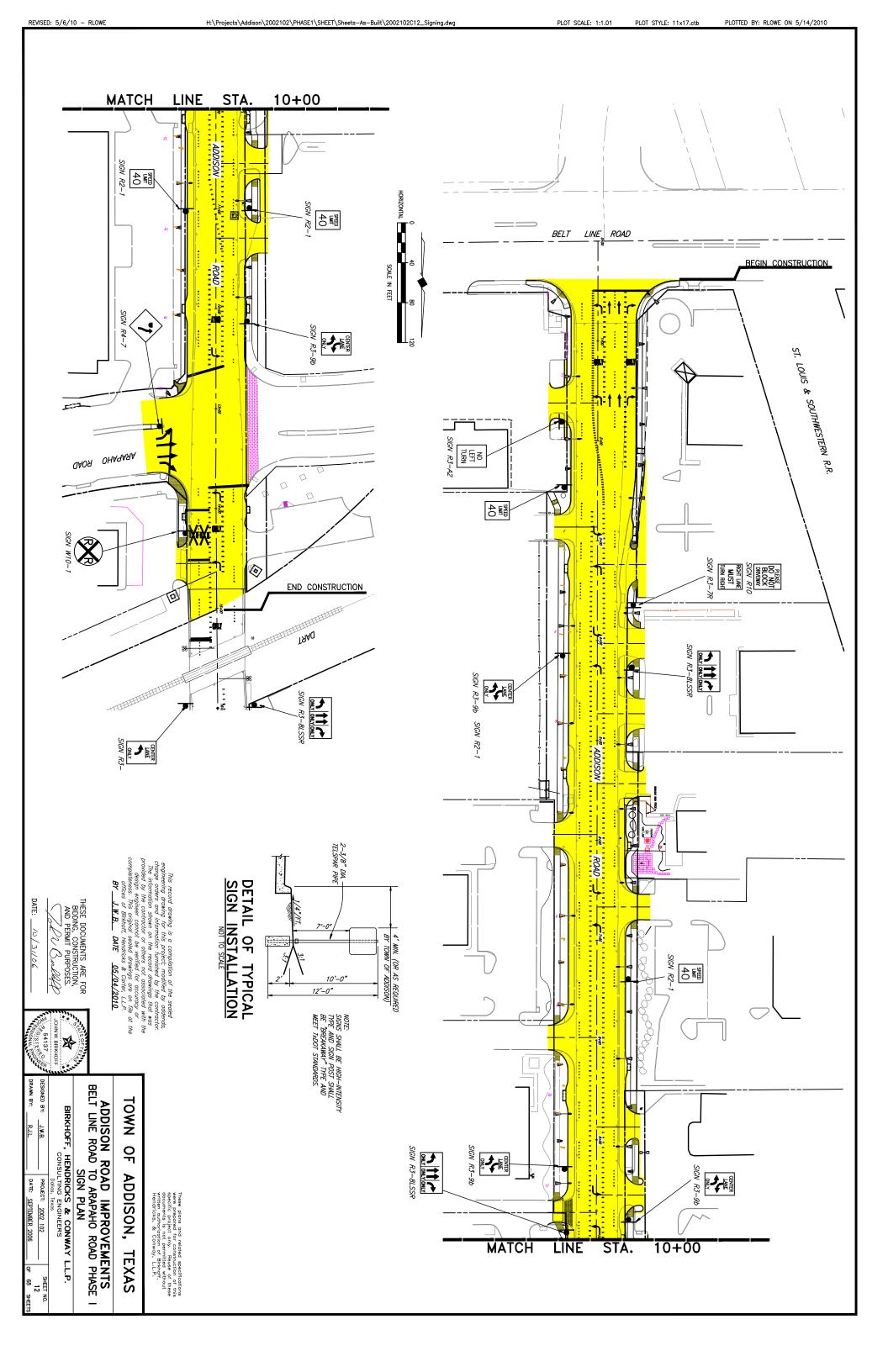


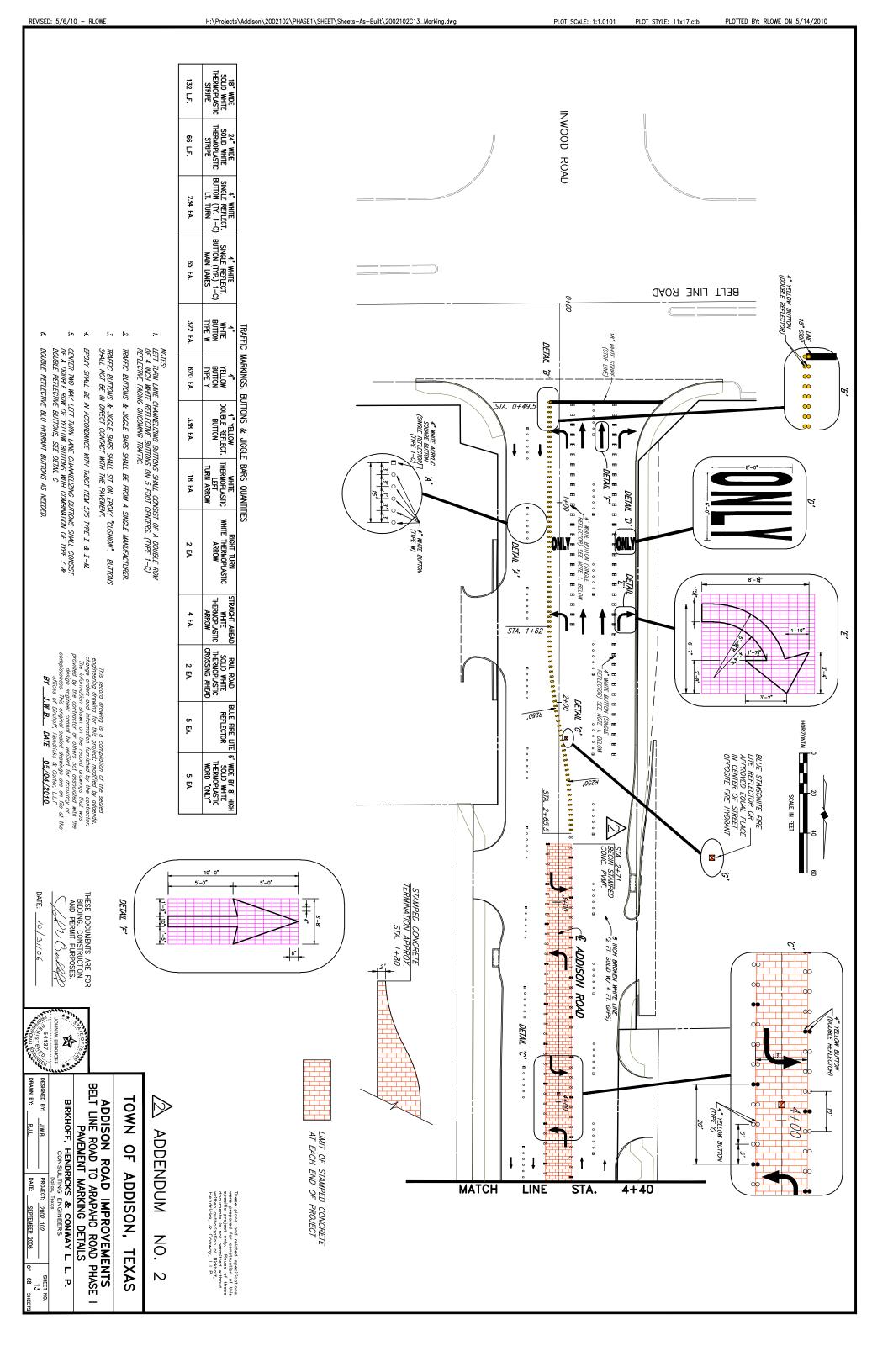


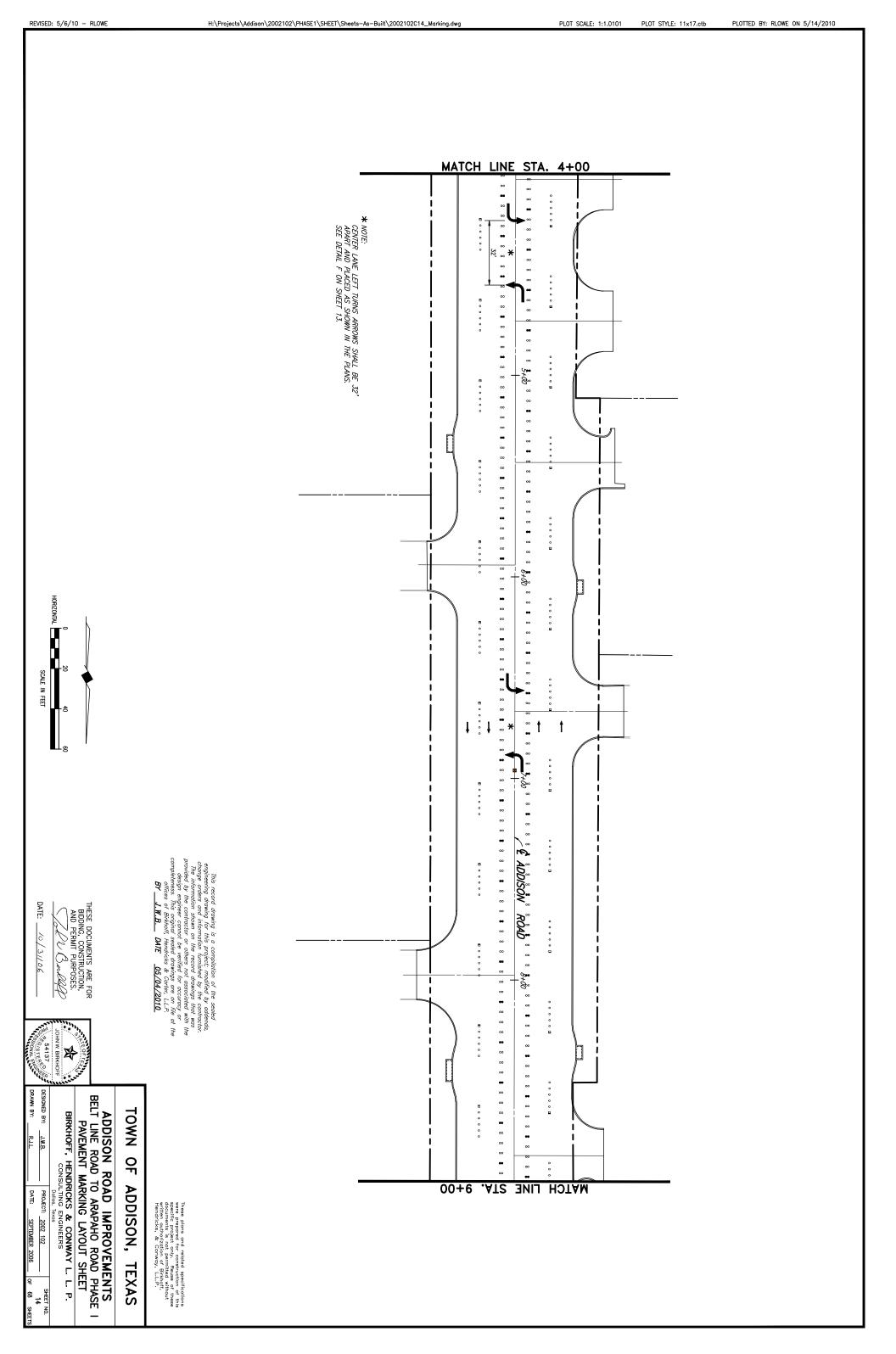


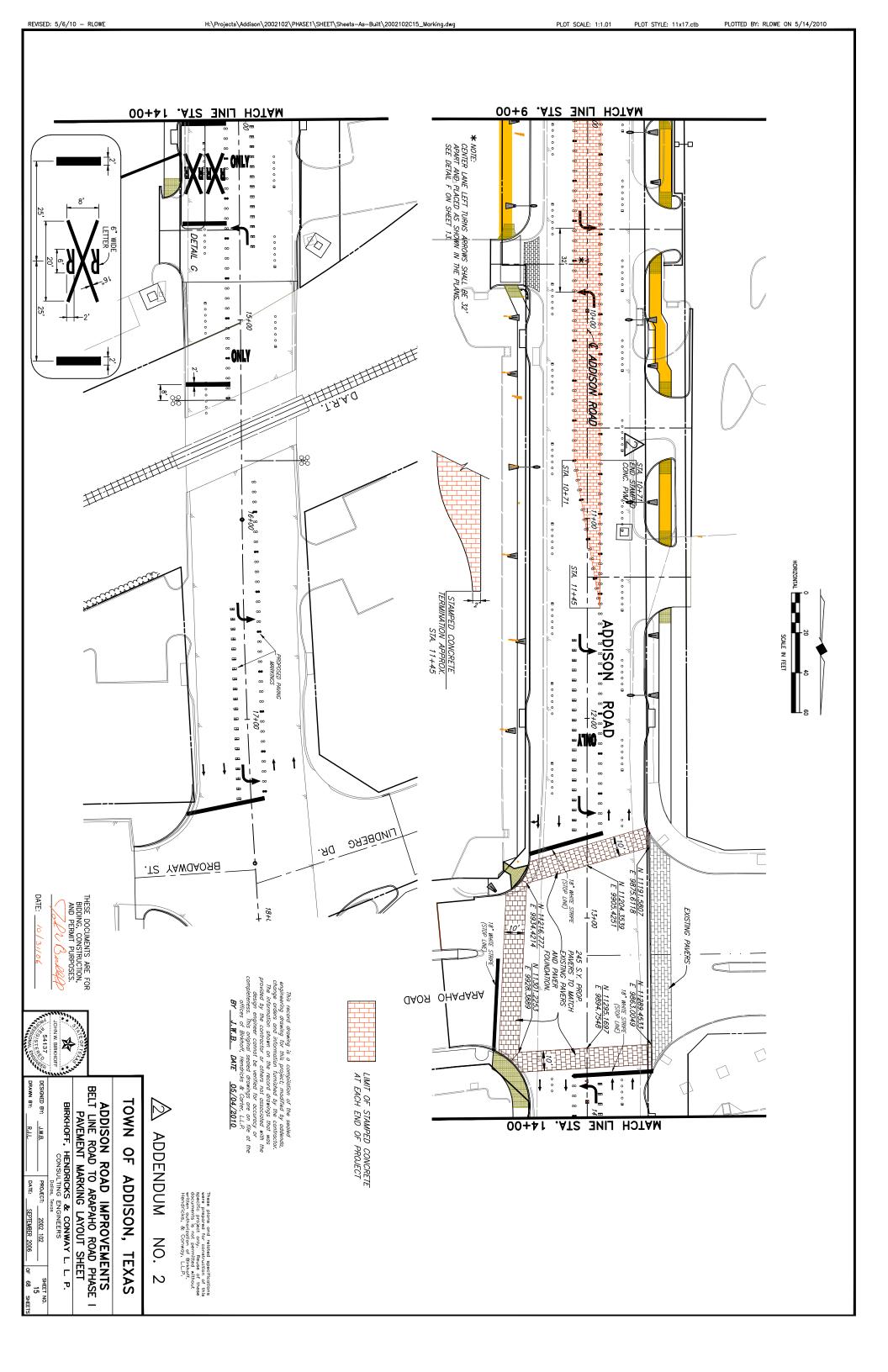


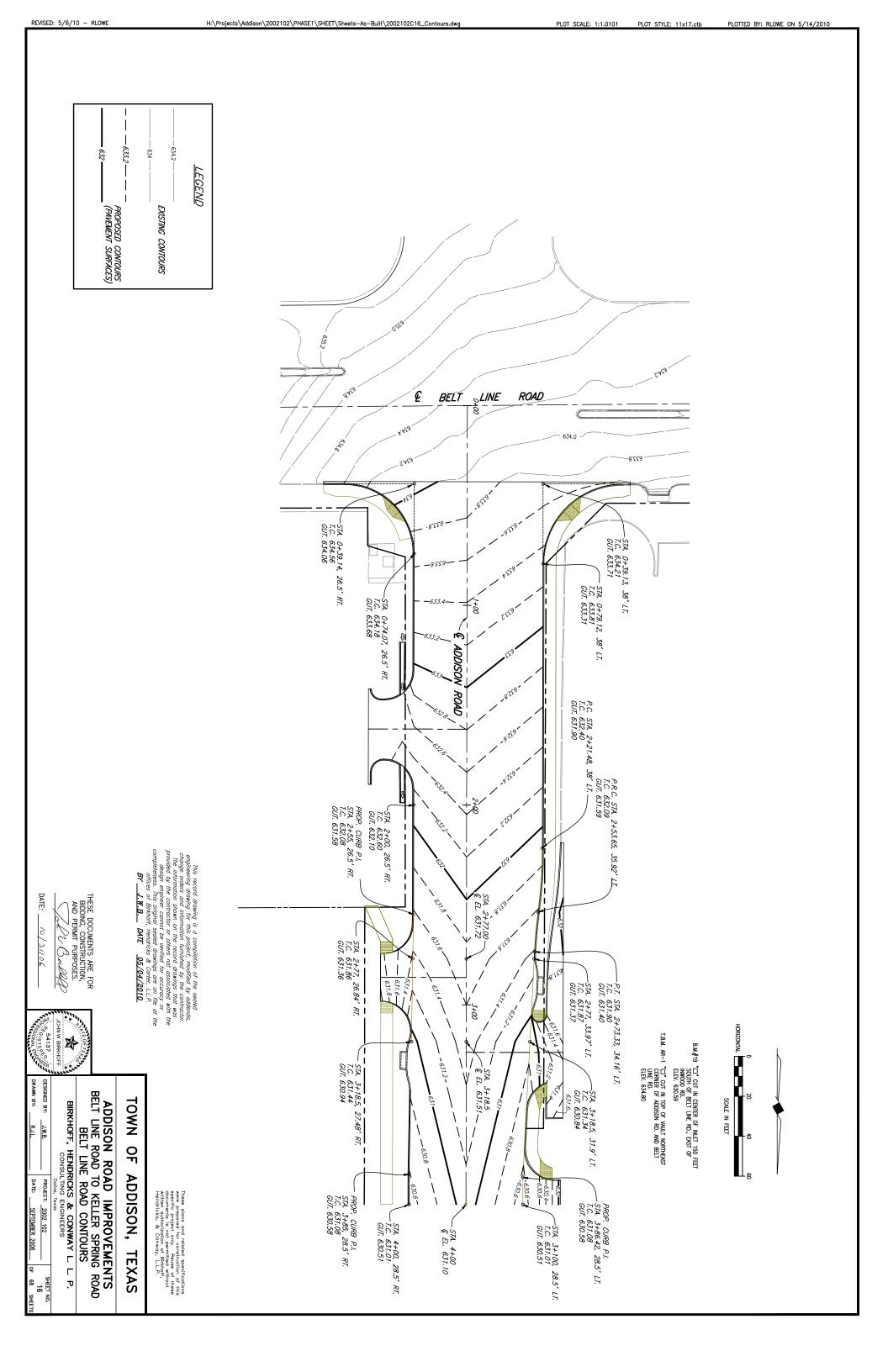


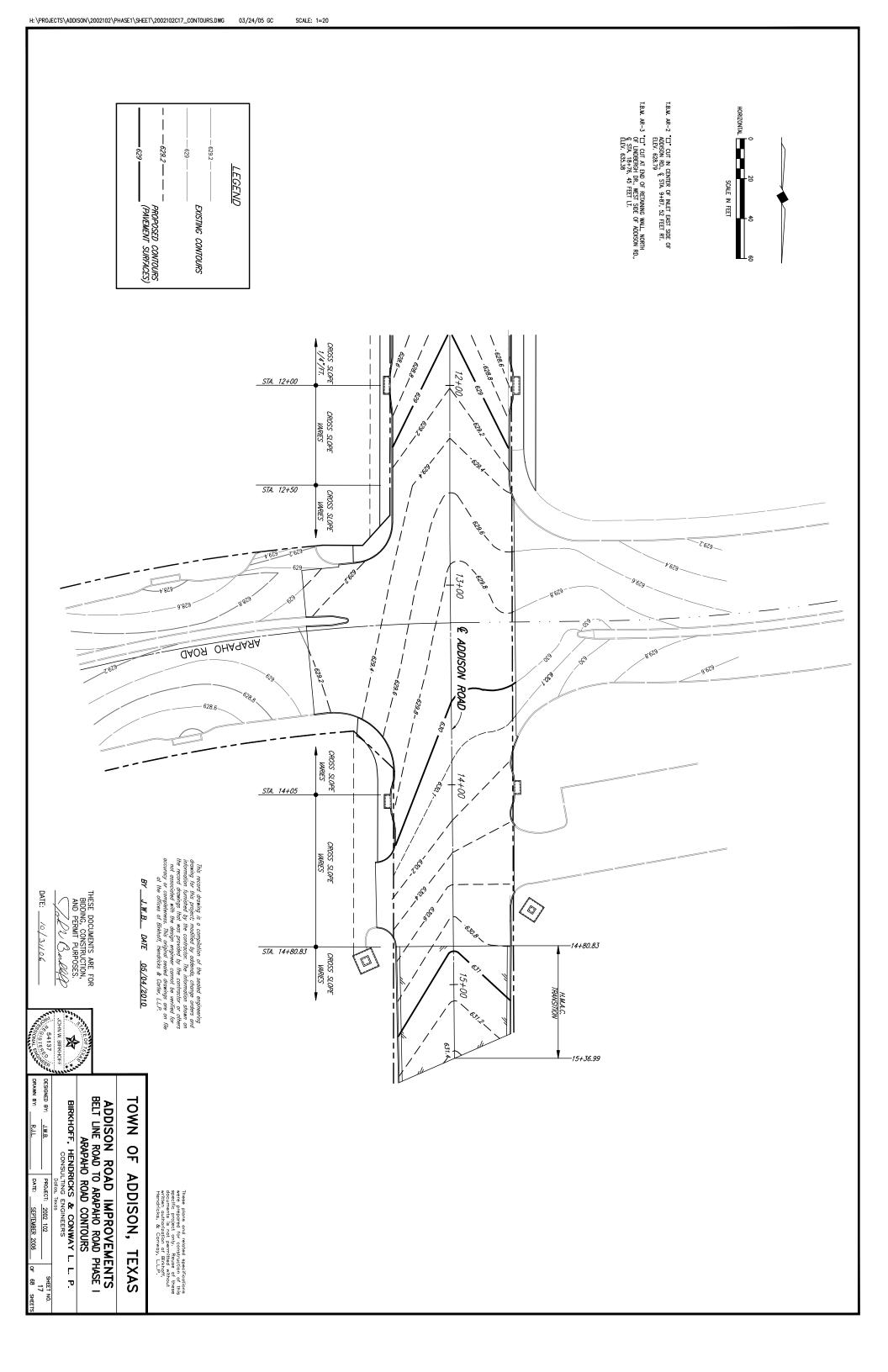


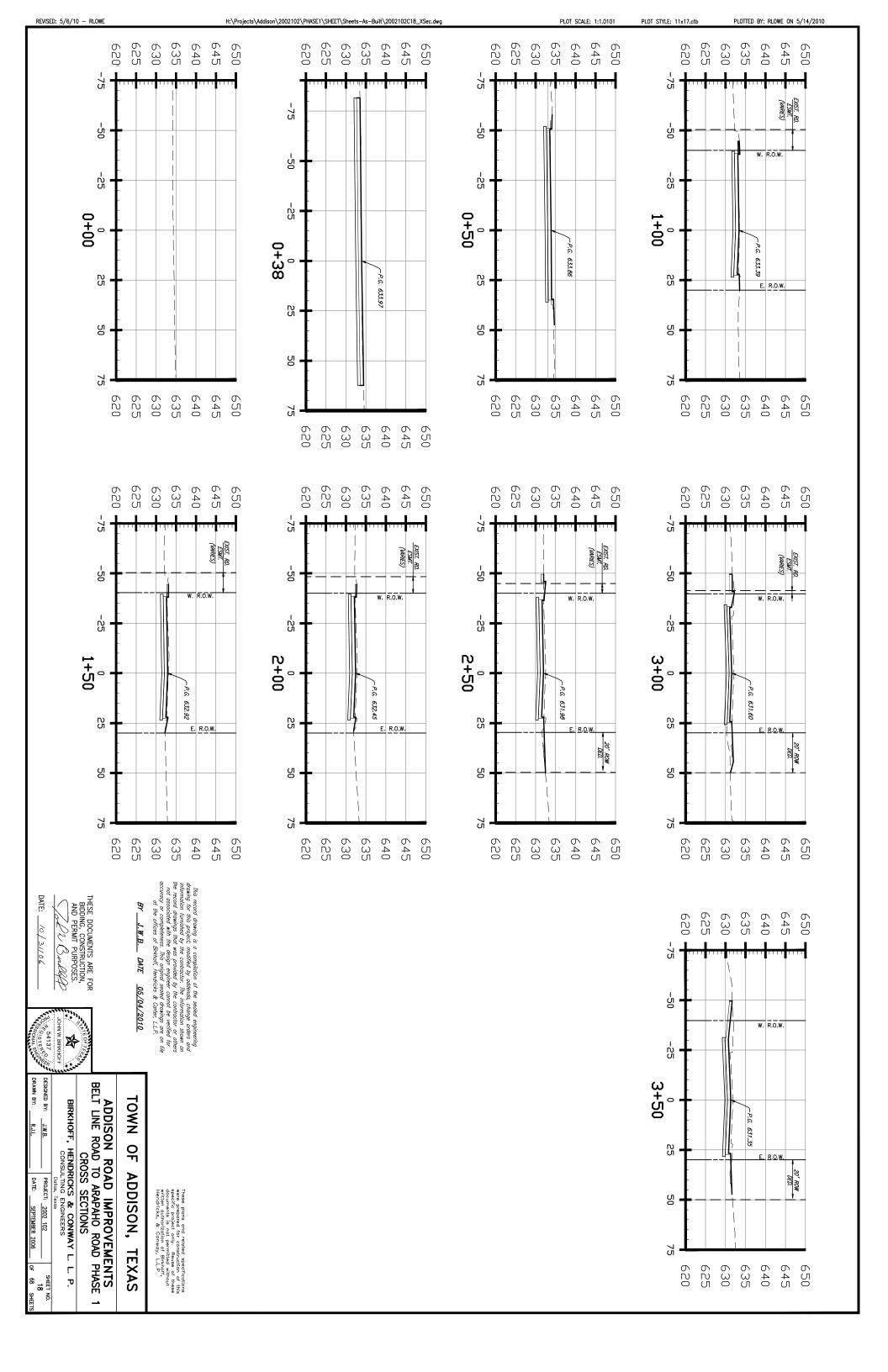


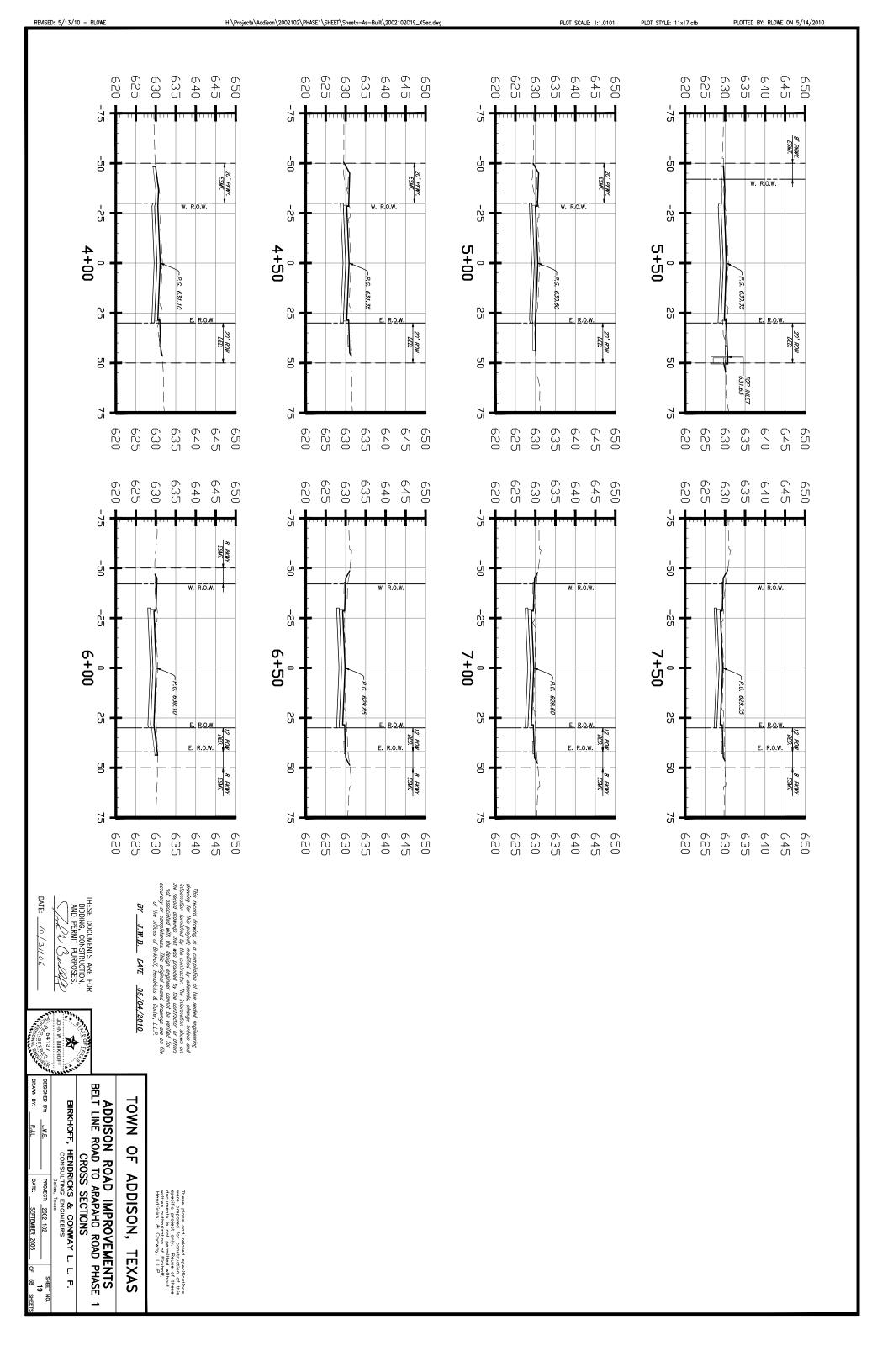


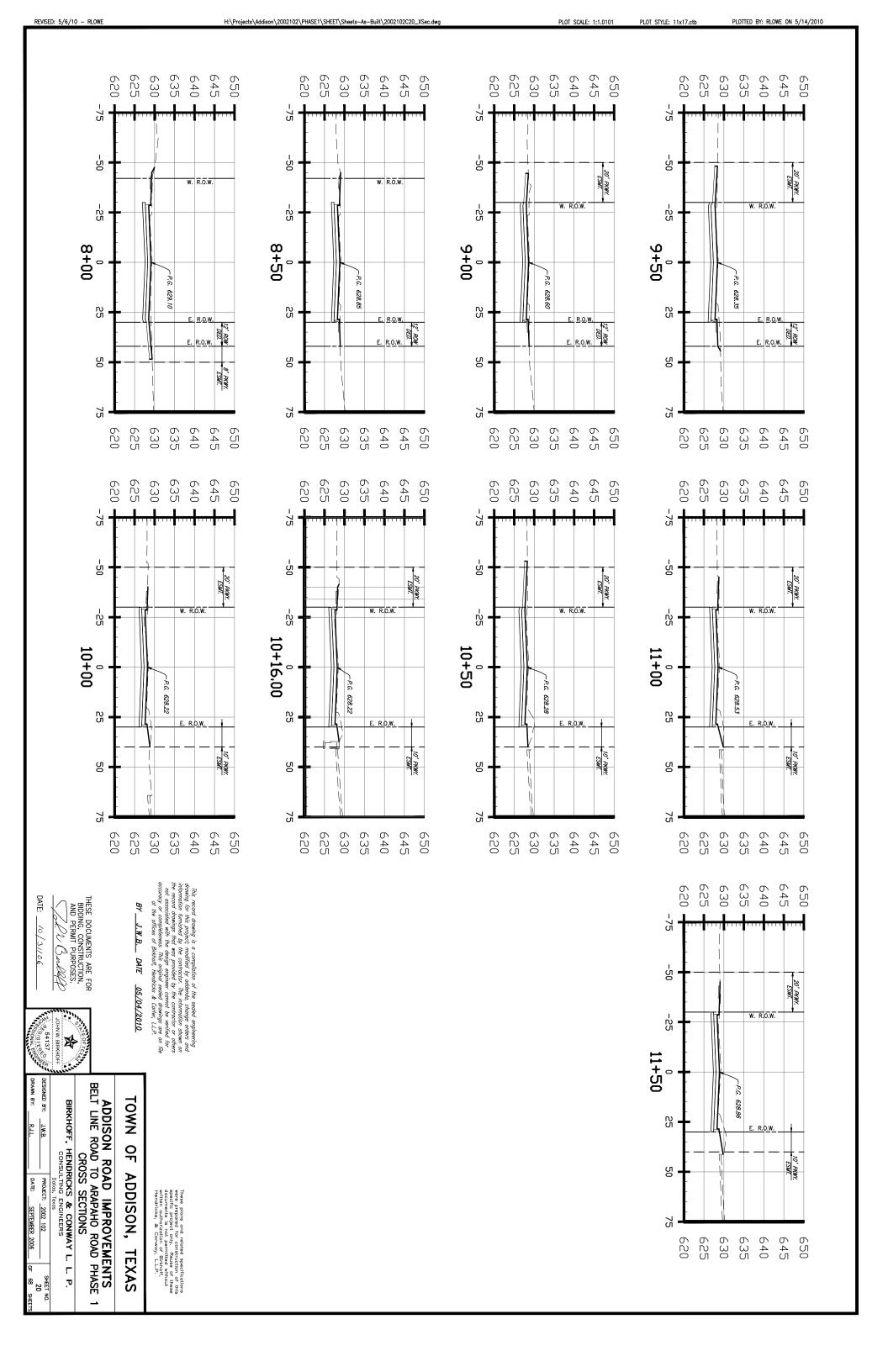


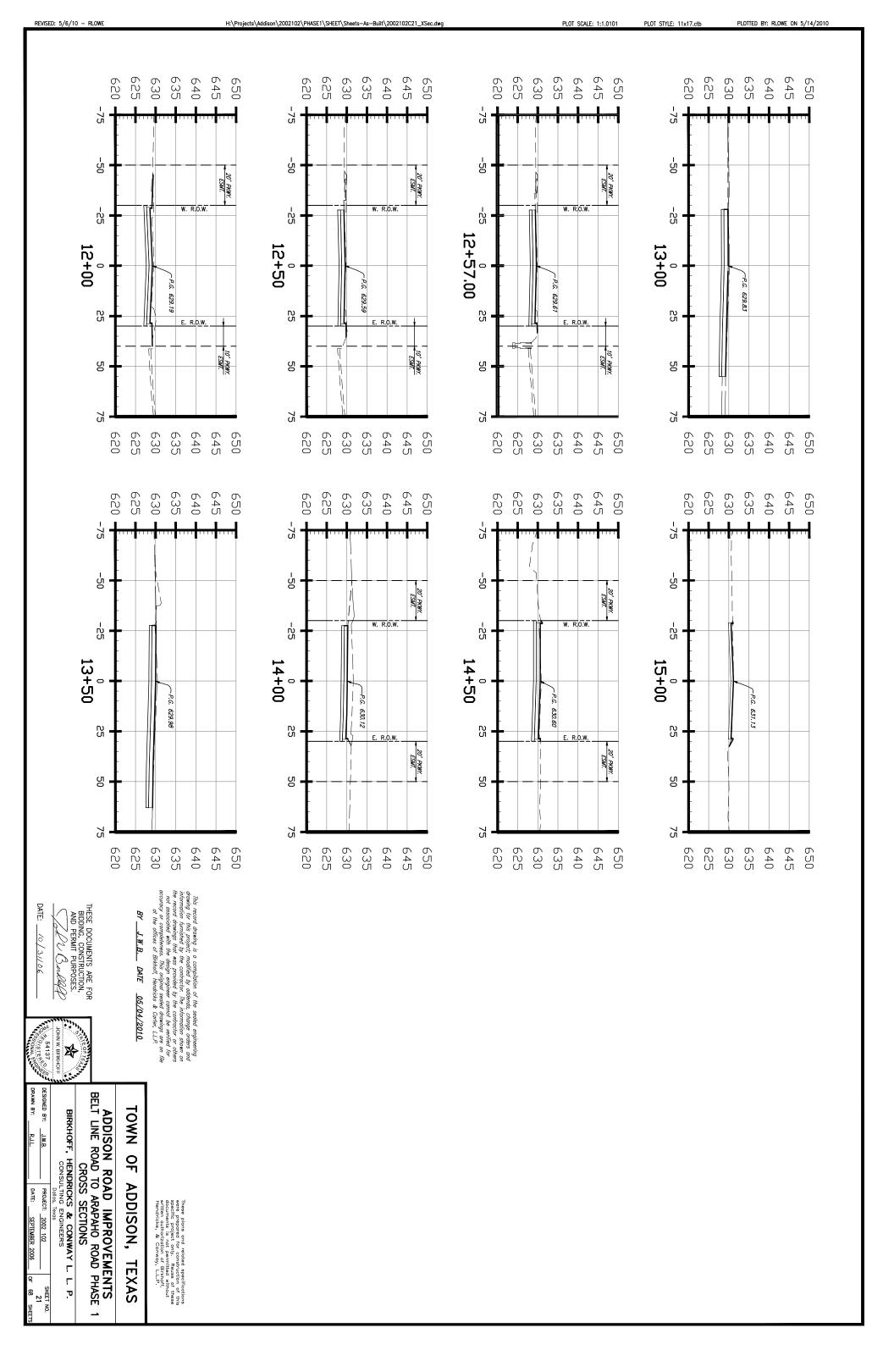


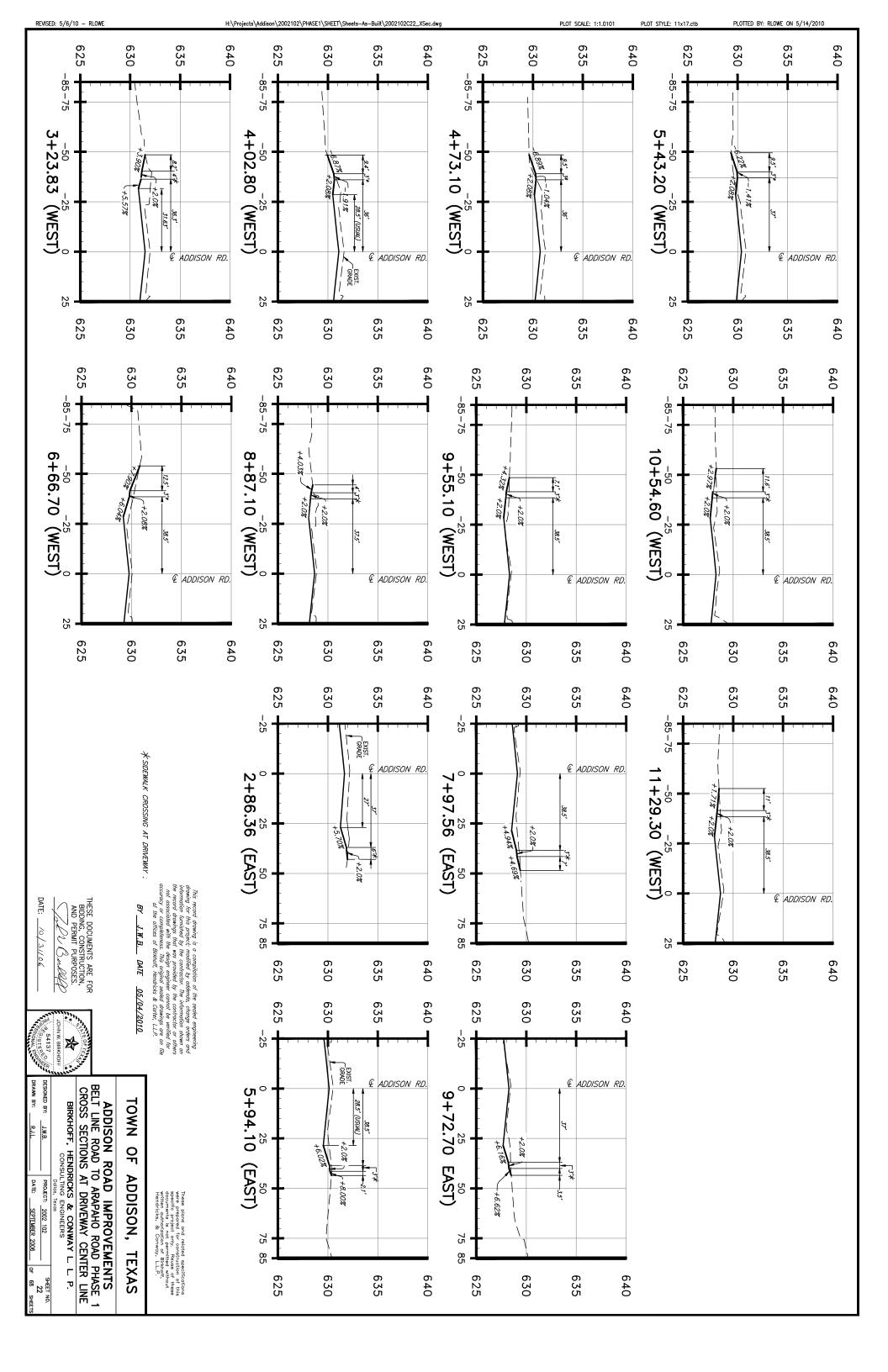








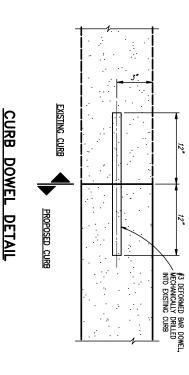


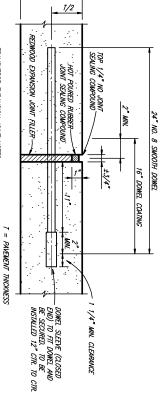


SAWED DUMMY JOINT

TRANSVERSE JOINTS SPACED 15 FT. C.-C. (MAX.) LONGITUDINAL JOINTS SPACED 20 FT. C.-C. (MAX.)

NO. 4 BARS ON 18" CTRS. BOTH WAYS

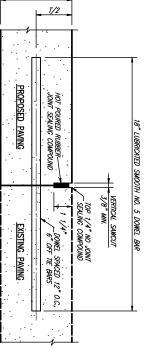




TRANSVERSE EXPANSION JOINT NOTES:

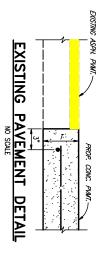
- DOWELS AND REINFORCING BARS SHALL BE SUPPORTED BY AN APPROVED DEVICE.
- TRANSVERSE EXPANSION JOINTS SHALL BE SPACED AT 400 FT. MAXIMUM AND AT DRIVEWAY CONNECTIONS.

TRANSVERSE EXPANSION JOINT



STREET HEADER

NOTE: PAVEMENT BARS TO BE BENT DOWN INTO HEADER. PAVEMENT AND HEADER SHALL BE MONOLITHIC.



T/2 ERTICAL SAW CUT (3/8" WIDE MIN.)— _NO. 4 BARS ON 18". CTRS. BOTH WAYS_7. 1ST POUR <u> 1/3</u> TOP 1/4" NO JOINT HOT POURED RUBBER LAP BARS 30 DM. AND THE

VERTICAL SAW CUT (3/8" MIDE MIN.)-

JOINT SEALING COMPOUND

NOTE.

JONEL BARS SHALL BE DRILLED NITO PAIEMENT HORIZONTALLY BY USE OF A
MICHANICAL OR HAND RG. HORIZONTAL HAND DRILLING METHOD IS ACCEPTABLE.
PUSHING DOMEL BARS NITO GREEN CONCRETE IS NOT ACCEPTABLE.

CONNECTION TO EXISTING PAVEMENT

NOT TO SCALE

CONFACTOR SHALL PROTECT KETWAY PRIOR TO SECOND POUR. IF LONGTIDINAL KETWAY IS DAMAGED, CONTRACTOR SHALL REPAIR MITH THE USE OF LONGTUDINAL BUTT JOINT (DRELL DOMELS INTO FIRST POUR).

T = PAVEMENT THICKNESS

CONSTRUCTION JOINT

BY J.W.B. DATE 05/04/2010

THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES. 黔 interest of

DATE:

DRAWN BY:

BIRKHOFF, HENDRICKS & CONWAY L. L. P.
CONSULTING ENGINEERS
Dallos, Texas

M.W.C.

DATE: SEPTEMBER 2006

PROJECT:

2002 102

ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
DETAILS

TOWN OF ADDISON, TEXAS

GENERAL NOTES

- CONTRACTOR SHALL REMOVE AND PROPERLY DISPOSE THE EXISTING ASPHALT PAVEMENT, STRUCTURE, EXCAMTION, CONCRETE CURB & GUTLER, CONCRETE DRIVENNIS AND CONCRETE SUBMAK.
- ALL CONCRETE TO BE REMONED SMALL BE WITH FULL DETTH SAW CUT, IF JOINT IS WITHIN & OUTSIDE 5 FEET OF R.O.W. OR CURB RETURN, REMONE CONCRETE AT JOINT. FINAL DETERMINATION TO BE MADE IN FRELD BY THE TOWN OF ADDISON.
- CONTRACTOR SHALL PROTECT ALL UNDERGROUND UTILITIES DAMAGE CHUSED BY THE CONTRACTOR SHALL BE REPAIRED AT THE CONTRACTORS EVPENSE.

T/2

HOT PO

SEALING COMPOUND

1/4

_DOWEL SPACED 24" O.C., 6" OFF TIE BARS

24" LUBRICATED SMOOTH DOWEL BAR

VERTICAL SAWCUT 3/8" MIN.

TOP 1/4" NO JOINT SEALING COMPOUND

- CONTRACTOR SHULL LAYOUT PROJECT BASED ON COORDINATES SHOWN IN THE PLANS. COORDINATES SHOWN AT 10 CURB RETURNS, OF PROPOSED ALIOMENTS.

 6) ACRIER LINES OF PROPOSED ALIOMENTS.

 6) AT RECOMMEND AND ENDING OF PROPOSED ALIOMENTS.

 d) AT RELD SURVEY POINTS.
- CONCRETE PAVING SHALL BE 10 INCHES N.C.T.C.O.G. CLASS "A"

- MINIMUM CÓMPRESSNE STRENGTH AT 28 DAYS—3600 P.S.I. MAX. W/C RATIO-6.5 REIMFORCING SHALL BE NO. 4 DEFORMED BARS ON 18 INCH CENTERS

- CONTRACTOR SHALL CONSTRUCT PROPOSED PAVEMENT 1/2 THE PROPOSED WIDTH AT A TIME. CONSTRUCTION JOINTS SHALL HAVE A WELL FORMED KEY WAY OR BE A LONGITUDINAL BUTT JOINT. PAVEMENT BASE SHALL BE SCARIFIED TO A DEPTH OF 6 INCHES AND COMPACTED TO 95% MODIFIED PROCTOR. CONTRACTOR SHALL FURNISH AND PLACE SOLID SOD IN DISTURBED AREAS. (NO PAY ITEM)

3. DOWEL BARS SHALL BE DRILLED INTO PAVEMENT HORIZONTALLY BY USE OF A MECHANICAL RIG. HAND DRILLING NOT ACCEPTABLE. DAMAGE TO EXISTING PAVEMENT SHALL BE REMOVED BY CONTRACTOR AND JOINT CONSTRUCTED AT CONTRACTORS EXPENSE.

TIE BARS SHALL BE NO. 5 BAR DEFORMED. TIE BAR SHALL HAVE A LENGTH OF 24 NICHES. TIE BARS ARE REQUIRED TO BE DEFORMED. DOWEL BAR SHOWN IS IN ADDITION TO TIE BARS (12" O.C.-6" OFF DOWELS).

NON Non

GITUDINAL BUTT JOINT

2. LONGITUDINAL BUT. HINGED (KEYWAY) .

TT CONSTRUCTION MAY BE UTILIZED IN PLACE OF LONGITUDINAL JOINT AT CONTRACTORS OPTION.

NOTE: 1. T—8" AND GREATER NO. 6 BAR, T—6" AND LESS NO. 5 BAR

PROPOSED PAVING

EXISTING PAVING

EXPANSION JOINT FILLER EXISTING PVMT.

PROP. CONC. PIMI.

FUTURE PUNT.

PROP. CONC. PVMT.

-PAVEMENT PAY LINE

-PAVEMENT PAY LINE

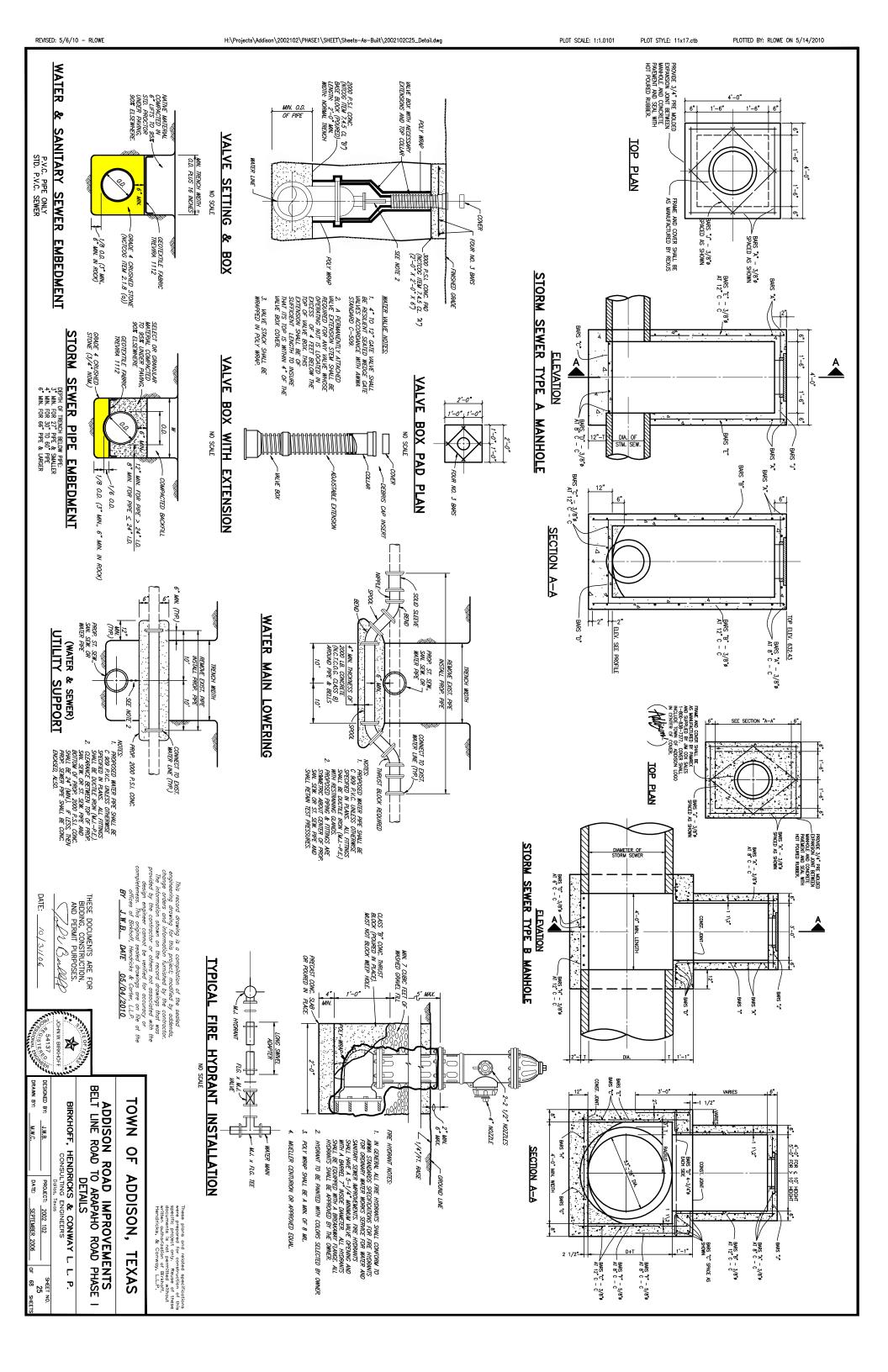
EXIST. OR PROP. PVMT. 1
BASE MATERIAL (TO REMAIN) EXIST. OR PROP. ASPH.
SURFACE COURSE
(TO REMAIN) GEOTEXTILE TREVIRA 11 EQUAL. E FABRIC 1120 OR -. ► MN. SECOND SAWCUT FOR REPLACEMENT UNDISTURBED
TRENCH BANK FOR TRENCH MN 12 8" REINF. CONC. 3600 P.S.I. @ 28 DAYS MIN. 5 SACK MIX REINF. #3 DEFORMED BASS ON 24" CENTERS EACH WAY STEEL SUPPORTED ON PLASTIC CHAIRS MATCH EXIST. PWMT. IF MORE THAN 8" THICK. FULL SAW CUT EXIST. PVMT.
-W/ LONGITUDINAL BUTT JOINT

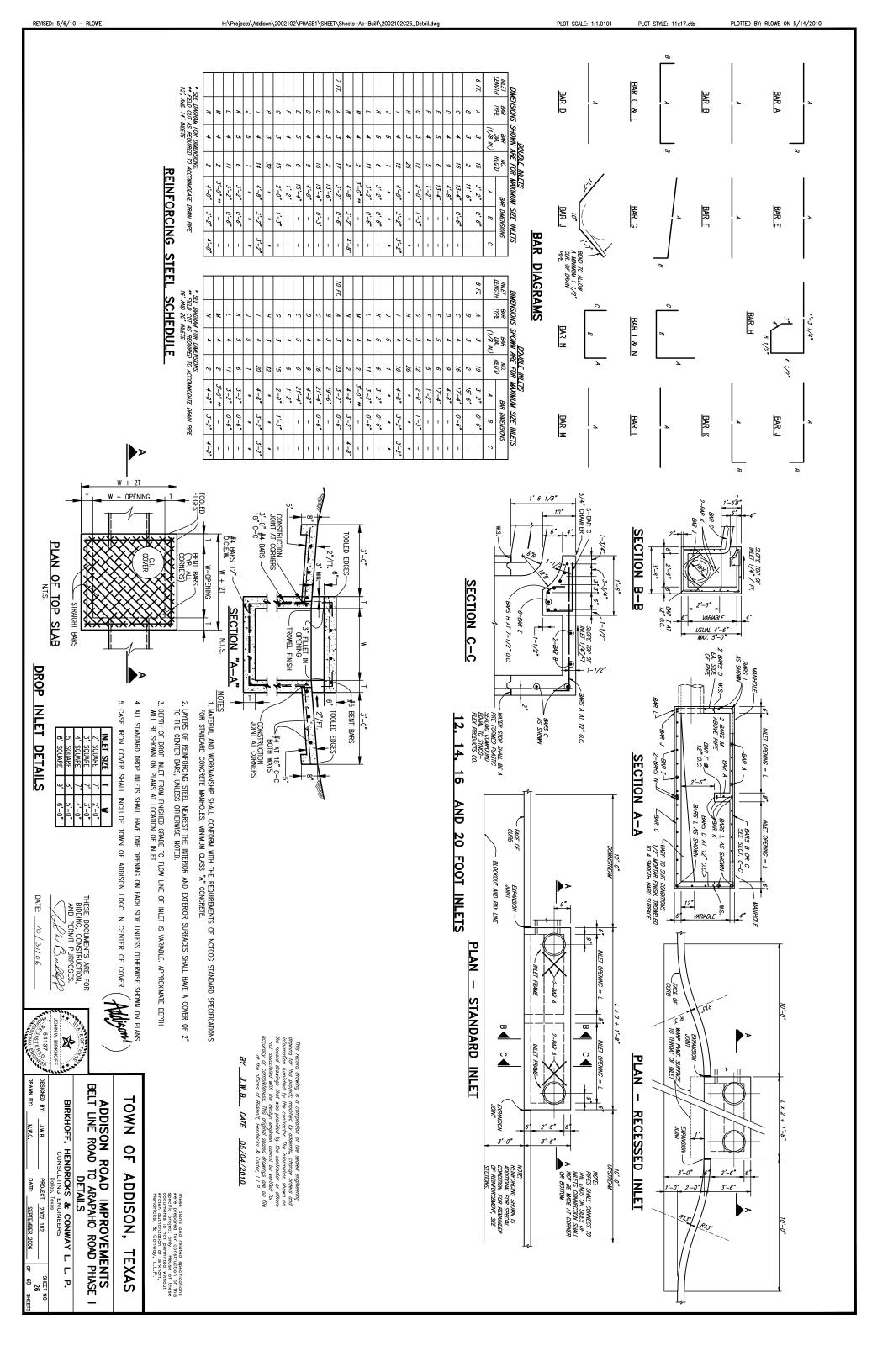
PROPOSED — PIPE STREET REPAIR

PIPE LINE CROSSING CONCRETE SELECT FILL COMPACTED TO 95% MODIFIED PROCTOR PLACED IN 8 NICH LOOSE LIFTS AND MECHANICALLY COMPACTED. CRUSHED STONE

This record drawing is a compilation of the scaled engineering drawing for this project; modified by oddenda, change orders and information furnished by the confractor. The information stown on the record drawings that was provided by the confractor or others not associated with the design engineer cannot be retried for accuracy or compeleness. This original scaled drawings are on file at the offices of Birkhoff, Hendricks & Carter, L.L.P.

These plans and related specifications were prepared for construction of this specific project only. Reuse of these specific project only. Reuse of the documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.





(1) Quantities increase slightly for metal pipe installations.

Structure

Bars E

W/2

TABLE OF VARIABLE DIMENSIONS AND QUANTITIES FOR ONE HEADWALL

Reinf (Conc) (Lbs) (1) 0" 122 1.1 1 1.3 2.4 3.7 1.3 1.5 2.7 2.7 2.7 2.7 4.7 2.7 2.7 4.7 2.7 2.7 4.7 2.9 2.9 2.0 2.0 2.9 2.6 2.2 2.7 2.9 2.0 2.0 2.0 2.2 2.7 2.9 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0	-		i	Values for		֓֟֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֟֓֓֓֓֓֓֓֓֓֓֟֓֓֓֟	Value	<u>-</u>	<u>-</u>
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72" 64'- 0" 2069 22.4 9'- 12" 25'- 0" 336 3.0 1'- 15" 28'- 3" 384 3.6 2'- 18" 31'- 6" 452 4.2 2'- 21" 34'- 9" 581 5.1 3'- 21" 34'- 9" 644 5.8 3'- 21" 41'- 3" 737 6.9 3'-1 27" 41'- 3" 737 6.9 3'-1 30" 44'- 6" 807 7.7 4'- 33" 47'- 9" 912 8.9 4'- 36" 51'- 0" 1108 11.0 5'- 48" 67'- 6" 1318 13.7 5'- 54" 73'- 6" 2064 21.3 7'- 60" 80'- 6" 2635 28.9 8'- 72" 93'- 0" 3123 53.1 9'-	_	<u>σ</u>	25	1	- 1	• •			-
12 24 36 3.6 2-1 115 28 - 3 36 3.6 2-1 115 28 - 3 3 36 3.6 2-1 115 31 - 6 4 452 4.2 2-1 118 31 - 6 4 452 4.2 2-1 118 31 - 6 4 452 4.2 2-1 118 31 - 6 4 452 4.2 2-1 118 31 - 6 5 6 6 6 6 6 7 6 7 7.7 4-1 118 30 4 - 6 6 7 6 7.7 4-1 118 30 4 - 6 7 6 7.7 4-1 118 11.0 5-1 118 118 11.0 5-1 118 118 11.0 5-1 118 118 118 118 118 118 118 118 118 11	_	710	1	, u		٠.			+
15" 28'- 3" 384 3.6 2'- 18" 31'- 6" 452 4.2 2'- 21" 34'- 9" 581 5.1 3'- 24" 38'- 0" 644 5.8 3'- 27" 41'- 3" 737 6.9 3'- 30" 44'- 6" 807 7.7 4'- 33" 47'- 9" 912 8.9 4'- 33" 47'- 6" 1108 11.0 5'- 42" 57'- 6" 1318 13.7 5'- 48" 67'- 0" 1674 17.9 6'- 54" 73'- 6" 2645 28.9 8'- 66" 86'- 6" 2635 28.9 8'- 72" 93'- 0" 3123 53.1 9'-	T		Ž,	ו ח ר				*	+
10 28 3.6 2.7 2.6 2.7 2.7 2.7 2.7 34' 9" 581 5.1 3' 2.7 2.7 2.7 38' 0" 644 5.8 3' 2.7 2.7 6.9 3-1 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 3' 3.7 6.9 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 5' 3.7 6" 1318 13.7 6" 3.7 6" 1318 13.7 6" 3.7 6"		1-	1	2"	300	10.0		*	+
18" 31' - 6" 452 4.2 2'- 21" 34' - 9" 581 5.1 3'- 24" 38' - 0" 644 5.8 3'- 27" 41' - 3" 737 6.9 3'-1 30" 47' - 9" 912 8.9 4'- 33" 57' - 6" 1108 11.0 5'- 42" 57' - 6" 1138 13.7 5'-1 48" 67' - 0" 1674 17.9 6'- 42" 57' - 6" 2064 21.3 7'- 60" 80' - 6" 2635 28.9 8'- 60" 86' 6" 2635 28.9 8'- 60" 86' 6" 2635 28.9 8'-		1	5	8 - 3	384	3.6		12	
21" 34'- 9" 581 5.1 3'- 24" 38'- 0" 644 5.8 3'- 27" 41'- 3" 737 6.9 3'-1 30" 44'- 6" 807 7.7 4'- 33" 47'- 9" 912 8.9 4'- 36" 51'- 0" 1108 11.0 5'- 42" 57'- 6" 1318 13.7 5'-1 48" 67'- 0" 1674 17.9 6'- 42" 67'- 6" 2064 21.3 7'- 60" 80'- 6" 2635 28.9 8'- 60" 86'- 6" 2635 28.9 8'-		1=	8	1	452	4.2		ω	_
24" 38'- 0" 644 5.8 3 27" 41'- 3" 737 6.9 3 30" 44'- 6" 807 7.7 4 33" 47'- 9" 912 8.9 4 36" 51'- 0" 1108 11.0 5 42" 57'- 6" 1318 13.7 5 48" 67'- 0" 1674 17.9 6 54" 73'- 6" 2635 28.9 8 66" 86'- 6" 2635 28.9 8		N	1,"	1	581	•		"	
27" 41'-3" 737 6.9 3 30" 44'-6" 807 7.7 4 33" 47'-9" 912 8.9 4 36" 51'-0" 1108 11.0 5 42" 57'-6" 1318 13.7 5 48" 67'-0" 1674 17.9 6 54" 73'-6" 2645 21.3 7 60" 80'-0" 2345 28.9 8 66" 86'-6" 2635 28.9 8		N	4	1	644	•	- 1	7,"	-
30" 44'- 6" 807 7.7 4 33" 47'- 9" 912 8.9 4 36" 51'- 0" 1108 11.0 5 48" 67'- 0" 1674 17.9 6 54" 73'- 6" 2064 21.3 7 60" 80'- 0" 2343 24.9 8 66" 86'- 6" 2635 28.9 8		N)	7"	ι ω	737			•	-
33" 47'-9" 912 8.9 4 36" 51'-0" 1108 11.0 5 42" 57'-6" 1318 13.7 5 48" 67'-0" 1674 17.9 6 60" 80'-0" 2343 24.9 8 66" 86'-6" 2635 28.9 8		<u>~</u> [,	ו	807		- 1 -	-	· •
36" 51' 0" 1118 11.0 5 42" 57' 6" 1318 13.7 5 48" 67' 0" 1674 17.9 6 54" 73' 6" 2064 21.3 7 60" 80' 0" 2343 24.9 8 66" 86' 6" 2635 28.9 8	1	4 6	,) 	04.5		- 1	g '	1
36 57 - 6" 1318 11.0 3 42" 57' - 6" 1318 17.7 5 48" 67' - 0" 1674 17.9 6 54" 73' - 6" 2064 21.3 7 60" 80' - 0" 2343 24.9 8 66" 86' - 6" 2635 28.9 8	6:1	بازد	<u>`</u>	, ,	10	- 0	- 1	1	44
57' - 6" 1318 13.7 5 67' - 0" 1674 17.9 6 73' - 6" 2064 21.3 7 80' - 0" 2343 28.9 8 86' - 6" 2635 28.9 8 93' - 0" 3123 33 4		ω	. 0			١.	5-1	1	<u> </u>
67'-0" 1674 17.9 6 73'-6" 2064 21.3 7 80'-0" 2343 24.9 8 86'-6" 2635 28.9 8		4	2"	7'-		•	5'-10	•	
73'- 6" 2064 21.3 7'- 80'- 0" 2343 24.9 8'- 86'- 6" 2635 28.9 8'- 93'- 0" 3123 53.1 9'-		4	۳,	7'-			6'- 7"	-	-
80'- 0" 2343 24.9 8'- 86'- 6" 2635 28.9 8'- 93'- 0" 3123 33 1 9'-		úι	4″	3′-	- 1	.		-	4
86' - 6" 2635 28.9 8'-		ച	<u> </u>	, 0"	343	• 1 •		<u>ح</u>	+
93'- 0" 3123 33 1 9'-		9	0;) 6	635	- 1.	- 1		96
		राव	,	2 0	3 0	١-		A. U	_

9" 9"

2 Indicated slope is perpendicular to Q Pipe or Pipes

ELEVATION

Bars A 1

Bars

" 1'- 4" 1'- 0" 3'- 5" 9" 2'- 0"
" 1'- 7" 1'- 0" 3'- 8" 9" 2'- 0"
" 1'- 8" 1'- 0" 3'- 8" 9" 2'- 0"
" 1'- 10" 1'- 0" 4'- 2" 9" 2'- 3"
" 1'- 11" 1'- 0" 4'- 5" 9" 2'- 6"
" 2'- 1" 1'- 0" 4'- 8" 1'- 0" 2'- 6"
" 2'- 4" 1'- 0" 5'- 2" 1'- 0" 2'- 6"
" 2'- 7" 1'- 3" 5'- 11" 1'- 0" 3'- 0"
3'- 3" 1'- 3" 5'- 11" 1'- 0" 3'- 6"
" 3'- 3" 1'- 3" 6'- 11" 1'- 0" 3'- 6"
" 3'- 4" 1'- 3" 7'- 11" 1'- 0" 3'- 6"

(4) TABLE C

OF STEEL

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Size

Spa

No.

1'-0"

Bars

A 2

12" 15" 18" 21" 24" 27" 30"

DIA OF PIPE,D

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CONSTANT DIMENSIONS

TABLE OF

E Pipe or Pipes

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O

0

Bars E

This record drawing is a compilation of the sealed engineering drawing for this project, modified by addends, change orders and information fundabled by the contraction. The information shown on the record drawings that was provided by the contractor or others not associated with the design engineer cannot be verified for accuracy or completeness. This original sealed drawings are an file at the offices of Birkhoff, Hendricks & Carter, L.L.P.

J.W.B.05/04/2010

3) For vehicle safety, curb heights and wall heights shall be reduced, if necessary, to provide a maximum 3" projection above finished grade.

No changes will be made in quantities and no additional compensation will be allowed for this work.

THESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.

DATE:

10/31/06

JOHN W. BIRKHOFF

DRAWN BY: DESIGNED BY:

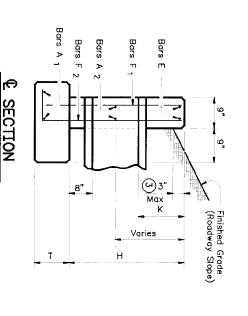
DATE:

SEPTEMBER 2006

PROJECT: 2002 102

J.W.B.

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GENERAL NOTES:

Designed according to current AASHTO Standard and Interim Specifications.

Reinforcing steel shall be placed with the center of the outside layer of bars 2" from the surface of the concrete.

All reinforcing steel shall be Grade 60.

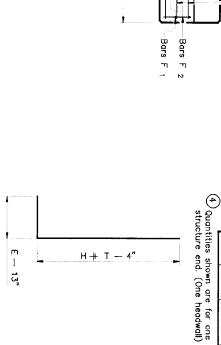
All concrete shall be Class "C" and shall have a minimum 28 day compressive strength of 3600 psi.

TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE I
CH-PW-0 CONCRETE HEADWALL DETAIL
BIRKHOFF, HENDRICKS & CONWAY L. L. P.
CONSULTING ENGINEERS SHEET NO.

28
68 SHEE

These plans and related specifications were prepared for construction of this specific project only. Reuse of these specific project only. Reuse of these documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.



PLAN OF NON-SKEWED PIPES

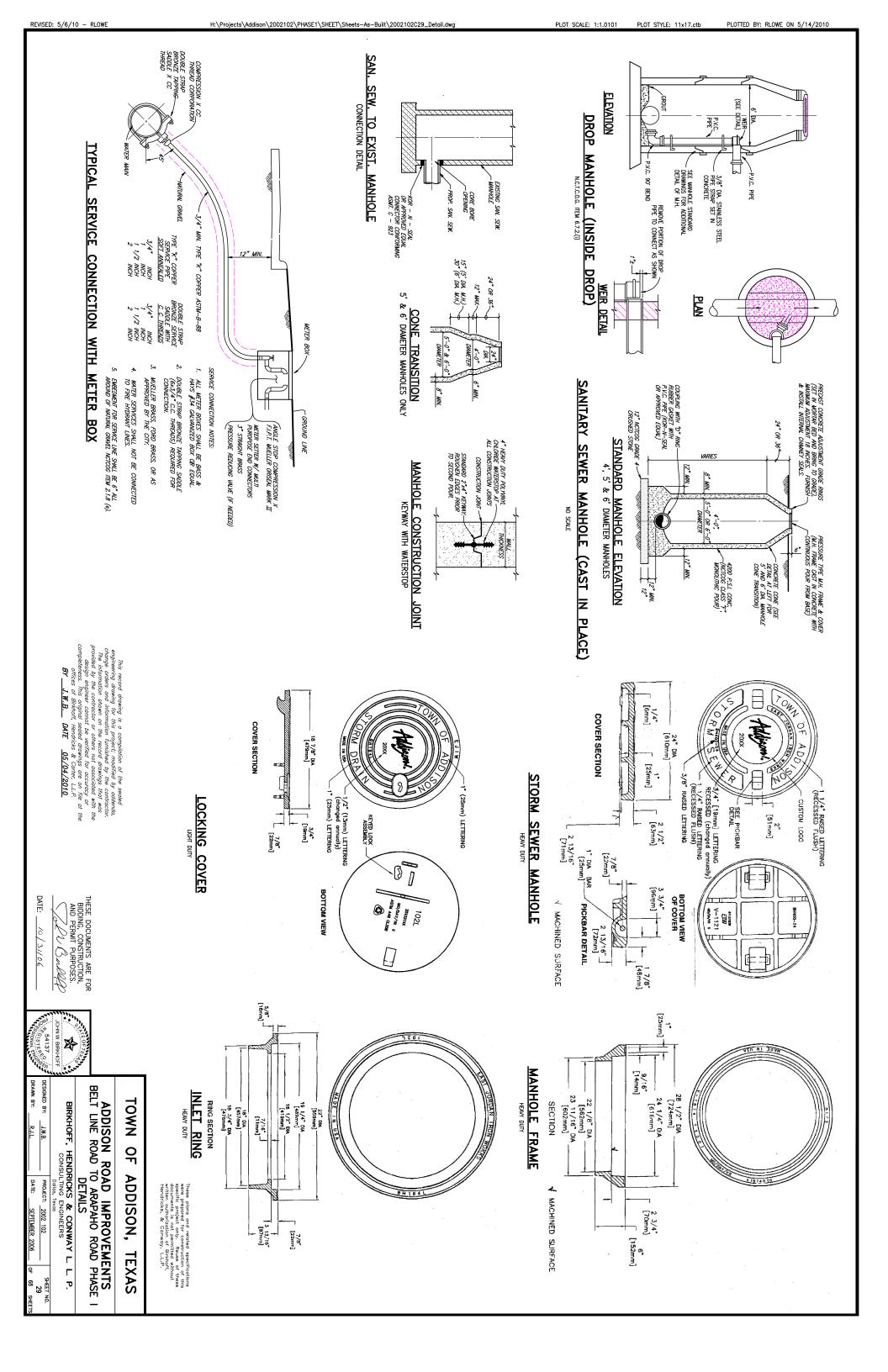
Toe of Slope

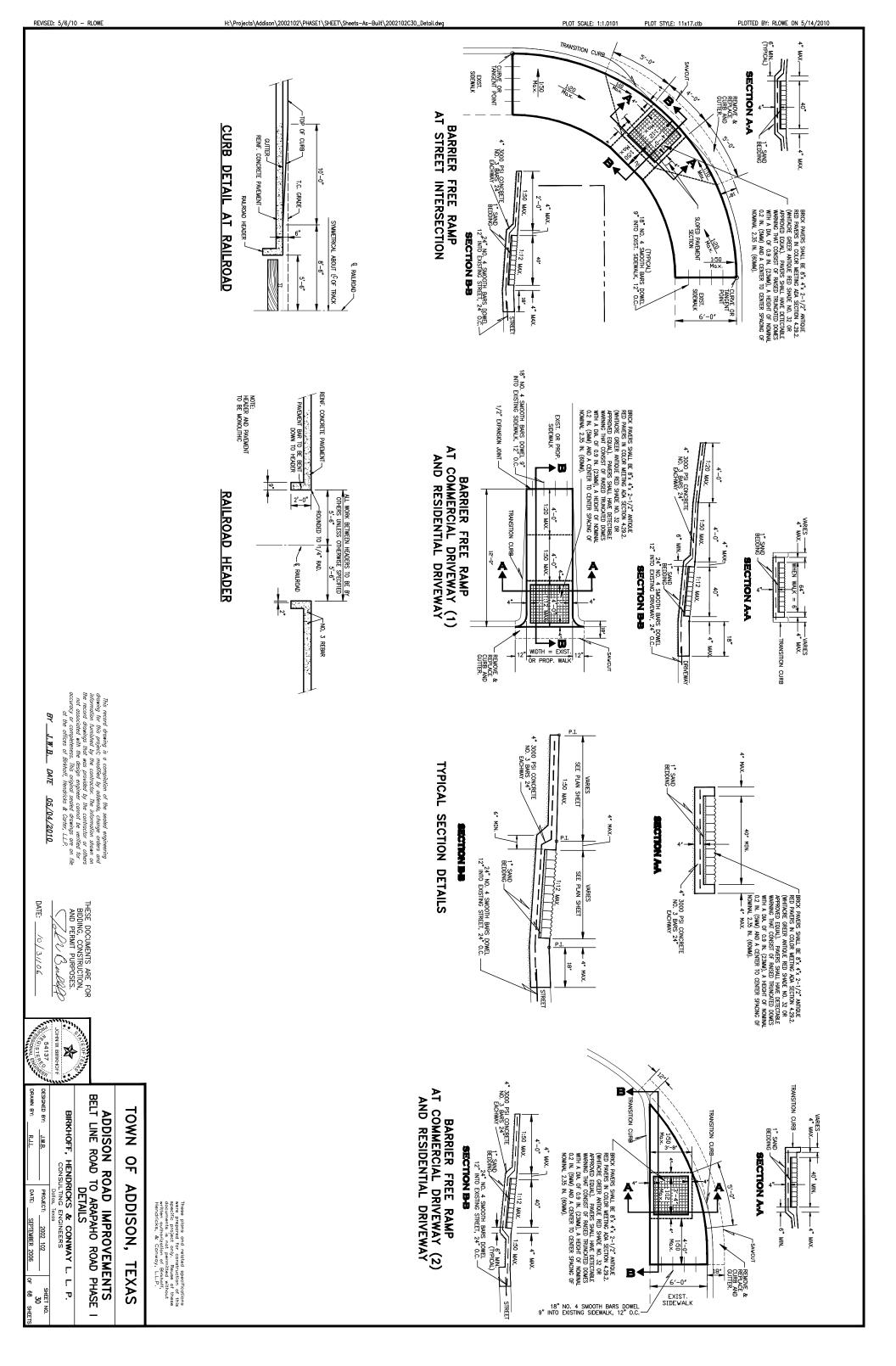
W/2

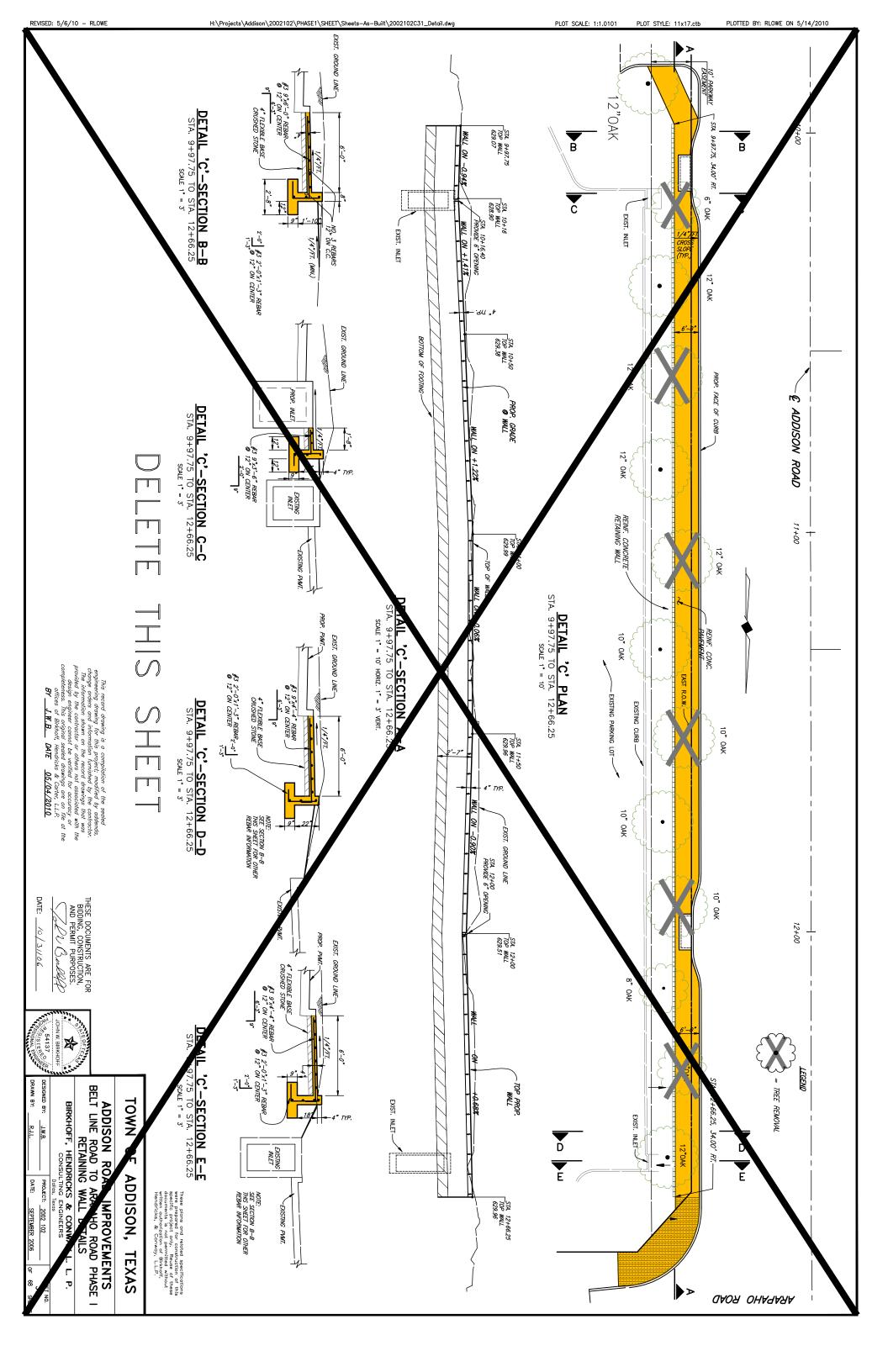
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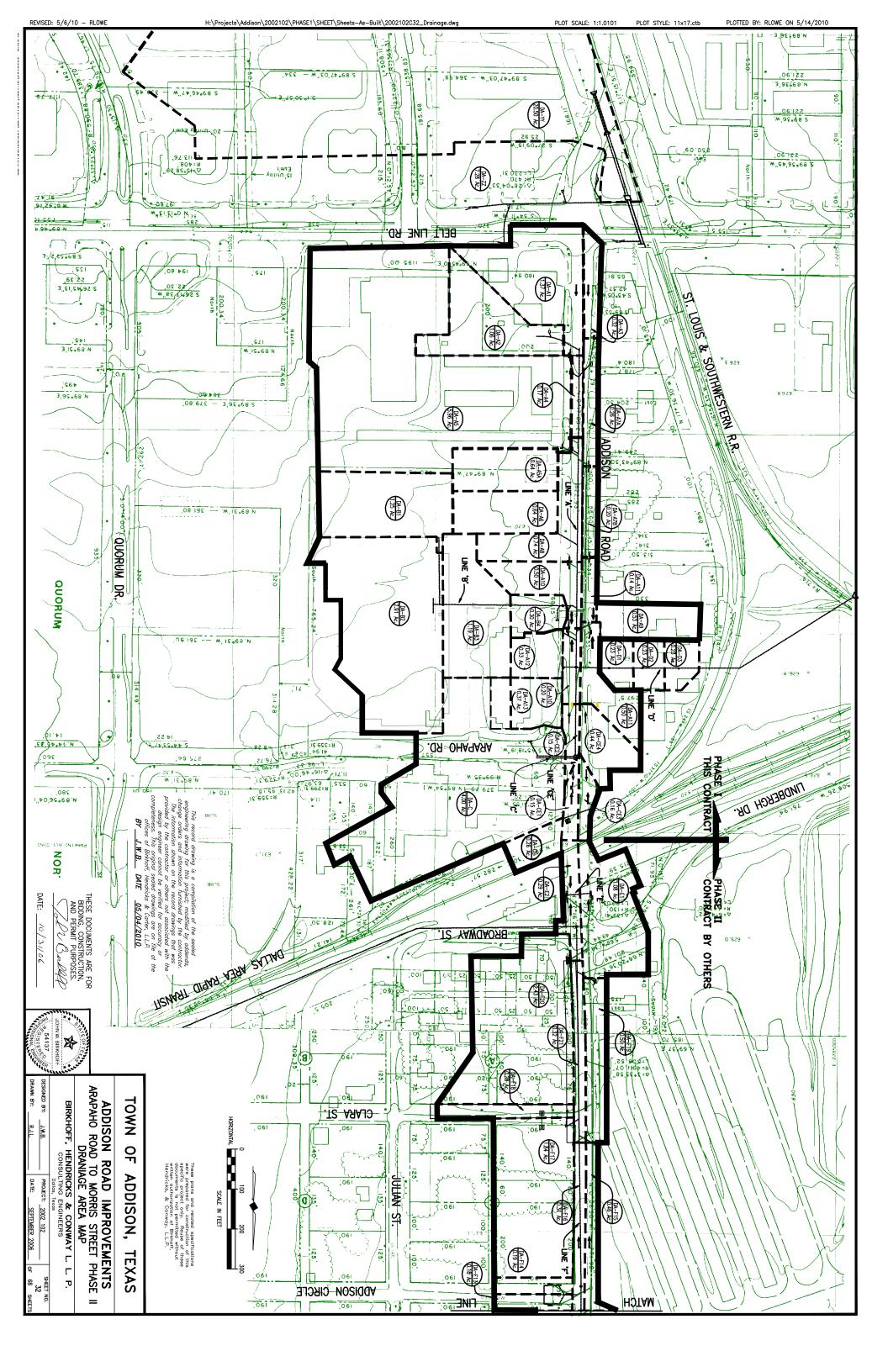
ᡊ Structure

BAR		-		
BARS F 2	E 1 13.		H # T — 4"	









RUNOFF CALCULATIONS -

BELT LINE ROAD TO ARAPAHO ROAD

CE	D3	D2	D1	CE4	CE3	CE2	CEI		2	B4	B 3	B2	В1		A13	A12	A11	A10	A9	A8	A7B	A7A	A6	ASA	A5	A4	А3	A2	A1		NUM.	AREA	
0.26	0.23	0.23	0.23	0.44	0.16	0.15	0.15	4.00	3	0.30	1.19	2.91	1.25		0.37	0.33	0.64	0.70	0.53	0.74	0.20	0.26	0.64	0.64	5.96	0.17	0.32	1.06	1.37	(AC.)		AREA	
0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.70		0.90	0.90	0.90	0.90		0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	"С"	COEFF.	RUNOFF	
100	100	100	100	100	100	100	100	100	3	100	100	100	100		100	100	100	100	100	100	100	100	100	100	100	100	100	100	100	(YEAR)	FREQ.	STORM	NUISEC
10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	500	10.00	10.00	10.00	10.00		10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	(MIN.)	CONC.	OF	TIME
8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	0.,1	0 74	8.74	8.74	8.74	8.74		8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	(IN./HR.)		INTENSITY	
2.05	1.81	1.81	1.81	3.46	1.26	1.18	1.18	06	21 46	2.36	9.36	22.89	9.83		2.91	2.60	5.03	5.51	4.17	5.82	1.57	2.05	5.03	5.03	46.88	1.34	2.52	8.34	10.78	(C.F.S.)		Q= CIA	
Commercial / Retail		Commencial / Retail	Commercial / Retail	Commercial / Retail	Commercial / Retail	Commercial / Retail		Conmercial / Retail	Commercial / Retail	Comnercial / Retail	Conmercial / Retail	Commercial / Retail	Commercial / Retail			REMARKS																	

JOHN W BIRKHOFF

54137

64137

69131 DESIGNED BY: J.W.B.

DRAWN BY: R.J.L.

TOWN OF ADDISON, TEXAS

ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE I
RUNOFF CALCULATIONS BIRKHOFF, HENDRICKS & CONWAY L. L. P.
CONSULTING ENGINEERS
Dollos, Texas

PROJECT: 2002 102 SHEET NO. 33

DATE: SEPTEMBER 2006 OF 68 SHEET

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BY J.W.B. DATE 05/04/2010

STORM SEWER LINES "A,B,C & D" STORM SEWER CALCULATIONS

	493.50	584.60	667.30	749.90			72.50	78.22	226.93	282.14	288.27	398.00			89.32	103.77	144.00	240.42	264.23	495.18	512.75			67.01	80.51	147.84	242.72	294.11	526.88	554.70	747 60	784.55			1	Opstream Station	
	0.00	493.50	584.60	667.30			0.00	72.50	78.22	226.93	282.14	288.27			0.00	89.32	103.77	144.00	240.42	264,23	495.18			0.00	67.01	80.51	147.84	242.72	204 11	528.88	00.717	720.06			2	Downstream Station	,
	493.50	91.10	82.70	82.60			72.50	5.72	148.71	55.21	6.13	109.73			89.32	14.45	40.23	96.42	23.81	230.95	17.57			67.01	13.50	67.33	94.88	51.39	232 77	27.91	2.00	64.49			s	Collection Points)
	D3	D2	9	A, A13,C&CE			CE4	CE2	င	CE3	CE1	CE	-		A11	Ą	LINEB	A6 & A8	А7В	A7A	A5 & A5A			A12	A10	L NE B	æ	A6 5	AR & ARA	2 2	: }	. A.			4	No.	_
25.36	0.23	0.23	0.23	24.67		5.16	0.44	0.15	4.00	0.16	0.15	0.26		8.43	0.64	0.53	3.00	0.80	0.20	0.26	3.00		10.78	0.33	0.70	2.65	0.34	0.24	3.60	0.17	1.32	1.06		1	5	(Ac,)	*
	0.90	0.90	0.90	0.90			0,90	0.90	0.90	0.90	0.90	0.90			0.90	0.90	0.90	0.90	0.90	0.90	0.90			0.90	0.90	0,90	0.90	0.90	0.00	2 5	0.80	0.90			a	Coeff.)
	0.21	0.21	0.21	22.20			0.40	0.14	3.60	0.14	0.14	0.23			0.58	0.48	2.70	0.72	0.18	0.23	2.70			0.30	0.63	2.39	0.31	0.22	324	0 15	1 22	0.95			7	ental CA"	
	22.82	22.62	22.41	21.97			4.64	4.25	4.11	0.61	0.37	0.23			7.59	7.01	6.53	3.83	3.11	2.93	2.70			9.70	9.41	8.78	6.39	6.08	5.87	283	7.24	0,95			8	"CA"	
	12.68	12.51	12.36	12.21			13.35	13.33	12.95	12.26	12.15	10.00			11.64	11.58	11.43	11.08	10.97	10.07	10.00			12.97	12.93	12.73	12.35	12.13	11 38	11 27	10.65	10.00			8	(MIN.)	2
	8	100	18	8			8	13	8	18	100	100	Existing Li		100	8	100	100	100	100	100	P		8	8	8	8	8	3 8	3 8	3 8	8	Existing Line A		10	(Yrs.)	1
	8.05	8.06	8.07	8.10	Existing Line D		7.84	7.85	7.94	8.09	8.12	8.74	Existing Line C & Proposed Line CE		8.25	8.27	8.30	8.40	8.42	8.70	8.74	Proposed Line A1		8.01	8.02	8.06	8,15	8.18	8 42	8.45	0.00	8.74	Existing Line A With/ Prop. Parallel Line A1		11	(ln./Hr.)	}
	183.73	182.29	180.85	177.96			36.41	33.35	32.66	4.15	3.00	2.05	d Line CE		62.59	57.98	54.23	32.21	26.22	25.53	23.60			77.71	75.43	70.73	52.08	49.77	49 41	29 21	10.76	8.34	Irallel Line A1		12	(Q) (c.f.s.))
	0.0050	0.0049	0.0048	0.0047			0.0030	0.0025	0.0063	0.0003	0.0002	0.0002			0.0014	0.0016	0.0014	0.0023	0.0015	0.0023	0.0020			0.0029	0.0028	0.0024	0.0013	0.0012	0.0024	0.0029	0.0007	0.0004			13	(Ft./Ft.)	
	-		-	_			_	_		_	-				1	1	-1	1	1	-	1				_		_		. .	- -	. -	-				Pipes	!
	8	8	8	60			36	36	ષ્ઠ	24	24	21			51	48	48	ઝ	36	33	33			\$	&	&	4 8	&	2	8 8	3 8	8 8			14	Sewer	1
	9.36	9.28	9.21	9.06	11.67		5.15	4.72	6.65	1.32	0.95	0.85			4.41	4.61	4.32	4.56	3.71	4.30	3.97			6.18	6.9	5.63	4.14	3.96	5 14	4.50	7 26 7	1.70			15	(f.p.s.)	7
	_	-	-	 - 			-	-	-	_	-1	-			-	1	1	-	-		1			-	-	-	-	_		- -	-	. _			16	7. TOOR	2
	0.02	0.02	0.04	0.00			0.00	0.32	0.67	0.02	0.01	0.00			0.00	0.04	0.00	0.11	0.00	0.29	0.00			0.03	0.07	0.23	0.02	0.00	0.00	0.00	0.07	0.00			17	(Feet)	
	0.88	0.16	0.15	0.15			0.23	0.02	0.37	0.70	0.11	2.15			0.34	0.05	0.18	0.35	0.11	0.90	0.07			0.18	0.04	0.20	0.38	0.22	0.78	0.10	0.02	0.63			18	(Min.)	1 10001
	13.55	12.68	12.51	12.36			13.58	13.35	13.33	12.95	12.26	12.15			11.97	11.64	11.58	11.43	11.08	10.97	10.07			13.15	12.97	12.93	12.73	12.35	12 13	11 38	11.03	10.63			19	Station (Min.)	21-1121
																											0.40 Ac. To Prop. A1	0.40 Ac. To Prop. A1	3 00 Ac To Prop A1						20		

DATE: _

JOHN W. BIRCHOFF

S4137 0 8

S6137 0 8

S6137 0 8

S6137 0 8 DESIGNED BY: J.W.B. DRAWN BY: __

ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE I
STORM SEWER CALCULATIONS BIRKHOFF, HENDRICKS & CONWAY L. L. P.
CONSULTING ENGINEERS
Dollas, Texas DATE: SEPTEMBER 2006 PROJECT: 2002 102

These plans and related specifications were prepared for construction of this specific project only. Reuse of these documents is not permitted without written authorization of Birkhoff, Hendricks, & Conway, L.L.P.

TOWN OF ADDISON, TEXAS

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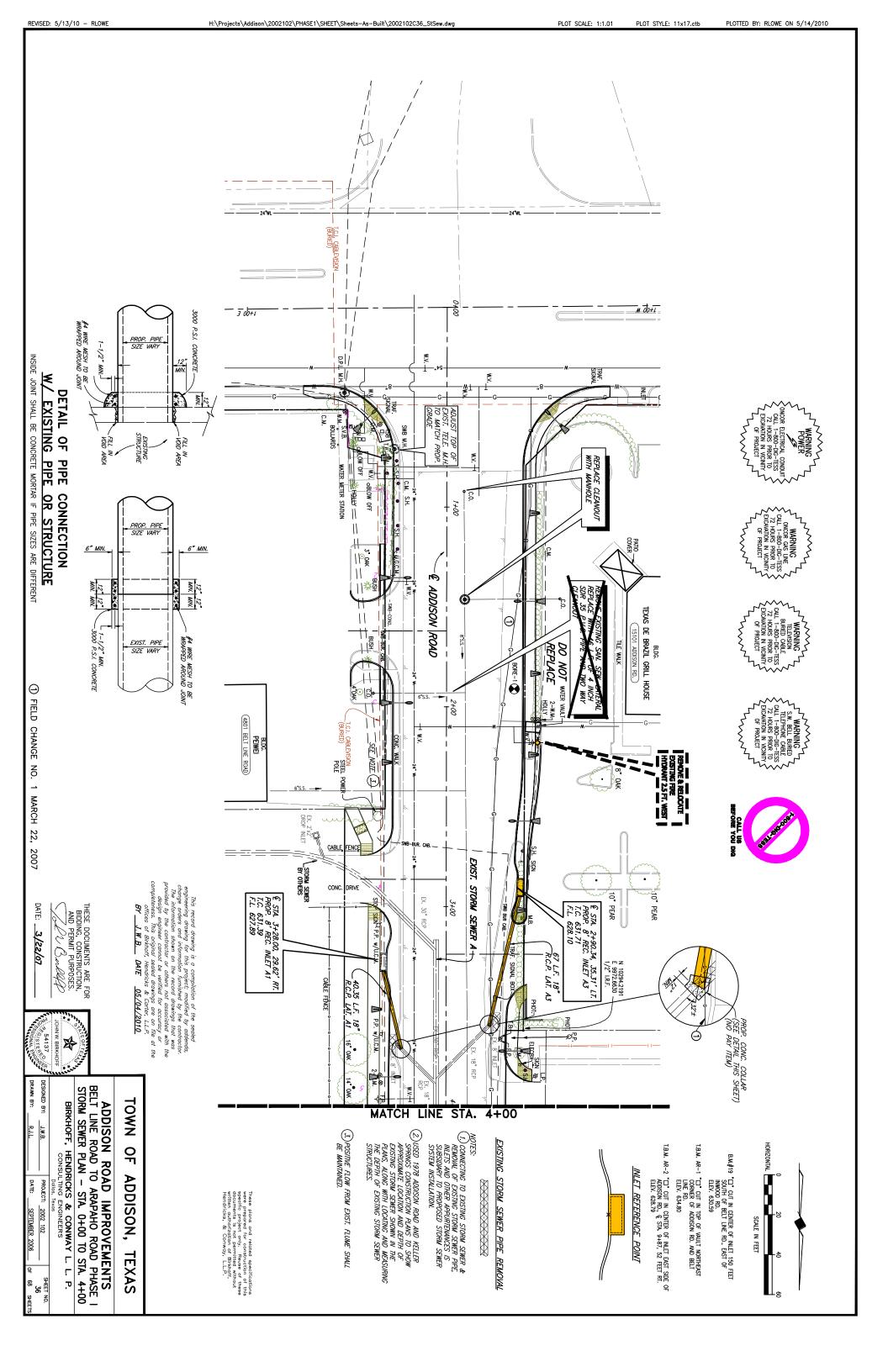
BY J.W.B. DATE 05/04/2010

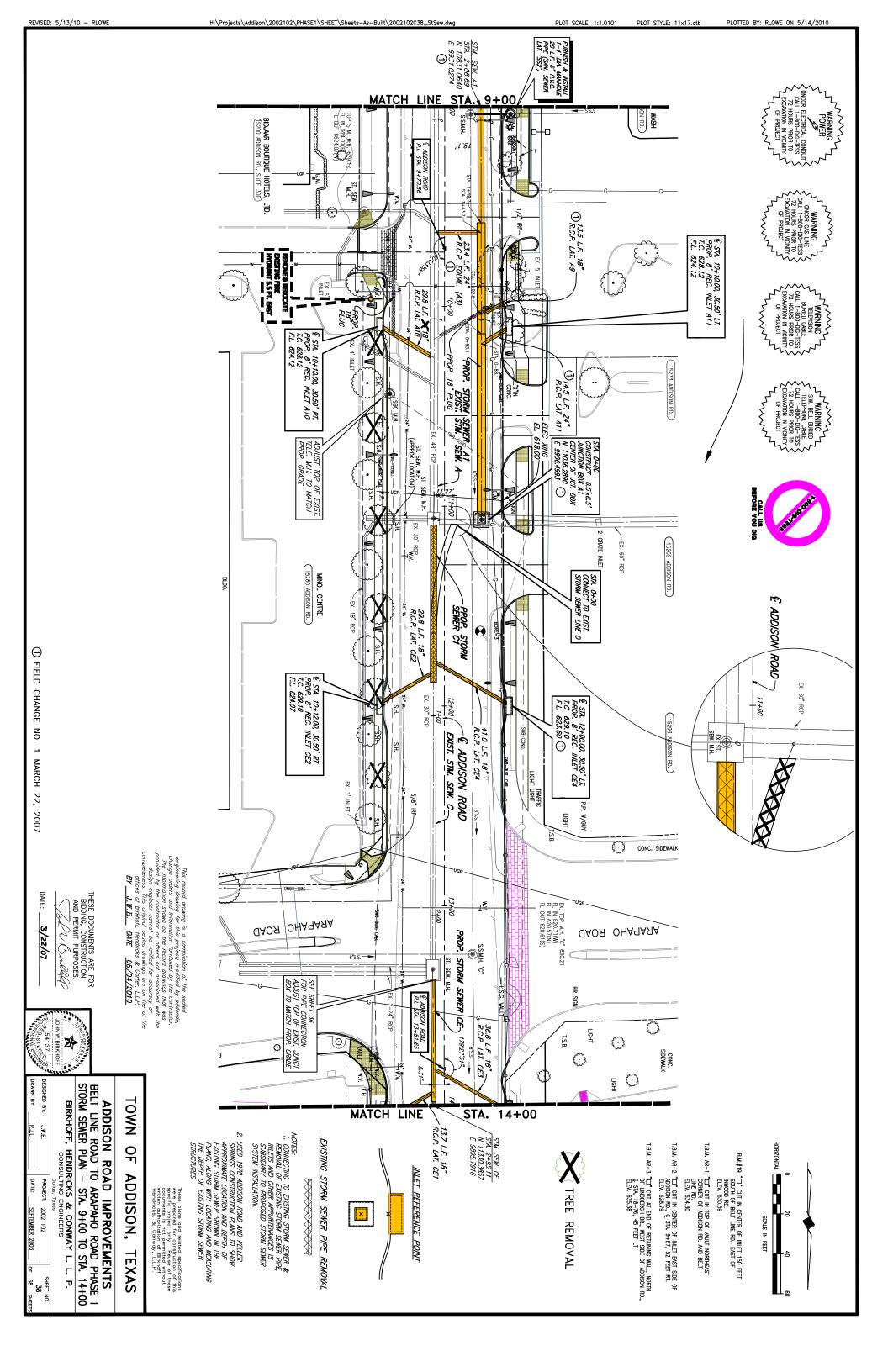
	}				0 1 .										n ee C-11	na
24	22 23	21	20	18	17	16	15	14	13	12	11	10	9	∞	7	6
					INLET CALCULATIONS	TCOT'	T CA	INLE								

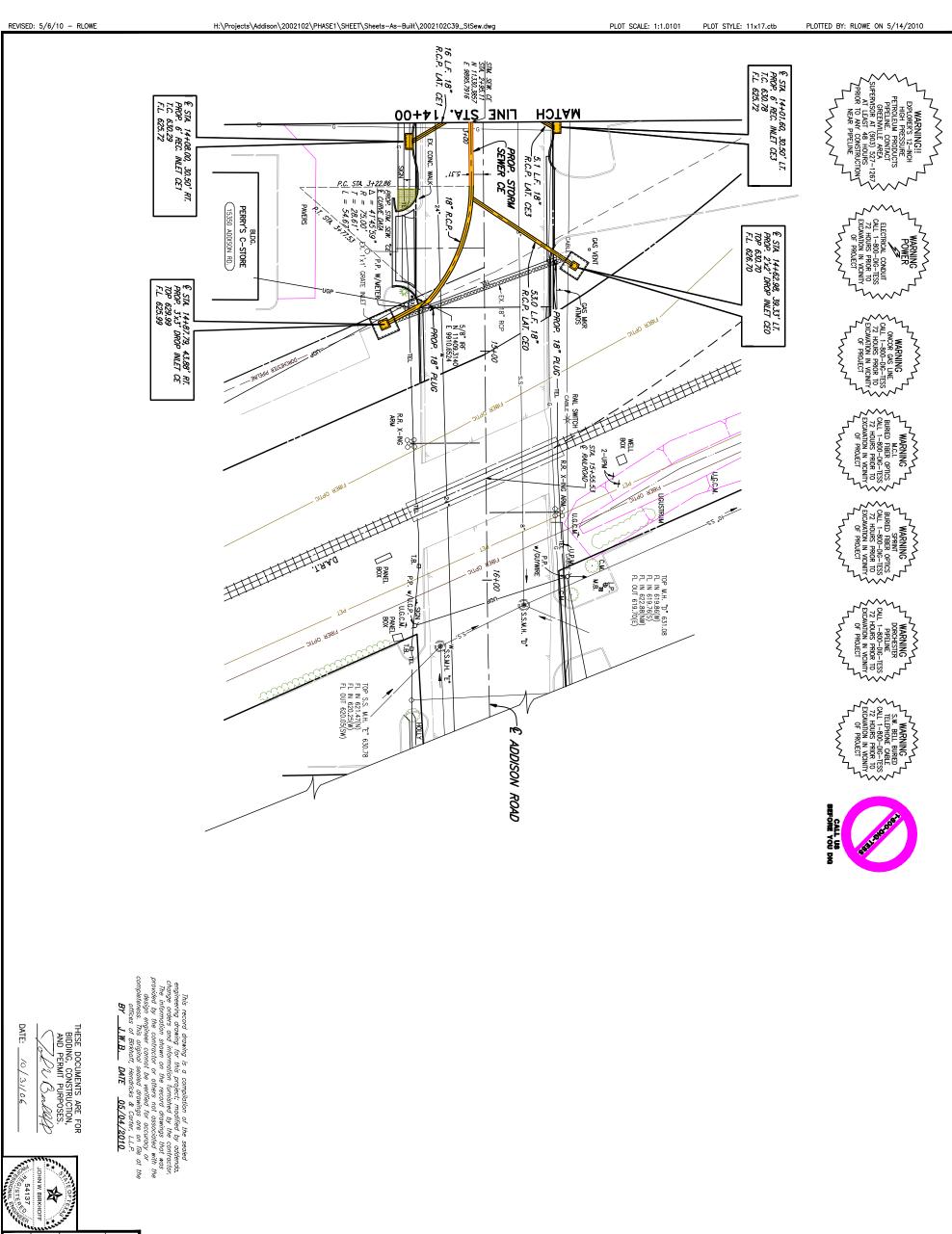
				CE	CE4	CE3	CE2	CE1		A13	A12	A11	A11	A10	A10	A9	Α8	А7В	A7A	A6	A5A	A5	Α4	A3	A2	A1		Number	Inlet			1
				CE	CE4	CE3	CE2	CE1		A13	A12	A11	A11	A10	A10	A9	A8	A7B	A7A	A6	A5A	A5	A4	A3	A2	A1		No.	A 102			2
				14+91.47 - 41.31' RT	12+00 - 30.50' LT.	14+01.60 - 30.50' LT	12+00 - 30.50' RT	14+08 - 30.50' RT		12+57.80 - 41.72' RT	10+16.37 - 41.75' RT	10+10 -30.50' LT.	10+10 -30.50' LT.	10+10 - 30.50' RT.	10+10 - 30.50' RT.	9+91.07 - 42.65' RT	8+55.61 - 30.50 RT	8+35.94 - 30.50' LT	6+05 - 30.50' LT.	7+69.49 - 60.83' RT	5+67.65 - 61.96' RT	5+52.75 - 47.83' RT	5+33.89 - 30.50' RT	2+90.34 - 35.31' LT	2+64.5 - 65.1' RT.	3+28.29 - 29.62' RT		Station	Center I inc	Roadway		ι.
				П 0.26	0.44	0.16	0.15	0.15	_	n 0.37	r 0.33	0.50	0.14	0.20	0.50	0.53	0.74	0.20	0.26	0.64	0.64	5.96	0.17	0.32	1.06	1.37		C=0.90	(Ac)	Paved Area		4
				0.00	0.00	0.00	0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		C=0.90	(Ac.)	Offsite Area		5
				0.90	0.90	0.90	0.90	0.90		0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90		"C"	Coeff	Effective Runoff	Runoff	6
				0.23	0.40	0.14	0.14	0.14		0.33	0.30	0.45	0.13	0.18	0.45	0.48	0.67	0.18	0.23	0.58	0.58	5.36	0.15	0.29	0.95	1.23		"CA"	ental	Increm-	Runoff Calculations	7
				0.23	0.40	0.14	0.14	0.14		0.33	0.30	0.45	0.13	0.18	0.45	0.48	0.67	0.18	0.23	0.58	0.58	5.36	0.15	0.29	0.95	1.23		"CA"	ıılated	Accum-	Suc	∞
				10.00	10.00	10.00	10.00	10.00		10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00		(Min.)	2	Time Upstr.		9
				100	100	100	100	100		100	100	100	100	100	100	100	100	100	100	100	100	100	190	100	100	100		_	_	Design Storm		10
				8.74	8.74	8.74	8.74	8.74		8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74	8.74		(In./Hr.)	€	Intensity		11
				2.05	3.46	1.26	1.18	1.18		2.91	2.60	3.93	1.10	1.57	3.93	4.17	5.82	1.57	2.05	5.03	5.03	46.88	1.34	2.52	8.34	10.78		(c.f.s.)	"	Flow		12
				0.00	0.00	0.00	0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		(c.f.s.)	Over	Carry		13
				2.05	3.46	1.26	1.18	1.18		2.91	2.60	3.93	1.10	1.57	3.93	4.17	5.82	1.57	2.05	5.03	5.03	46.88	1.34	2.52	8.34	10.78		(c.f.s.)	׆֖֖֟	Total Flow		14
				48.0 3	48.0 3	48.0 3	48.0 3	48.0 3		48.0 3	48.0 3	48.0 3	48.0 3	48.0 3	48.0 3	48.0 3	48.0 3	48.0 3	48.0 3	48.0 3	48.0 3	0.0	48.0 3	48.0 3	48.0 3	48.0 3				2	Gutter	15
				3200 0.	3200 0.	3200 0.	3200 0.	3200 0.		3200 0.	3200 0.	3200 0.	3200 0.	3200 0.	3200 0.	3200 0.	3200 0.	3200 0.	3200 0.	3200 0.	3200 0.	0	3200 0.	3200 0.	3200 0.	3200 0.				S C	Capacit	16
	+	_		0.3333	0.0066	0.0066	0.0066	0.0066		0.0050	0.0050	0.0066	0.0066	0.0066	0.0050	0.0050	0.0050	0.0050	0.0050	0.0050	0.0050	0.4417	0.0050	0.0050	0.0050	0.0050		.)		Gutter Slope I	Gutter Capacity Calculations	17
				0.10	0.25	0.17	0.16	0.16		0.24	0.23	0.26	0.16	0.18	0.27	0.28	0.32	0.19	0.21	0.30	0.30	0.00	0.18	0.23	0.36	0.40		(ft.)	Flow in Carl	Y Depth of	tions	18
	-	1		0.75	0.42	0.42 (0.42 (0.42 (0.42	0.42	0.42	0.42 (0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.00	0.42	0.42	0.42	0.42	*	(ft.)	מ			20
	+	1		1.68	1.17	0.98	0.97	0.97		1.17	1.14	1.21	0.95	1.01	1.24	1.26	1.36	1.04	1.09	1.32	1.32	0.00	1.01	1.13	1.49	1.60		ن:	oI R	- K		21
_	1		_	1.21	2.95	1.29	1.22	1.22		2.50	2.28	3.26	1.15	1.55	3.16	3.31	4.27	1.52	1.88	3.83	3.83	0.00	1.33	2.22	5.60	6.76		Lr (ft.)	mired	Minimum I	C	22
				12.00	8.00	6.00	8.00	6.00		3.00	4.00	8.00	8.00	8.00	8.00	5.00	10.00	6.00	6.00	5.00	5.00	2.75	8.00	8.00	8.00	8.00				Length	Ourb Inlet (23
	1	1		9.88	2.72	4.65	6.54	4.91		1.20	1.76	2.45	6.94	5.15	2.53	1.51	2.34	3.95	3,19	1.31	1.31	0.00	6.03	3.60	1.43	1.18			1.7/2.1		alculatio	24
.	$\frac{1}{1}$	+	_	7.75 9	1.71 2	2.49 4	2.56 6	2.56 4		1.73	1.80 1	1.63 2	2.62 6	2.29 5	1.54 2	1.51 1	1.33 2	2.18 3	1.97 3	1.41 1	1.41	0.00 0	2.31 6		1.17 1	1.06			2/Y 0		Calculations (Type I)	25
	+	+		9.88 2	2.72	4.65	6.54	4.91		1.20	1.76 4	2.45	6.94	5.15	2.53	1.51	2.34 1	3.95	3.19	1.31	1.31	0.00	6.03	3.60	1.43	1.18			၇ ဂွ		I)	26
				20.21	9.40	5.85	7.72	5.79		3.50	4.56	9.65	7.64	8.10	9.95	6.30	13.62	6.22	6.53	6.58	6.58	0.00	8.06	9.06	11.92	12.76			ي	0c≡ ——		27
				0.00	0.00	0.00	0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		0.00	0.00	0.00	0.00		0.00	0.00		(c.f.s.)	Carryover			28
				3 ft.x 3 ft. Drop Inlet Low Pt.	8 ft. Recessed Inlet Low Pt.	6 ft. Recessed Inlet On Grade	8 ft. Recessed Inlet Low Pt.	6 ft. Recessed Inlet On Grade		Exist. 3 ft. Standard Inlet Low Pt.	Exist. 4 ft. Standard Inlet Low Pt.	8 ft. Recessed Inlet Low Pt.	Exist. 5 ft. Standard Inlet Low Pt.	10 ft. Recessed Inlet On Grade	6 ft. Recessed Inlet On Grade	6 ft. Recessed Inlet On Grade	Exist. 5 ft. Standard Inlet Low Pt.	Exist, 5 ft. Standard Inlet Low Pt.	16 ft. Standard Inlet Low Pt.	8 ft. Recessed Inlet On Grade	8 ft. Recessed Inlet On Grade	Existing 2'x2' Drop Inlet By Others	8 ft. Recessed Inlet On Grade				Remarks		29			



DESIGNED BY:
DRAWN BY: ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE I
INLET DATA TOWN OF ADDISON, TEXAS







HORIZONTAL 20

SCALE IN FEET

T.B.M. AR-1 "CI" CUT IN TOP OF WAULT NORTHEAST CORNER OF ADDISON RD. AND BELT LINE RD. ELEV. 634.80

B.M.#19 "" CUT IN CENTER OF INLET 150 FEET SOUTH OF BELT LINE RD., EAST OF INWOOD RD. ELEV. 630.59

T.B.M. AR-3 "O" CUT AT END OF RETAINING WALL, NORTH OF LINDBERCH DR., WEST SIDE OF ADDISON RD., Q. STA. 18+76, 45 FEET LT. ELEV. 635.38

T.B.M. AR-2 "''' CUT IN CENTER OF INLET EAST SIDE OF ADDISON RD., © STA 9+87, 52 FEET RT. ELEV. 628.79

TOWN OF ADDISON, TEXAS

PROJECT: 2002 102 SHEET NO. 39

DATE: SEPTEMBER 2006 OF 68 SHEET.

2. USED 1978 ADDISON ROAD AND KELLER SPRINGS CONSTRUCTION PLANS TO SHOW APPROXIMATE LOCATION AND DEPTH OF EXISTING STORM SEWER SHOWN IN THE PLANS, AUGUS WITH LOCATION AND MEASURING THE DEPTH OF EXISTING STORM SEWER STRUCTURES.

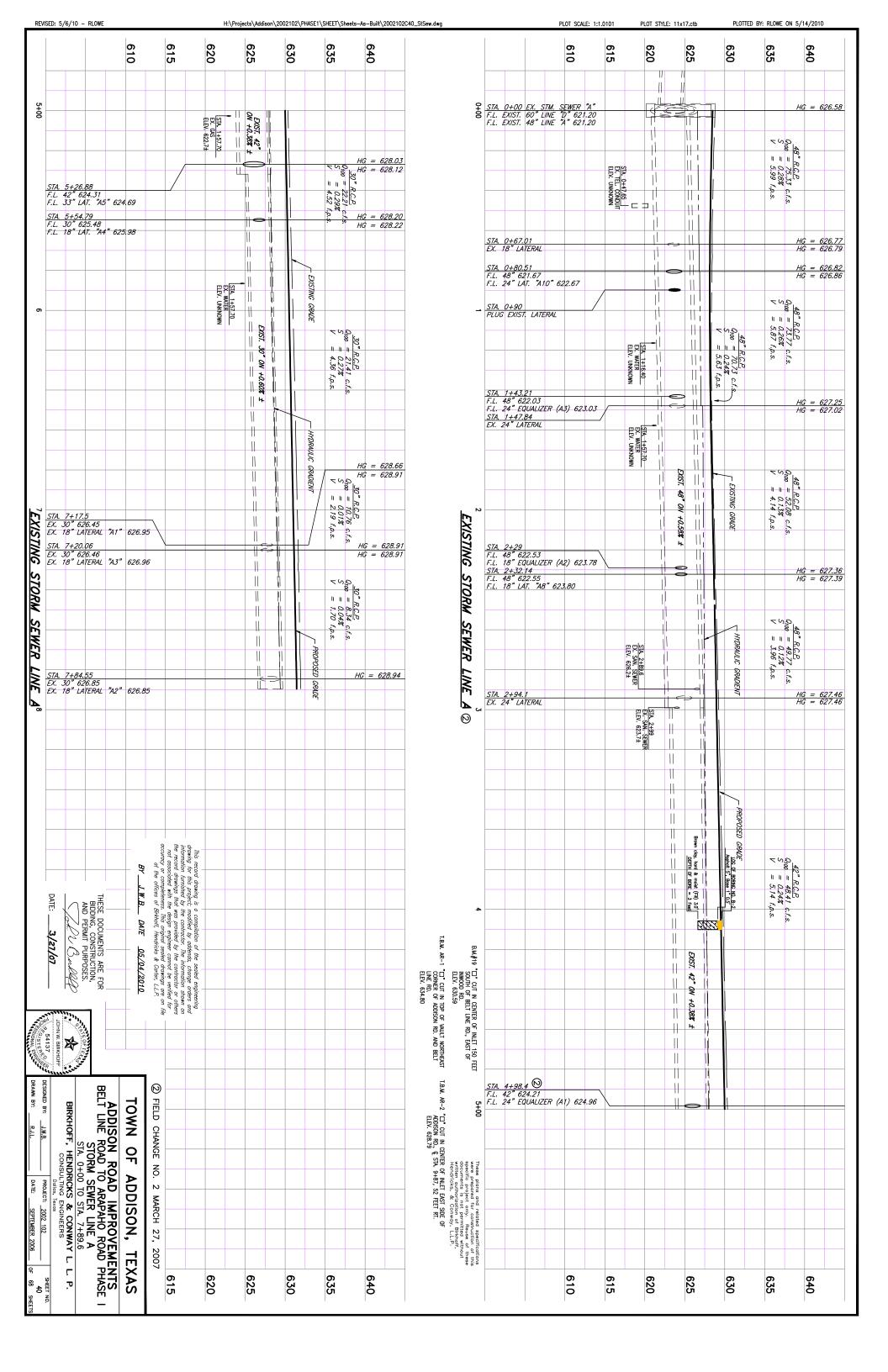
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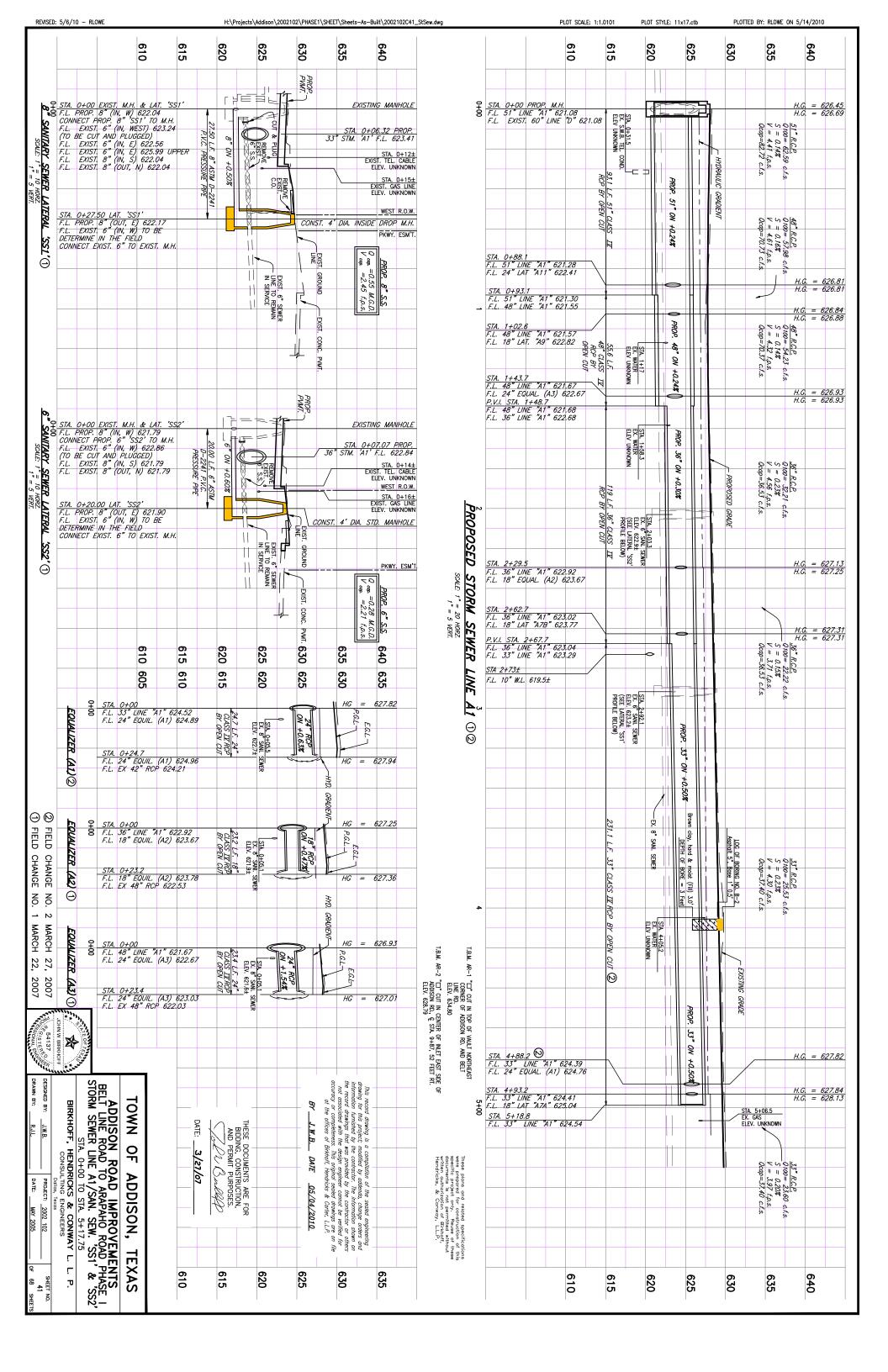
1. CONNECTING TO EXISTING STORM SEMER & REMOJAL OF EXISTING STORM SEMER PIPE, INLETS AND OTHER APPURITEMANCES IS SUBSIDIARY TO PROPOSED STORM SEMER SYSTEM INSTALLATION.

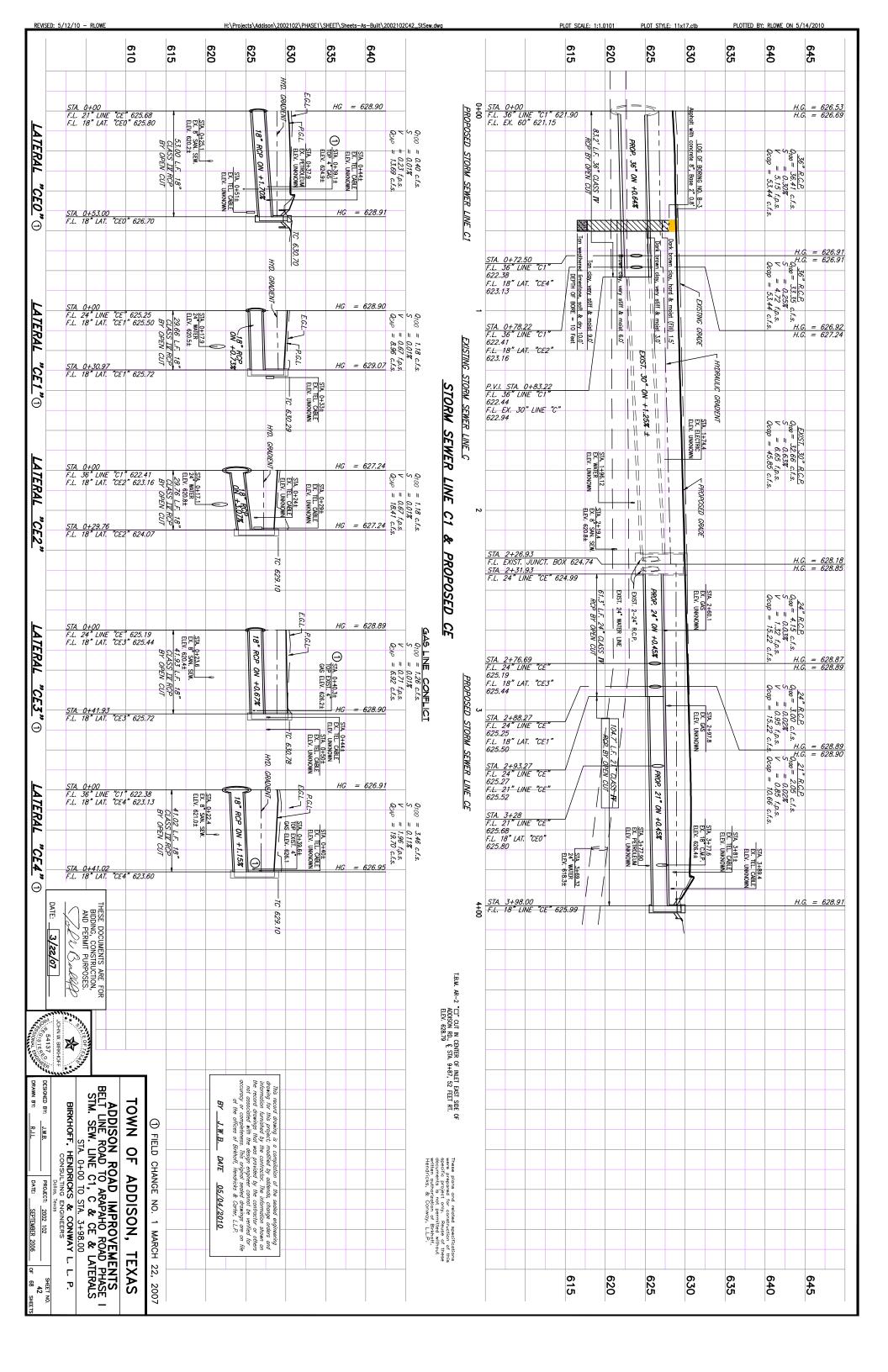
EXISTING STORM SEWER PIPE REMOVAL

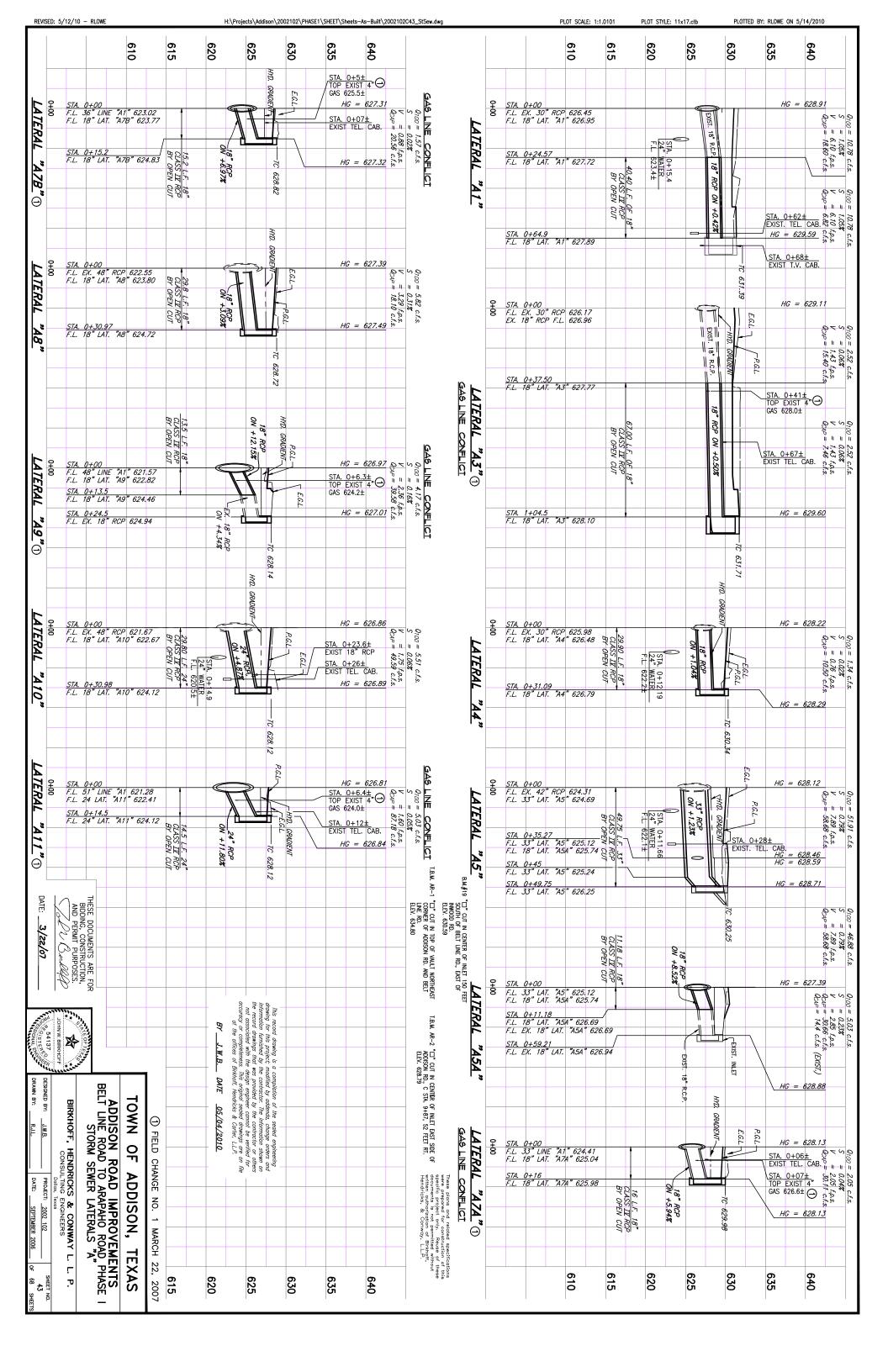
INLET REFERENCE POINT

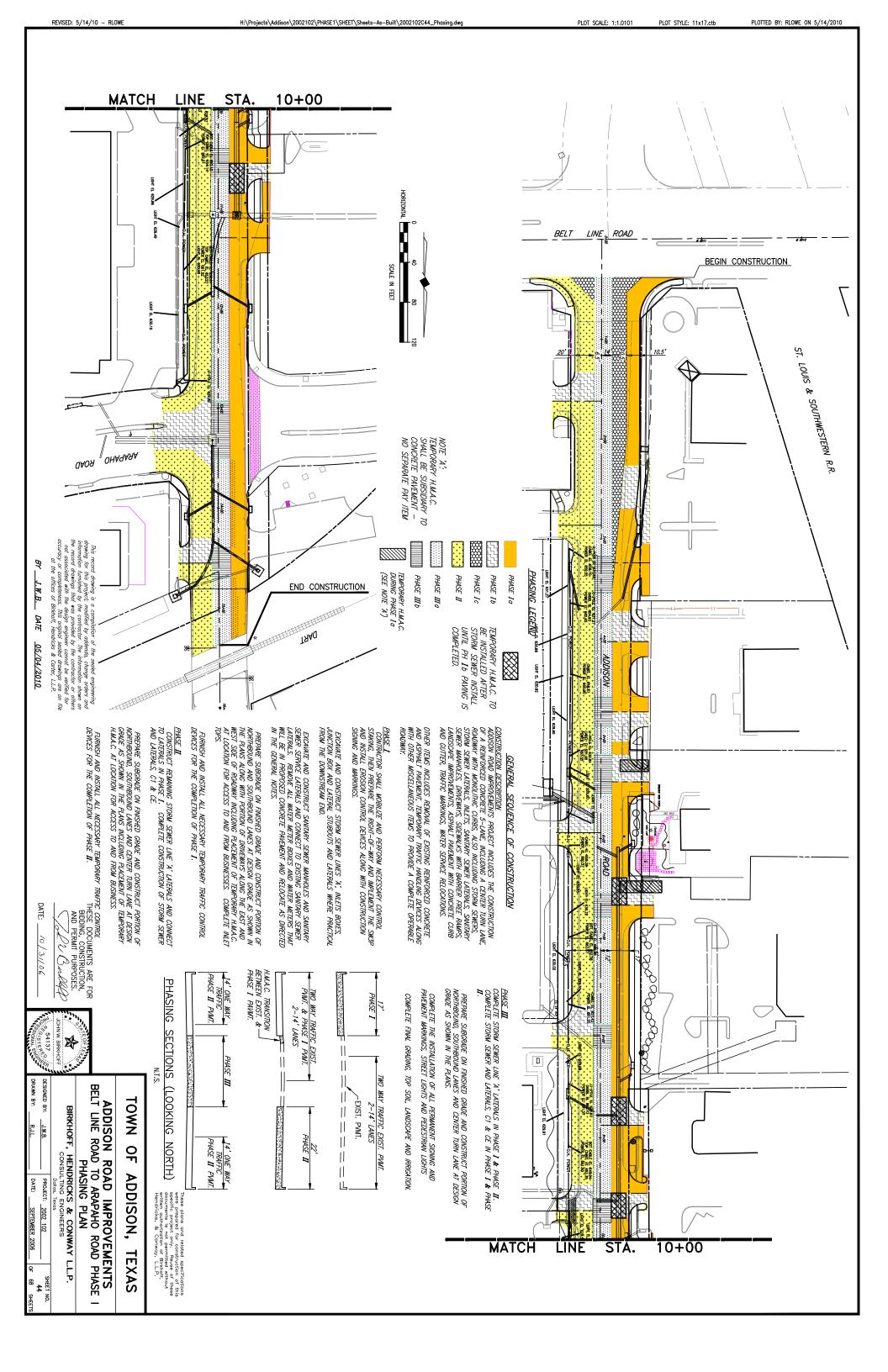
ADDISON ROAD IMPROVEMENTS
ARAPAHO ROAD TO ARAPAHO ROAD PHASE I
STORM SEWER PLAN — STA. 14+00 TO STA. 15+37 DESIGNED BY: J.W.B. DRAWN BY: R.J.L. BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS
Dollos, Texas

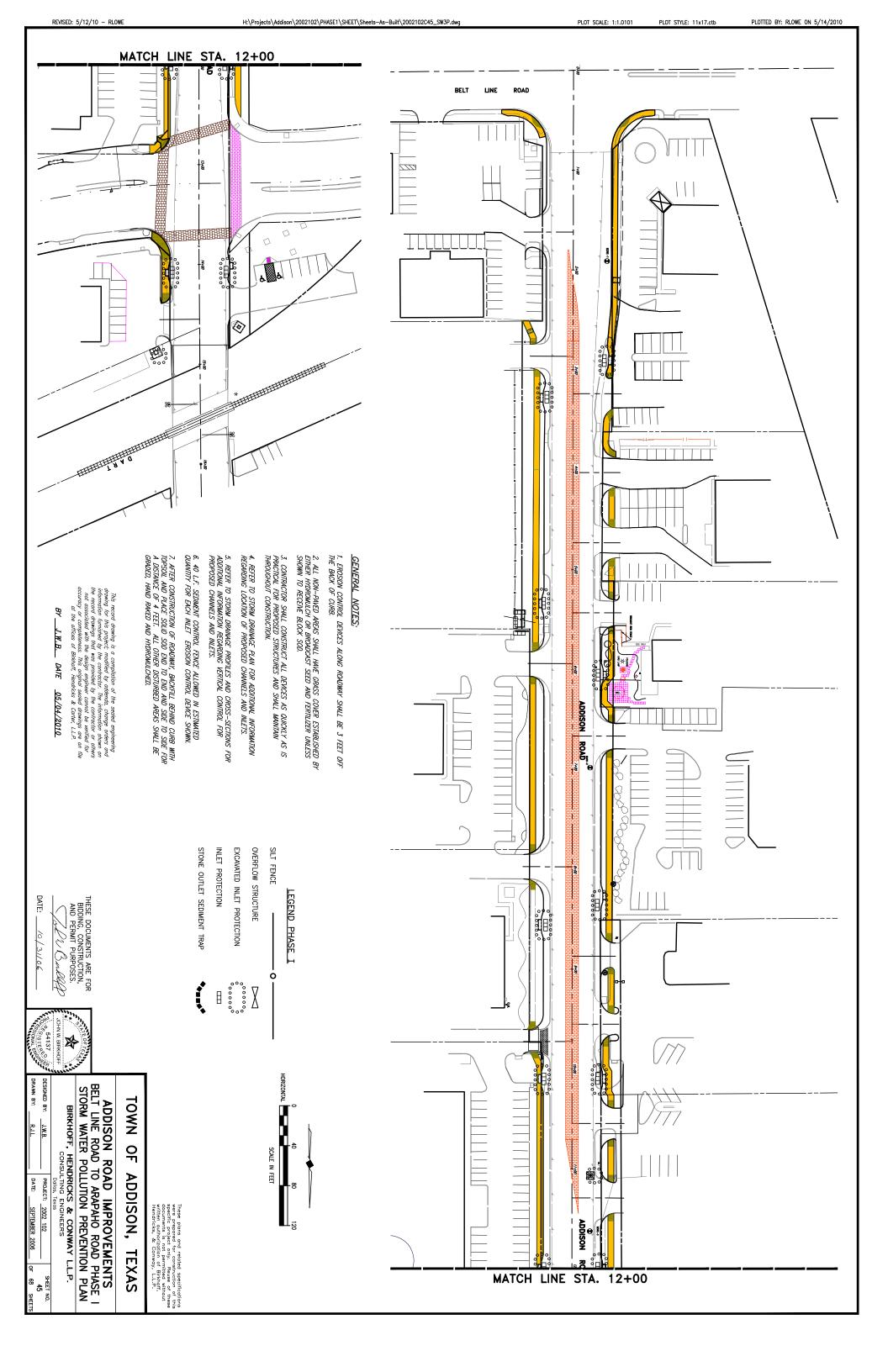












ROLS

ent controls:

SEDIMENT CONTROLS WILL BE MAINTAINED IN GOOD WORKING ORDER.
Y, IT WILL BE DONE AT THE EARLIEST DATE POSSIBLE, BUT NO LATER
IFTER THE SURROUNDING EXPOSED GROUND HAS DRIED SUFFICIENTLY
MAGE FROM HEAVY EQUIPMENT. THE AREAS ADJACENT TO CREEKS
L HAVE PRIORITY FOLLOWED BY DEVICES PROTECTING STORM SEWER INLETS.

* PERFORMED BY A CONIRACTOR APPROVED INSPECTOR IN ACCORDANCE S REQUIREMENTS. AN INSPECTION AND MAINTENANCE REPORT ECTION. BASED ON THE INSPECTION RESULTS, THE CONTROLS ISPECTION REPORT. CITY SHALL REVIEW ALL CONTRACTOR INSPECTIONS. ROSION CONTROL DEVICES EVERY 14 CALENDAR DAYS.

ATERIALS WILL BE COLLECTED AND STORED IN A SECURELY LIDDED METAL WILL MEET ALL STATE AND LOCAL CITY SOLID WASTE MANAGEMENT NO CONSTRUCTION DEBRIS FROM THE SITE WILL BE DEPOSITED IN STER WILL BE EMPTIED AS NECESSARY OR AS REQUIRED BY HE TRASH WILL BE HAULED TO A LOCAL LAMBFILL.

MATERIAL WILL BE BURIED ON SITE.

REPORTING):

AT A MINIMUM, ANY PRODUCTS IN THE FOLLOWING
D TO BE HAZARDOUS: PAINTS, ACIDS FOR CLEANING MASONRY
VITS, ASPHALT PRODUCTS, CHEMICAL ADDITIVES FOR SOIL STABILIZATION
TOUNDS AND ADDITIVES, IN THE EVENT OF A SPILL WHICH MAY BE
THE SPILL COORDINATOR SHOULD BE CONTACTED IMMEDIATELY.

WILL BE E COLLECTED FROM THE PORTABLE UNITS AS NECESSARY BY A LICENSED SANTARY WASTE MANAGEMENT CONTRAC

LL BE RESPONSIBLE FOR AUGMENTING THESE PLANS WITH OTHER EMPORARY EROSION CONTROL MEASURES OCCASIONED BY THE WORK AND BORROW PIT ACCESS. ALL CONTINGENT EROSION CONTROL BY THE ENGINEER PRIOR TO INSTALLATION OR CONSTRUCTION.

MANNER THAT

OCKPILES, AND HAUL ROADS STALL DE COLOR RECEIND THE AMOUNT OF SEDIMENT THAT MAY ENTER RECEIND THE AMOUNT OF SEDIMENT THAT MAY ENTER BODY OR ST.

NOT BE LOCATED IN ANY WETLAND, WATER BODY OR ST.

NIC AREAS AND VEHICLE MAINTENANCE AREAS SHALL BE A MAINNER TO MINIMIZE THE RUNOFF OF POLLUTANTS. BE CLEARED AS SOOW AS PRACTICAB ING, FALSE WORK, PILING, DEBRIS OR FRATIONS THAT ARE NOT A PART OF T NDANGERED SPECIES IMPACTED BY TH RECEIVING WATERS.
RESTREAM BED
BE CONSTRUCTED WORK. EMBANKMENTS, DNS PLACED . THERE ARE



54137 O. B. DESIGNED BY:

DRAWN BY:

DATE:

SEPTEMBER 2006

TOWN OF ADDISON, TEXAS

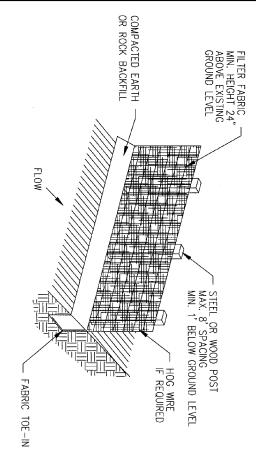
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ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
STORM WATER POLLUTION PREVENTION PLAN DETAILS

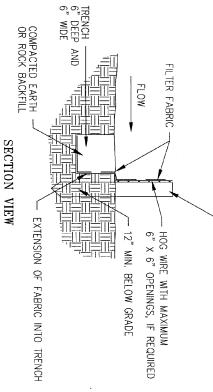
BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS J.W.B. PROJECT: 2002 102

SILT FENCE

N.T.S.







FILTER EACH SIDE 8 MAX

STONE OVERFLOW STRUCTURE

file

DATE 05/04/2010

SILT FENCE

5.5

- embedded in the soil. At installations draining larger areas the filter fabric will be attached between and attached to metal or wooden posts, with the bottom of the fabric firmly DESCRIPTION - A temporary sediment barrier consisting of filter fabric stretched to a hog wire support that is attached to the fence posts.
- sedimentation to occur and to filter out larger sediment particles as the water flows through the filter fabric. PURPOSE - To slow the flow of sediment laden water from small disturbed areas to allow
- drainage areas, but does not function as well as a normal diversion channel and is usually downstream of small disturbed areas. It can also be used as a flow diversion for very small much more expensive. APPLICATIONS - Silt fence is normally used as a perimeter control immediately
- access so the locations should be well planned to prevent obstructions. Water will pond or areas of concentrated water flow. Silt fence locations can limit construction vehicle behind the silt fence resulting in localized flooding during major rain events. LIMITATIONS - Do not install silt fences across channels, ditches, streams, pipe outlets,
- structures or other outlet device shall be installed at all low points along the fence or every 300 feet if there is no apparent low point. Provide hog wire support backing whenever the drainage area exceeds 0.10 acres per 100 lineal feet of fence. Maximum post spacing shall not exceed 8 feet. Stone overflow runoff must be controlled. Limit the drainage area to 0.25 acres per 100 lineal feet of fence. DESIGN CRITERIA - Place silt fence along perimeter of site where disturbed area sheet

'n

MATERIAL SPECIFICATIONS -

6

- Filter Fabric NCTCOG Specification 2.23.4.
- Washed Stone NCTCOG Specification 2.1.8.(e).
- Hog Wire NCTCOG Specification 2.8.2.(b)(1).
- Fence Posts NCTCOG Specification 2.8.2.(b) for wood or steel

.7

MAINTENANCE REQUIREMENTS - Silt fence should be inspected weekly and after above grade. Inspect the base of the fence to ensure that no gaps have developed and major rain events to ensure that the device is functioning properly. Remove sediment from deteriorated, replace it in accordance with the original specifications and details. behind fence when the depth of sediment has built up to one-third the height of the fence re-trench as necessary. Inspect fence posts to ensure that they are properly supporting the ence. Straighten, reset and add posts if necessary. If filter fabric is ripped, damaged or

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TOWN N 유 ADDISON, TEXAS

BELT LINE ROAD TO ARAPAHO ROAD PHASE STORM WATER POLLUTION PREVENTION PLAN DETAILS ADDISON ROAD IMPROVEMENTS

BIRKHOFF, HENDRICKS & CONWAY L. L. P.
CONSULTING ENGINEERS
Dollos. Texas

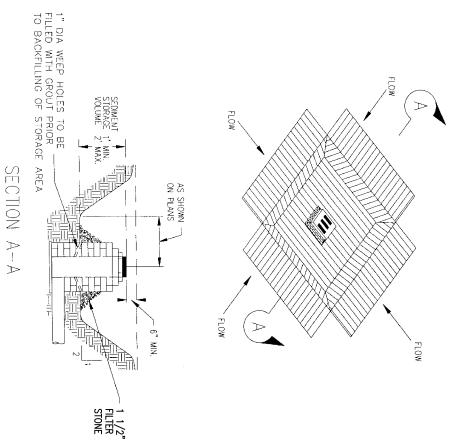
ΤJH J.W.B. 2002 102

X DRAWN BY: DESIGNED BY:

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DATE:

DATE: SEPTEMBER 2006



EXCAVATED DROP INLET PROTECTION

 $\label{lem:haddison2002102\PHASE1\SHEET\Sheets-As-Built\2002102C48_SW3P.dwg} H:\Projects\Addison\2002102\PHASE1\SHEET\Sheets-As-Built\2002102C48_SW3P.dwg$

EXCAVATED DROP INLET PROTECTION

5.7

- **DESCRIPTION** An excavated area surrounding a storm drain drop inlet.
- PURPOSE To remove sediment from storm runoff before it enters into the storm drain

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- relatively heavy flows and overflow capability is needed. Weep holes or other methods of de-watering the storage area must be provided. This method can also be used in conjunction with other drop inlet protection methods (block and gravel or silt fence barriers) to provide **APPLICATIONS** - Where storm drain drop inlets are to be used prior to final stabilization of the area draining to the structure. This method is used where the inlet will collect more efficient sediment removal.
- paved area. drop inlet is to be paved. This method is not applicable LIMITATIONS - Ponding will occur around the inlet with possible localized flooding as the result. Final stabilization and cleanout may be difficult if the finished area around the to use around existing inlets in a

DESIGN CRITERIA -

'n

- Drainage Area 5.0 acres or less.
- Depth 1 foot minimum, 2 feet maximum, measured from crest of inlet.
- Volume 1800 cubic feet per acre of drainage area.
- Side Slopes 2H:1V or flatter.
- De-watering Time 48 hours or less. Size and number of holes as needed

6 **MATERIAL SPECIFICATIONS -**

Filter Stone - NCTCOG Specification 2.1.8.(e)

.7

MAINTENANCE REQUIREMENTS - Excavated drop inlet protection should be inspected weekly and after major rain events to ensure that the device is functioning properly. Remove sediment from the excavation when the depth of sediment has built up to replace filter stone surrounding weep holes. Clean the stone surface the first few times by one-half of the design depth. If de-watering of storage raking. Repeated sediment build-up will require filter stone replacement. volume is not occurring, clean or

BY J.W.B. DATE 05/04/2010

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10/31/06



TOWN OF ADDISON, TEXAS

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ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
STORM WATER POLLUTION PREVENTION PLAN DETAILS

BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

PROJECT: 2002 102

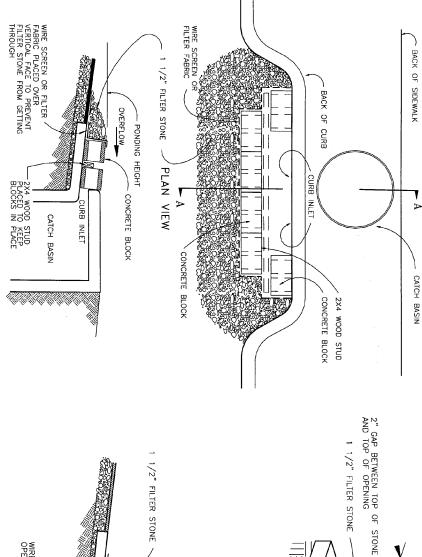
DRAWN BY: DESIGNED BY: J.W.B. DATE: SEPTEMBER 2006

1 1/2" FILTER STONE -

CURB INLET

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5



SECTION A -

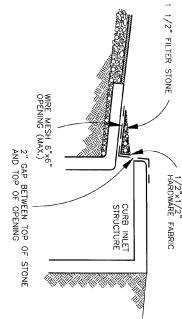
BLOCK AND GRAVEL RECESSED

CURB INLET PROTECTION

 $\label{lem:hamilton} \begin{tabular}{ll} H:\Projects\Addison\2002102\PHASE1\SHEET\Sheets-As-Built\2002102C49_SW3P.dwg \\ \end{tabular}$



ISOMETRIC PLAN VIEW



INLET SECTION

INLET PROTECTION TYPE A CURB

s and wn on others file

J.W.B. DATE 05/04/2010

CURB INLET PROTECTION

5.10

EXTEND MESH, FABRIC AND FILTER STONE 12" BEYOND END OF INLET ON BOTH ENDS.

12"MIN. OVERLAP

OPENING MAX. (MAX.)

1/2"x1/2" HARDWARE HABRIC

- concrete block, welded wire fabric, hardware fabric or 2x4 studs surrounding a storm drain DESCRIPTION - A temporary sediment control barrier made of filter stone and standard curb inlet.
- PURPOSE To remove sediment from storm runoff before it enters into the storm drain system.
- APPLICATIONS Where storm drain curb inlets are to be used prior to final stabilization of the area draining to the structure. This method is used where the inlet will collect protect existing curb inlets located in paved areas. relatively heavy flows and overflow capability is needed. This method is also to be used to
- placed at each end of the inlet protection device. the result. When used at locations that are open to vehicle traffic, this device will encroach LIMITATIONS - Ponding wil into the traveled way. If the curb inlet is not a recessed type inlet a traffic barricade shall be l occur around the inlet with possible localized flooding as

DESIGN CRITERIA -

'n

- Drainage Area 2.0 acres or less. Height 6" (Maximum).

MATERIAL SPECIFICATIONS -

6

- Concrete Block ASTM C 139, Concrete Masonry Unit for Construction.
- Filter Stone NCTCOG Specification 2.1.8.(e).

Wire Fabric - Standard galvanized hardware fabric with 1/2" by 1/2" openings.

- Wire Mesh Welded wire fabric conforming to NCTCOG Specification 2.2.7 maximum opening 6"x6".
- sediment from the storage area when the depth of sediment has built up to one-half of the storage depth. If de-watering of the storage volume is not occurring, clean or replace the and after major rain events to MAINTENANCE REQUIREMENTS - Curb inlet protection should be inspected weekly build-up will require filter stone filter stone. Clean the filter stone surface the first few times by raking. Repeated sediment ensure that the device is functioning properly. Remove replacement.

7.

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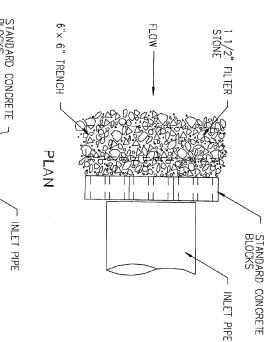
TOWN OF ADDISON, TEXAS

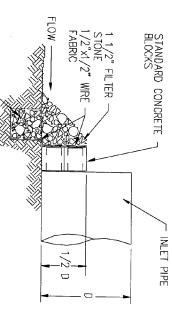
BELT LINE ROAD TO ARAPAHO ROAD PHASE STORM WATER POLLUTION PREVENTION PLAN DETAILS ADDISON ROAD IMPROVEMENTS

BIRKHOFF, HENDRICKS & CONWAY L. L. CONSULTING ENGINEERS SEPTEMBER 2006

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PROFILE

6"x6" TRENCH

 $\label{lem:hamilton} H:\Projects\Addison\2002102\PHASE1\SHEET\Sheets-As-Built\2002102C50_SW3P.dwg. Addison\PHASE1\SHEET\Sheets-As-Built\2002102C50_SW3P.dwg. Addison\PHASE1\Sheets-As-Built\2002102C50_SW3P.dwg. Addison\PHASE1\$

CINDER BLOCK PIPE INLET PROTECTION

PIPE INLET PROTECTION

- DESCRIPTION A temporary sediment control barrier made of standard concrete block pipe or inlet pipe headwall. and filter stone or stone rip-rap and filter stone surrounding the inlet end of a storm drain
- PURPOSE To remove sediment from storm runoff before it enters into the storm drain
- the pipe inlet will collect relatively heavy stormwater flows and overflow capability is APPLICATIONS - Where existing or proposed storm drain pipes or culverts are to be used prior to final stabilization of the area draining to the pipe inlet. This method is used where

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difficult and may create a separate erosion problem if not properly constructed. Do not use Cinder Block Pipe Inlet Protection for pipes larger than 36" in diameter. as the result. Excavation of a sediment storage area can make final channel stabilization LIMITATIONS - Ponding will occur around the pipe inlet with possible localized flooding

DESIGN CRITERIA-

'n

- Volume 1800 cubic feet per acre of drainage area.
- Side Slopes 1.5H:1V or flatter.
- Top of Stone and Sediment Storage 1/2 of inlet pipe diameter.

MATERIAL SPECIFICATIONS -

6

- Concrete Block ASTM C 139, Concrete Masonry Unit for Construction.
- Wire Fabric Standard galvanized hardware fabric with 1/2" by 1/2" openings.
- Filter Stone NCTCOG Specification 2.1.8.(e).
- Stone Rip-Rap NCTCOG Specification 2.1.6.(b)(2), Type A.

.7

and after major rain events to ensure that the device is functioning properly. Remove the filter stone surrounding the pipe inlet. Clean the stone surface the first few times by of the design depth. If de-watering of the storage vol sediment from the sediment storage area when the depth of sediment has built up to one-half MAINTENANCE REQUIREMENTS - Pipe inlet protection should be inspected weekly raking. Repeated sediment build-up will require filter stone replacement. ume is not occurring, clean or replace

8 J.W.B.DATE 05/04/2010

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DATE:

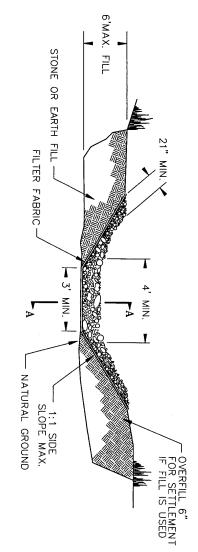


TOWN OF ADDISON, TEXAS

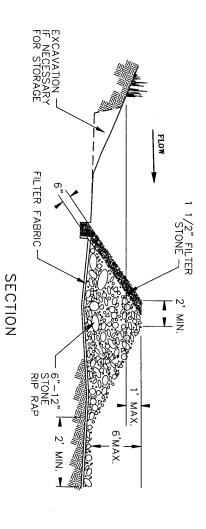
ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
STORM WATER POLLUTION PREVENTION PLAN DETAILS

BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

2002 102



VIEW LOOKING UPSTREAM



STONE OUTLET SEDIMENT TRAP

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5.13 STONE OUTLET SEDIMENT TRAP

- across a drainageway or swale. The ponding area may be natural or improved to provide the required storage volume. DESCRIPTION - A ponding area formed by placing an earth and/or stone embankment
- PURPOSE To detain sediment laden runoff long enough to allow sediment to settle from the water and to allow diffused runoff from the outlet. the majority of the
- construction of the embankment to form the ponding area. stone outlet sediment trap can be used instead of the standard sediment basin. APPLICATIONS - Normally used where the natural topograp adequate space for the storage area and will accept the runoff from the disturbed site. The improvements, and localized grading will allow placement in almost any location that has Diversions, drainage hy allows for the

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settling time in the basin. LIMITATIONS - Do not place device in a live stream. Avoid placing located as far away from the stone outlet as is practicable to allow for maximum sediment future improvements such as pavement or buildings. Inlet channels or pipe should be in areas planned for

'n DESIGN CRITERIA -

- Drainage Area 30 acres, maximum.
- Б. Б. Б. Storage Volume - 1800 cubic feet per acre of disturbed land draining to the device.
 - Surface Area of Storage Area 1% (0.01) of the area draining to the device.
- Embankment Height 6 feet (maximum) to top.
- Embankment Top Width 2 feet (minimum) Embankment Slopes - 1.5H:1V or flatter.
- Stone Outlet Width 4 feet (minimum)
- Outlet Capacity 10-year design storm, C = 0.50, Max. Tc = 15 minutes.
- Freeboard @ Design Flow 6 inches.

MATERIAL SPECIFICATIONS -

9

- Stone Rip-Rap NCTCOG Specification 2.1.6.(b)(2), Type A or re-cycled concrete meeting the same gradation.
- Filter Stone NCTCOG Specification 2.1.8.(e).
- Filter Fabric NCTCOG Specification 2.23.4.

.7

rain event. The filter stone on the upstream face of the stone outlet may downstream flow channel to ensure that no excessive erosion or gullies weekly and after major rain events to ensure that the device is functioning properly. Remove sediment from the storage area when the depth of sediment has built up to one-half MAINTENANCE REQUIREMENTS - Stone outlet sediment traps should be inspected replacement if standing water remains for longer periods. repair as necessary. The sediment storage area should drain within 48 Remove sediment from the storage area when the depth of sediment has built up to one-half the height of the stone outlet. Inspect the downstream base of the stone outlet and the have developed and to 72 hours after a require cleaning or

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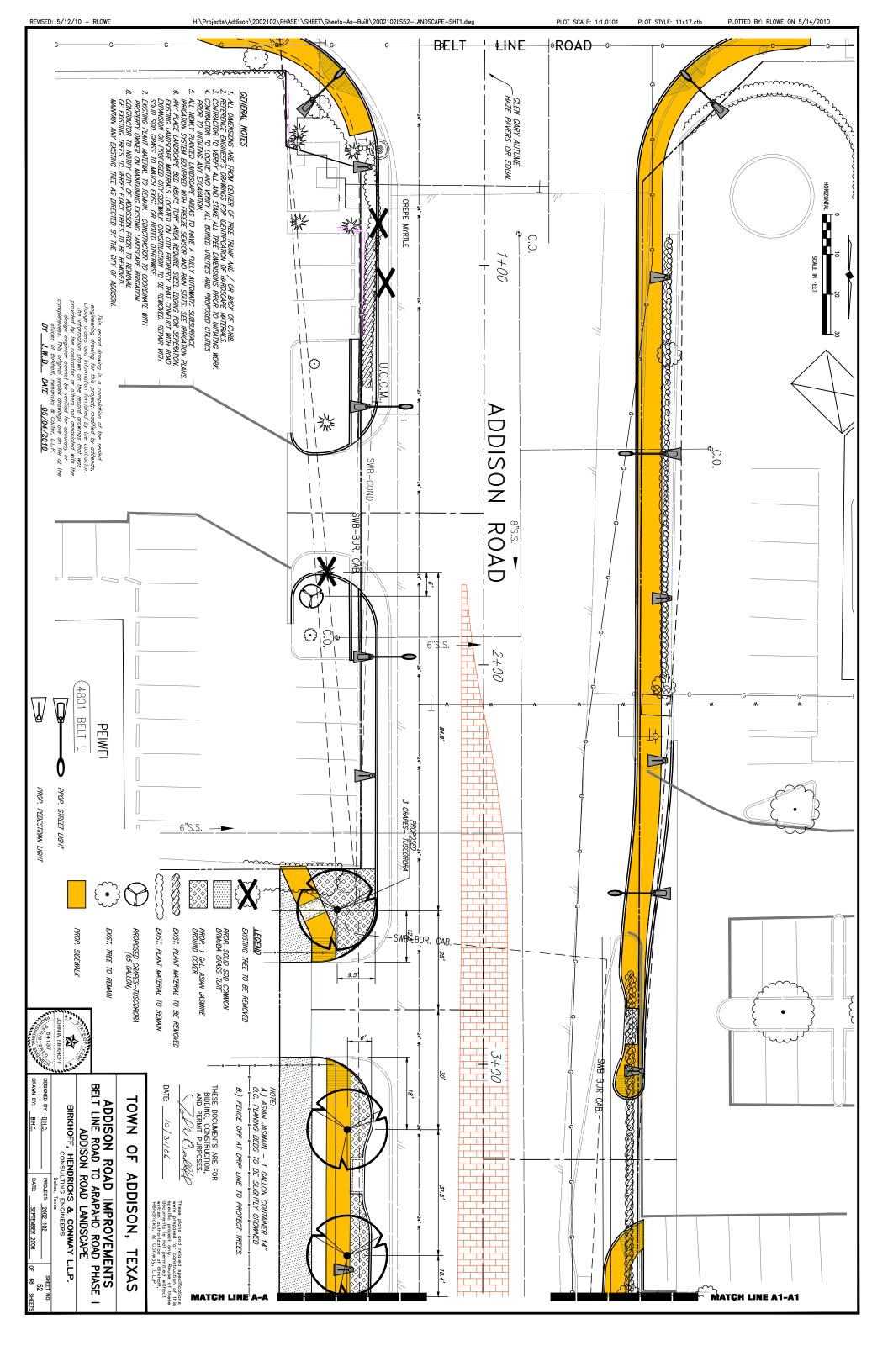
TOWN OF ADDISON, TEXAS

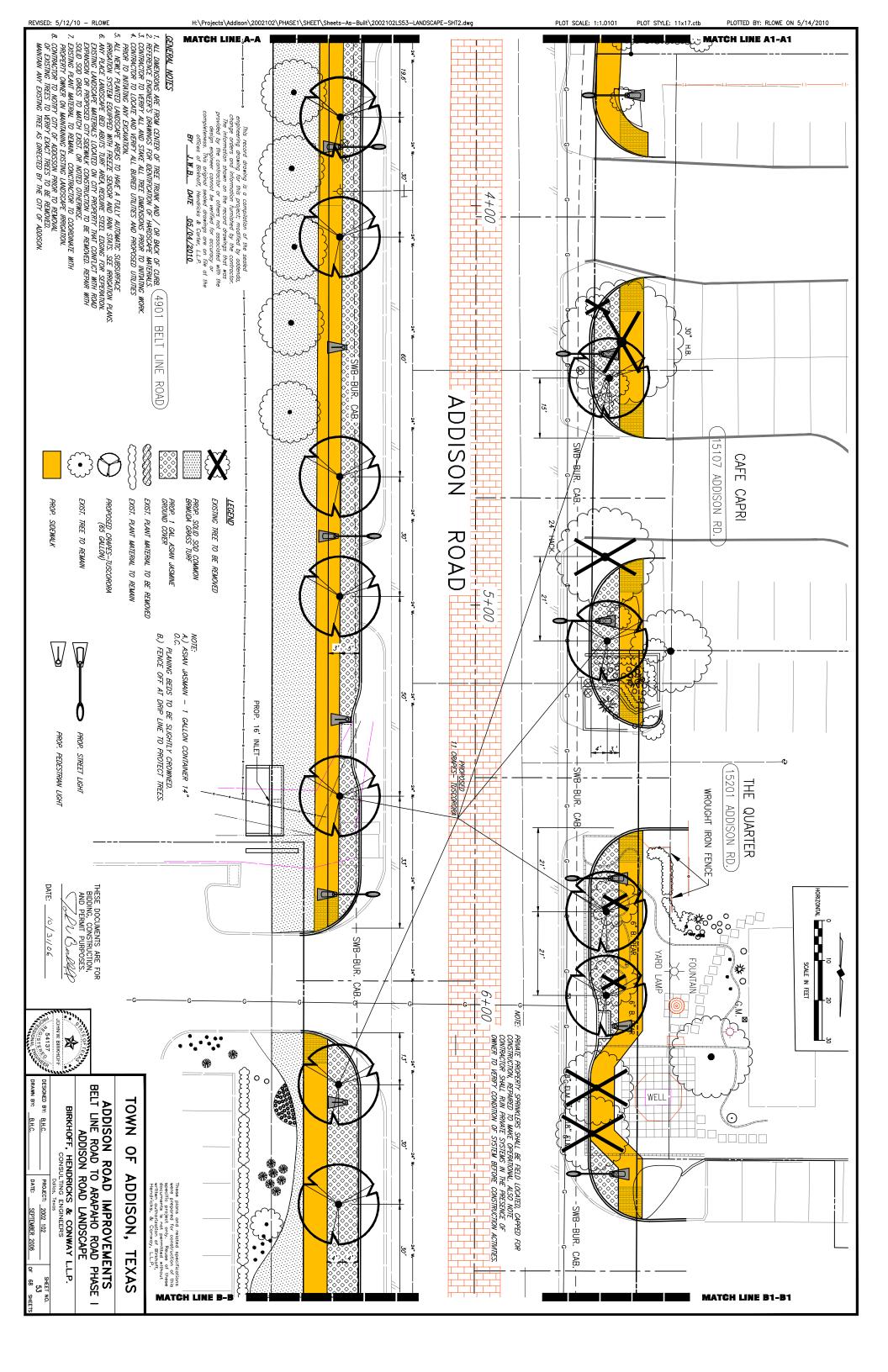
ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
STORM WATER POLLUTION PREVENTION PLAN DETAILS

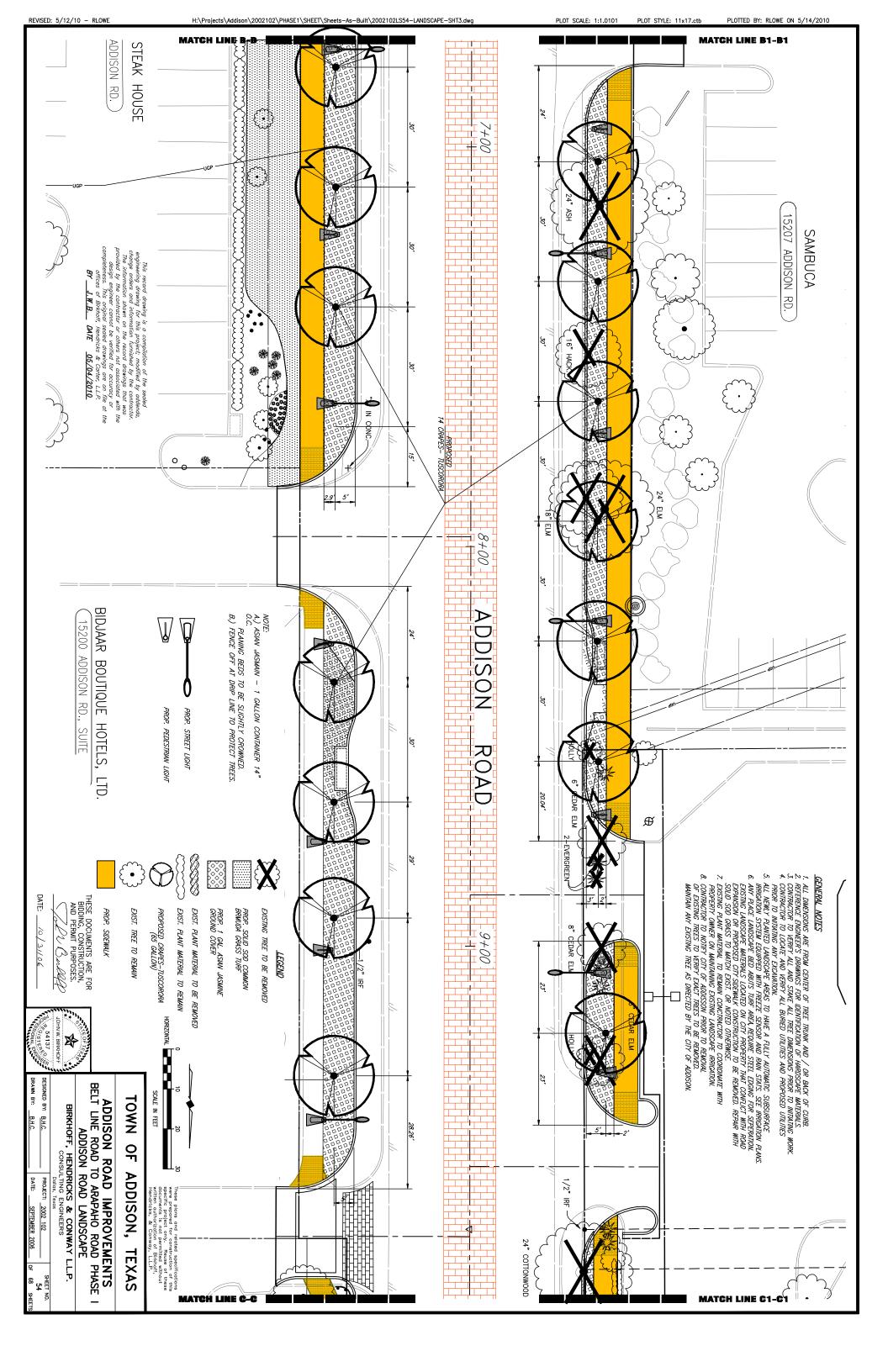
BIRKHOFF, HENDRICKS & CONWAY L. L. P. CONSULTING ENGINEERS

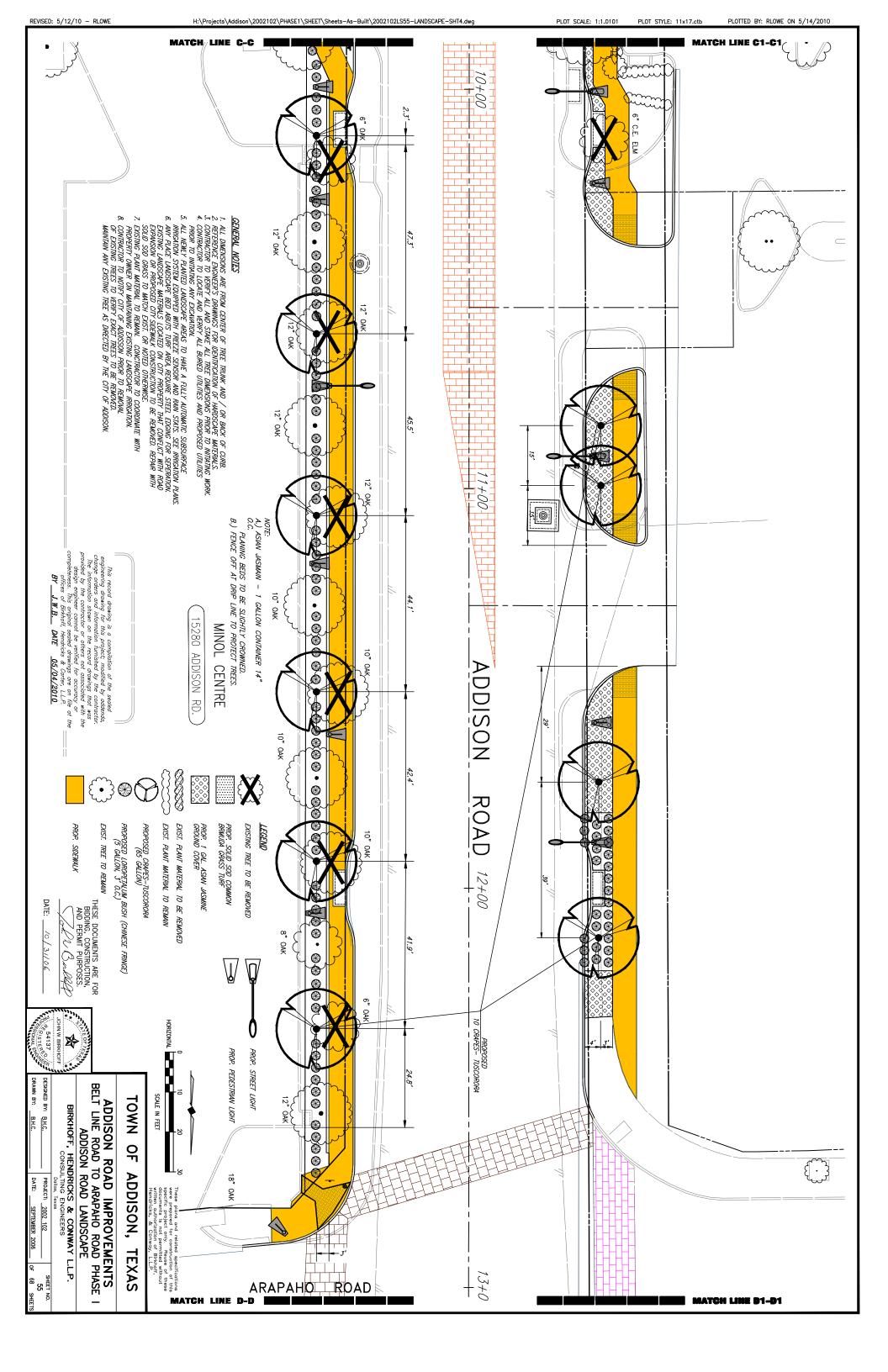
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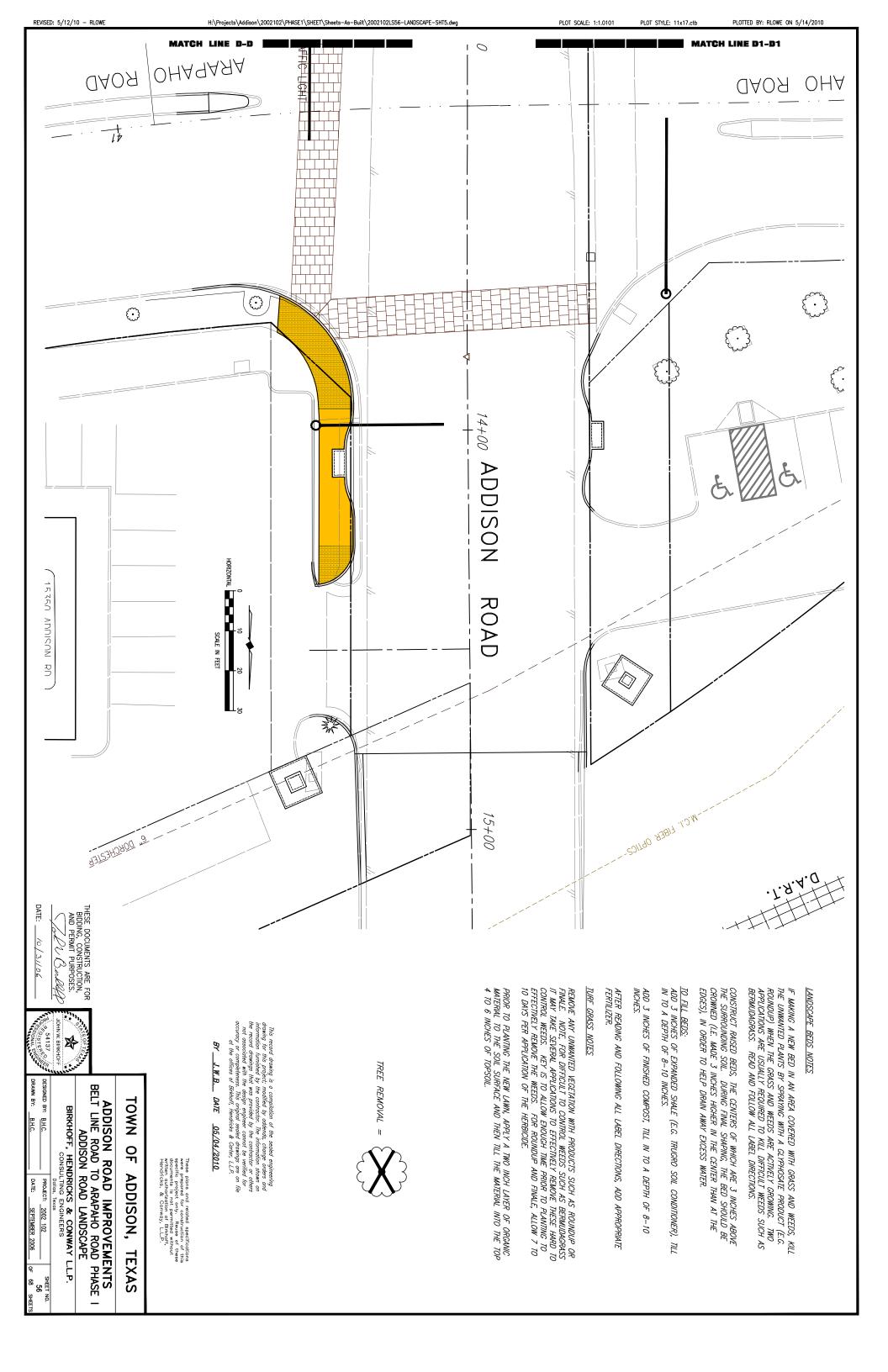
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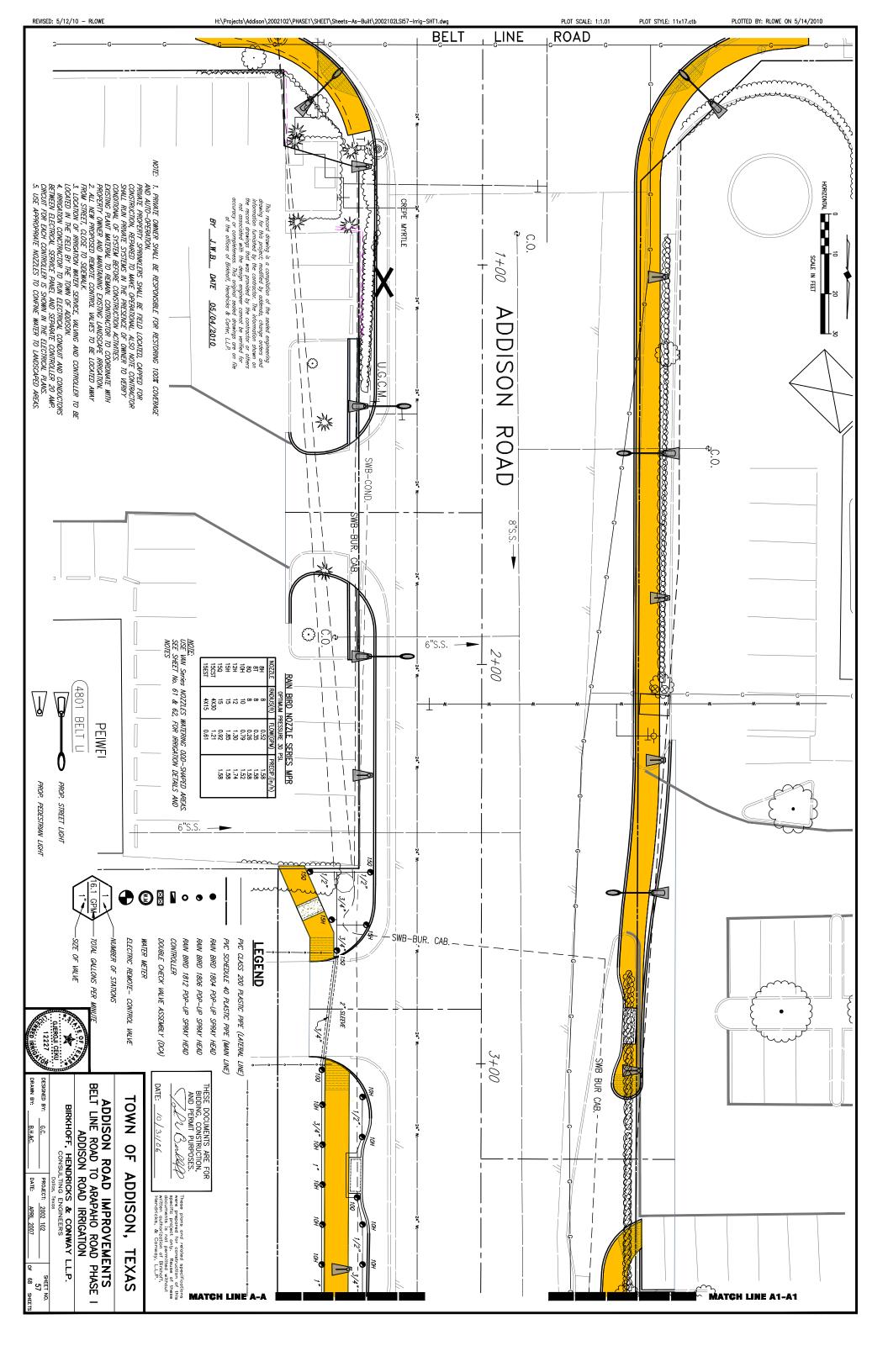


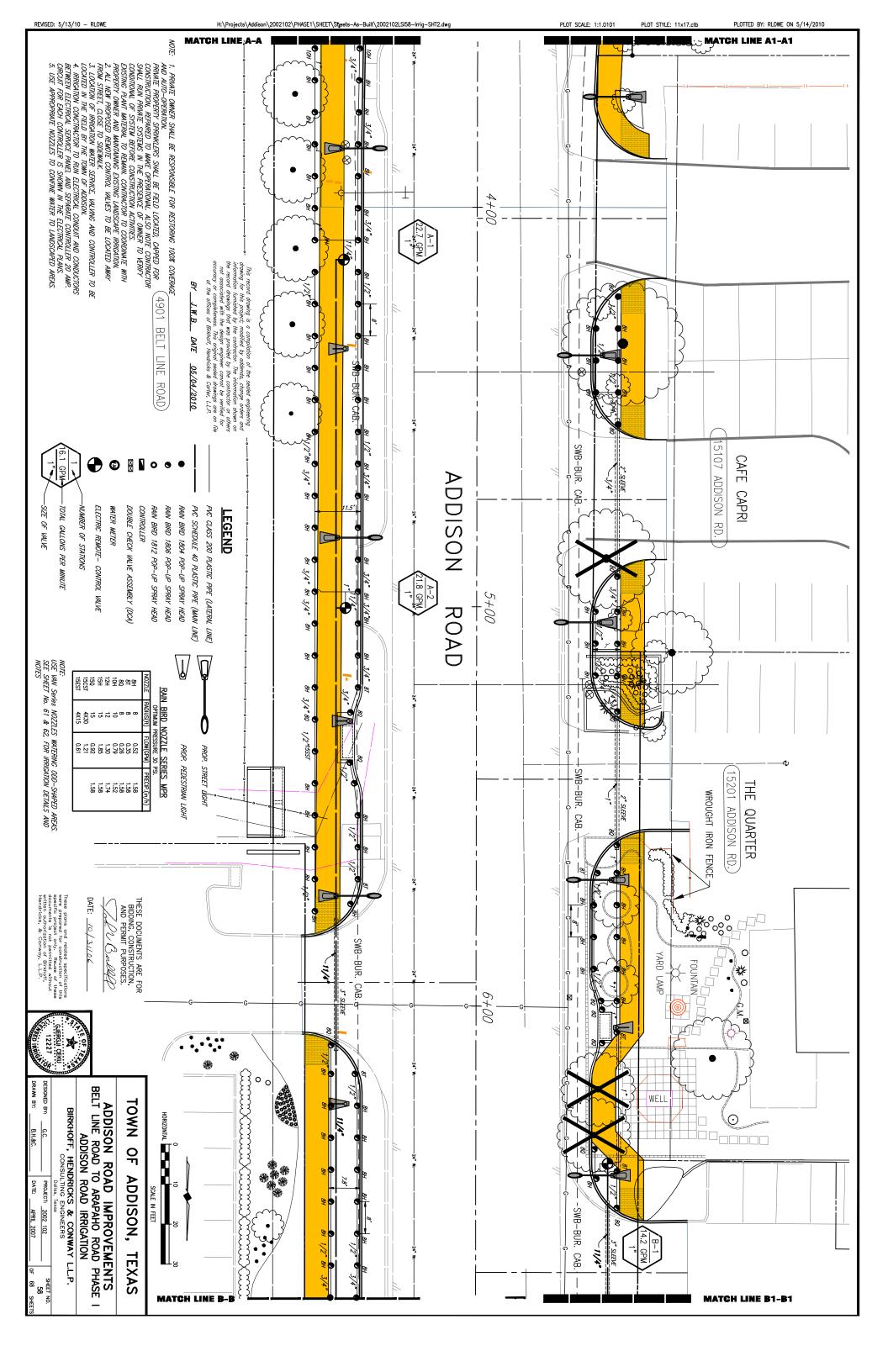


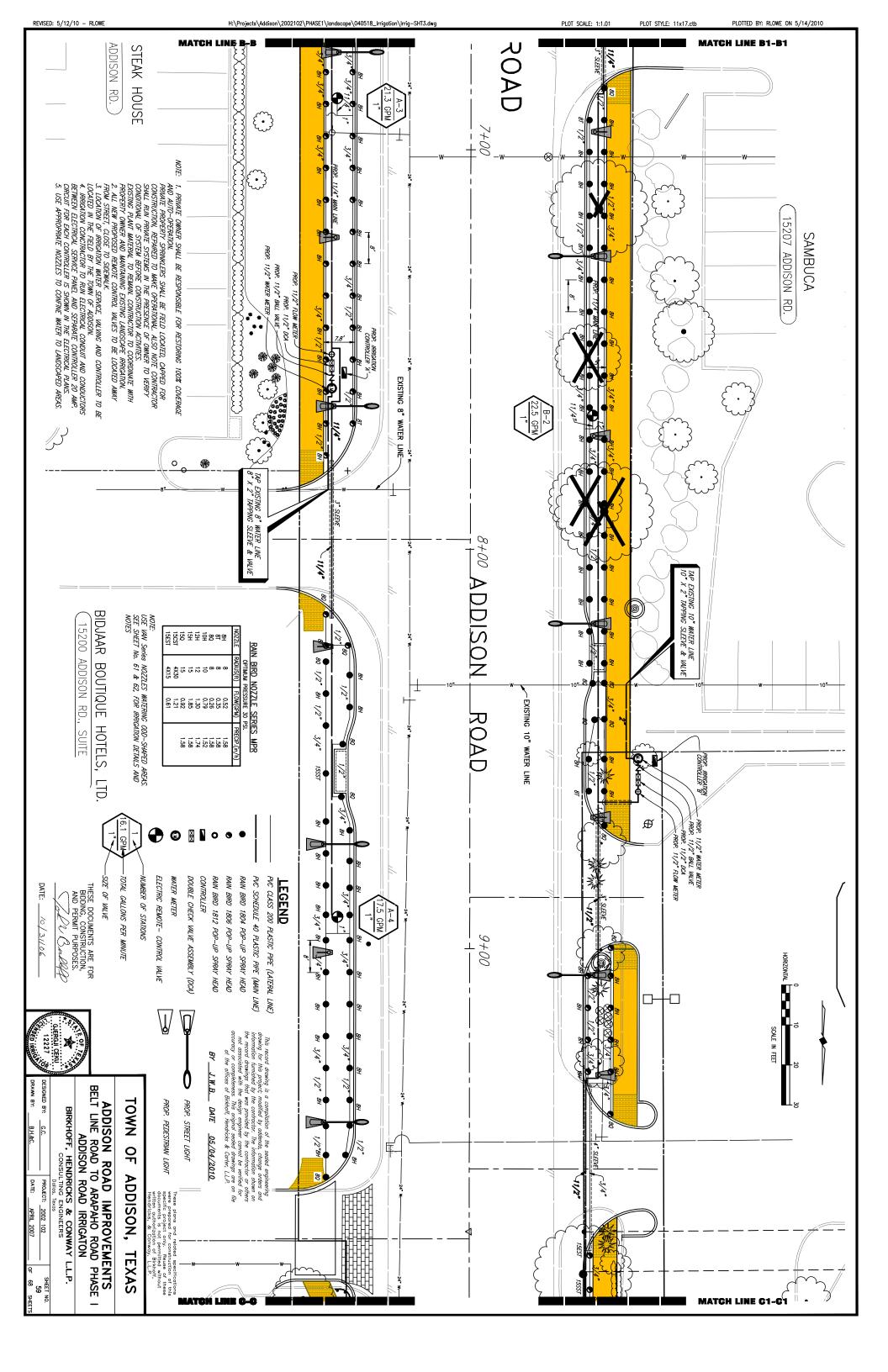


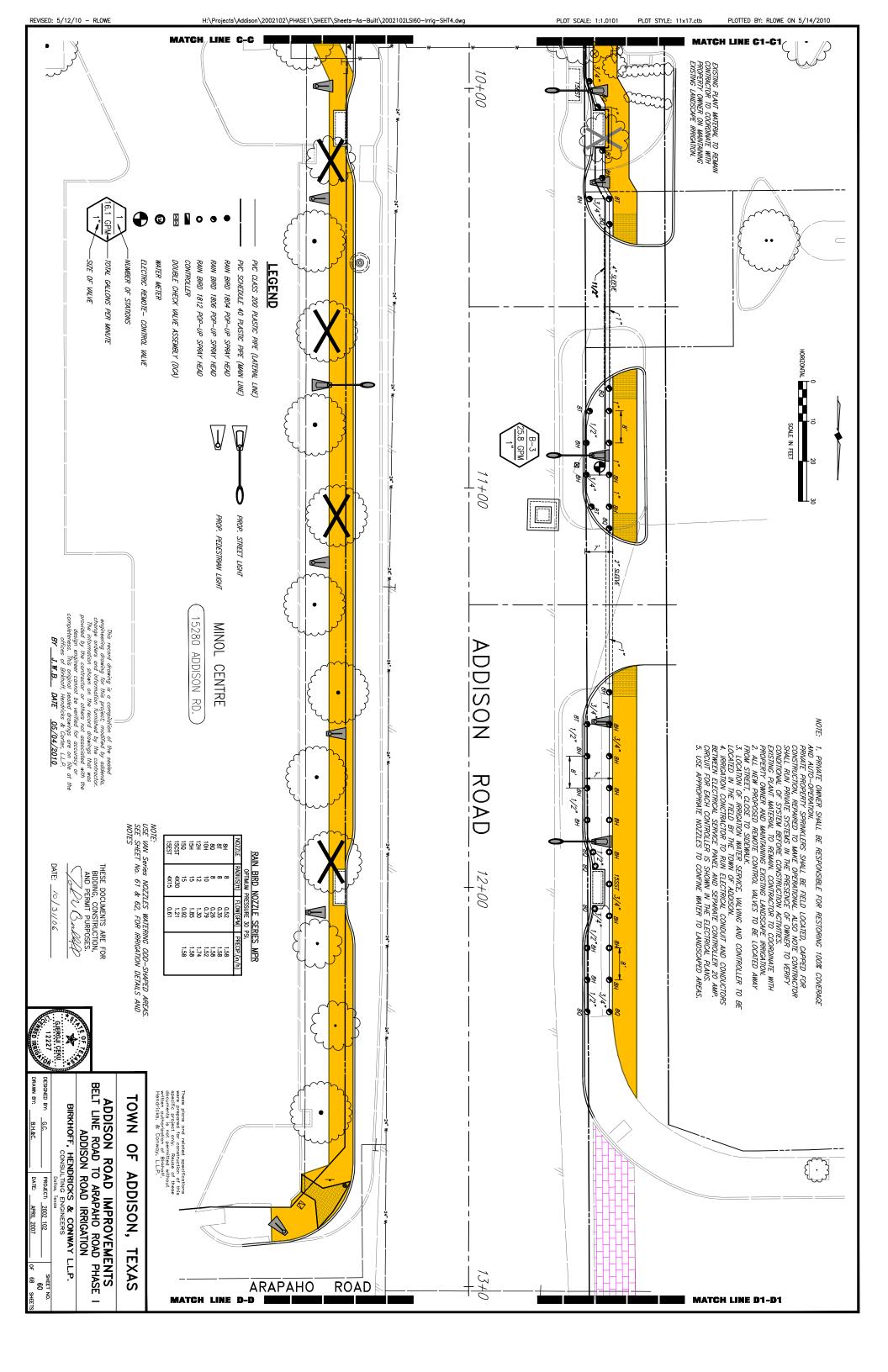












IINSTALLATION DETAIL

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05/04/2010

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HALF PLOT: 11x17.PCP

<u>≤</u>

LATERAL

12" MIN

RAIN MASTER EVOLUTION DX—2
CONTROLLER—ELECTRICAL POWER
DISTRIBUTION AND PHONE SERVICE
LINES TO THE IRRIGATION CONTROLLERS
SHALL BE THE RESPONSIBILITY OF THE
IRRIGATION CONTRACTOR. THE ELECTRICAL
AND PHONE LINES (AND ALL RELATED
WORK) SHALL BE INSTALLED PER ALL
LOCAL, STATE AND NATIONAL CODES.

REF. SPEC.

FINISHED GRADE

1.75" typ.-

6.5"

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2

Pedestal Mounting

Mounting Template provided for easy installation

BUNDLE WIRING

MAIN LINE

TRENCHING DETAIL
NO SCALE

INSTALLATION DETAIL

EVOLUTION DX2 STAINLEES STEEL PEDESTAL CABINET INSTALLATION

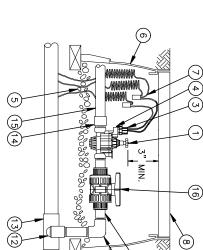
RAIN BIRD 1800 SERIES

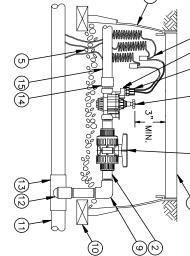
WEATHERMATIC 11000CR SERIES REMOTE CONTROL VALVE

(5)

- (a) (4) (d) \bigcirc
- PVC SCH 40 ELL PVC SCH 40 FITTING
- POP-UP SPRAY SPRINKLER: RAIN BIRD 1800
- FINISH GRADE/TOP 1/2" ABOVE TOP OF MULCH
- \bigcirc

-) 6" GREEN NIPPLES CUT TO FIT.) PVC LATERAL PIPE





- 9 PVC SCH. 40 ELBOW

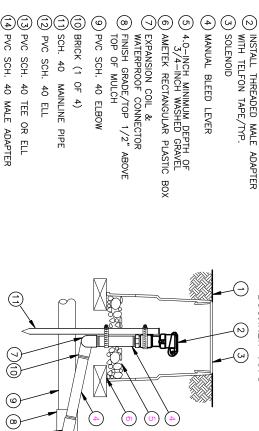
- (16) PVC BALL VALVE

INSTALLATION
NO SCALE

INSTALLATION DETAIL
NO SCALE

- (3) PVC SCH. 40 TEE OR ELL (14) PVC SCH. 40 MALE ADAPTER (5) PVC LATERAL PIPE

- (10) BRICK (1 OF 4) (11) SCH. 40 MAINLINE PIPE (12) PVC SCH. 40 ELL



5 4.0-INCH MINIMUM DEPTH OF 3/4-INCH WASHED GRAVEL

(4) MANUAL BLEED LEVER

3 SOLENOID

Weathermatic 11000 FCR SERIES REMOTE CONTROL

. VALVE

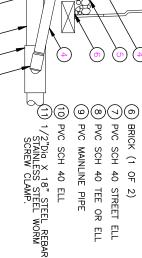
QUICK COUPLER VALVE

BUCKNER V075

(3) 10" ROUND PLASTIC AMETEK VALVE BOX.

(5) 3-INCH MINIMUM DEPTH OF 3/4-INCH WASHED GRAVEL (4) PVC SCH 80 NIPPLE (LENGTH AS REQUIRED) 2 QUICK-COUPLING VALVE: BUCKNER VO75

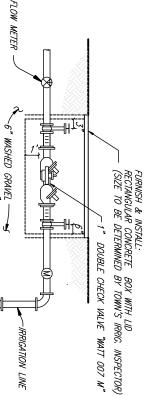
(1) FINISH GRADE/TOP 1/2" ABOVE TOP OF MULCH



(1) 1/2"Dia X 18" STEEL REBAR WITH STAINLESS STEEL WORM SCREW CLAMP.

DETAIL

DOUBLE CHECK VALVE ASSEMBLY



EXIST. WATER LINE -

TOWN OF

ADDISON, TEXAS

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~6" WASHED GRAVEL ——► 2" CORPORATION STOP— INSTALLATION DETAIL
NO SCALE

ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE I
ADDISON ROAD IRRIGATION NOTES & DETAILS

BIRKHOFF, HENDRICKS & CONWAY, L.L.P.
CONSULTING ENGINEERS

DATE:

SHEET NO.

Town of Addison Irrigation Specifications

levised 04/22/04

These revised specifications supersede any and all others. However, any MUST discrepancies between the approved construction plans and those of the Town MUST be brought to the attention of the Town's designated representative for a final determination. The contractor will present the Town's representative an as-built plan at the final walk-through, along with three Bucker VO75 quick coupling keys with

All work is to be accomplished by or directly supervised at all times by an on-site Irrigator licensed by the State of Texas.

2. The contractor shall verify the water pressure before the installation begins. If the static pressure is different than that of the design pressure, contact the designer and Town's representative immediately so changes can be made. Send o fox to the Parks dept. at 972-450-2834 with the current dated and timed static pressure reading, Design head to head with no single head coverage. Use appropriate size nozzles for a given landscape area so as not to spray onto or over paved surfaces or structures. Do not exceed manufacturer's recommendations.

PLOT STYLE: 11x17.ctb

3. The irrigation installer is responsible for resetting head and/or box height due to settling and after turf, groundcover, shrubs, trees, and mulch is added to the landscape areas. The irrigation contractor must supply a workmanship warrantly for (1) year from date of acceptance.

4. Plans are diagrammatic and field adjustments are often necessary. For this reason, prior to trenching, valve locations and head layout with flags needs to be done and approved by the Town's Irrigation inspector. Not cloing so may result in the relocation of heads and/or valves at the irrigation contractor's expense.

5. Water Taps: Water taps will be 2" in size. All parts must conform to the Town of Addison Water Department specifications and are the responsibility of the irritation contractor to provide. Inspection of taps by the Water Department Representative <u>must</u> occur. Excavation and tap permits are required. Contact the Town of Addison Water Department at (972) 450-2871.

6. Water Meters: Only Master or Hersey meters with two (2) brass flanges are acceptable. Meter lay lengths must be in accordance with the Town of Addison Water Department's specifications, housed in appropriate size (to be determined by the Town's Irrigation Inspector) concrete box with ild. New stainless steel bolts and nuts must be used in the installation along with new neoprene gaskets. The box should be level with the fland grade using concrete pavers to support and prevent sinking. Backfill inside the box, 3" below meter base with at least 6" of fine (1/2") pea gravel. Connection to main must be approved and inspected by the Town's Water Department Inspector and all tap materials are to be purchased at the expense of the contractor and must comply with the Town of Addison's specifications.

7. Backflow Devices: Only Watts 007 M series inline check valve assemblies with the staniess steel ball valve handles and nuts are to be used. Irrigation contractor shall provide and install brass plugs for the test codes. Connect to the flange using tefion taped copper nipple of sufficient length to center the DCA within its housing.

B. Rotors - Only Hunter I-20 Series are permitted, unless noted otherwise. Install $\mathscr U$ above finished grade.

14. Risers: Use Sch 80 PVC with Weathermatic LXS Series shrub head adapters with a ½"X6" green poly cut-off nipple screwed into the threaded fitting in the ground and a threaded coupling between the two. The irrigation inspector reserves the right to determine placement of risers versus pop-ups.

15. Wiring: All wires will be 14 gauge UF. Station wires will be red. Common wires will be white. Master valve wire will be blue. Anyther wiring changes direction, such as at an elbow or a tee, allow a loop of at least 12 inches alongside the fitting that location. Only continuous wire runs are permissible. Wher should follow the main line where possible and lay along a single side not crossing over lateral lines. Wire is to be placed under mainline with 2" of dirt between wire and pipe.

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16. Wire Connectors: <u>Use only DBY connectors</u> for all field wire splices other than at the valves themselves. Allow at least 35 inches of pigsialed wire at each splice. <u>Use King One Step</u> tan colored connectors for all valve splices, All valve box splices are to be housed in standard (large) Ametek rectangular plastic valve boxes. All field splices are to be in 10 inch round Ametek plastic valve boxes or standard, large rectangular Ametek plastic valve boxes at the discretion of the Town's representative. Number of splices is determining factor for choice of box size.

17. Backfilling: Prior to any backfilling of trenches, an inspection by the Town's irrigation representative must take place and any necessary changes implemented; otherwise manual excavation to enable proper inspection will be necessary. Use clean and approved topsoil to backfill all pipe to depth. All heads and boxes are to be backfilled to grade with clean topsoil. No rocks greater than 1 inch are allowed. Compact trenches to alleviate settling. Minimal depth of coverage is 12 inches.

34s. Valve sequencing must be performed by the contractor and in an order approved by the Town Irrigation Inspector. At least 12 inches of extra station wiring within the bottom of the pedestal is necessary for each zone and must be of neat and orderly

19. Any deficiencies in coverage noted by the Town's irrigation inspector will be rectified at the cost of the contractor.

20.Controller: A Town irrigation representative will determine the type of controller to be used. All controllers shall have a concrete pad of 36°,36°%c". Pad will be set at 3" above final grade. Install the controller after the concrete pad is completely cared (two days). Use only appropriately sized stainless steel bolts, washers and nuts to secure the controller to the concrete pad. All wiring is to enter the padestal via appropriately sized PVC sweep ablows extending at least 1" thu and 6" out from under the pad. Control/master valve wiring, flow meter wiring and 120-4 service wiring are to be separated with each having its own access elbow. An additional spare %" sweep elbow for phone service is to be installed as well. All national and local codes must be followed during the installation.

A. A/C controller - Only Irritrol MC Plus controllers will be acceptable. Both Min-click rain and freeze sensors will be installed and placed where they can operate properly. All non-Rain Master controllers must be permanently wired for quick attachment to a Rain Master remote control unit.

The device will be housed in an appropriate size, (to be determined by the Town's Irrigation Inspector) rectangular concrete box with lid using concrete pavers for proper stability and use extensions to adjust height. The irrigation contractor shall be responsible for the DCA testing in accordance with State of Texas law, using a leresponsible for the DCA testing in accordance with State of Texas law, using a leresponsible for the DCA testing in accordance with State of Texas law, using a leresponsible for the DCA testing in accordance with State of Texas law, using a leresponsible for the DCA testing in accordance with State of Texas law, using a leresponsible for the DCA testing in accordance with State of Texas law.

8. Sleeves: All paving must have Town approved sleeve sizes and quantities present. It is the responsibility of the irrigation contractor to notify the Town's Irrigation Inspector of any area where sleeves should be present but are not and provide such materials at his cost. Any paving installed without sleeves will necessitate a bore and subsequent materials at the Irrigation contractor's expense. All sleeves 2" and smaller will be Schedule 40 PVC with size and location noted on the plan. Larger sizes will be Class 200. All piping underneath paving, including sleeves in the sleeves are to be belied and PVC pipe. A minimum length of 12 inches of sleeve material must extend beyond the pavement.

9. Glue and Primer: Use Turfitte brand glue on laterals and IPS Grey Heavy Body on main lines and a good quality purple primer on all . Avoid excessive use and wipe excess glue off of all joints and fittings with a clean rag.

10. Pipe: All main line pipe 2 inches and smaller is to be Schedule 40 belled PVC; larger sizes are to be Class 200 belled PVC with a minimum depth of 14* and a maximum depth of 16*. Put not nore than two (2) pipes in any one trench and separate the main line from the lateral line with at least two (2) inches of cover. Class 200 belled PVC lateral piping is to be used with a minimum depth of 12* and a maximum depth of 14*.

11. Fittings: No crosses are permitted. Separate tees, 45's, elbows and other fittings by at least 12 inches. Reduction tees are preferred over use of single-reducer bushings. Multiple reducer bushings will not be accepted. Only Spears and/or Lasco fittings are permitted. Allow 18 inches outside of sleeve before the first fitting. No 45 degree elbows on 1 inch and larger pipe are allowed.

B. Station Valves: Only Weathermatic 11000 FCR series valves are permitted. A Ball Valve will be installed before every station or zone valve. They are to be located within a standard (large) Ametek rectangular plastic valve boxes with 4 to 6 inches of (1/2") pea gravel placed underneath the valve in such a manner as to prevent soil infiltration into the box. The pea gravel should be 2 inches from the bottom of the valve body. A minimum of 3" inches of valve box must extend below bottom of valve. If necessary use valve box extensions.

B. Battery and/or Solar Operated Controllers - Only LET controllers will be acceptable. Install rain or freeze sensors on these controllers with SKIT821-4 installation kit. Install on galvanized thick wall poles and set controller panel to height above finished grade to be determined by Town's representative.

C. Rain Master: Only an approved size Rain Master Evolution DX-2 controller with a stainless steel pedestal and heavy duty transient protection is permitted. The controller must include all necessary hardware to ensure reliable communication and operation with the Town's central control located at 16801. Westgrove. Installation must include the following Rain Master hardware, purchased only from a Rain Master supplier: DX-03 sensor board, DX-PH phone communication option, Data Industrial flow meter (same size as the mainline), and shielded EV-CAB-SEN flow meter cable. It is the irrigation contractor's responsibility to entail the cost of and work in conjunction with South Western Bell Telephone to establish a dedicated phone service and install an interface within the pedestal at each controller location via direct burfal cable within 1. PVC condult. The entire installation must conform to Rain Master specifications and be approved by the Town's irrigation inspector prior to and be inspected during installation. Such specifications will include grounding and pad configurations and distances of separation from water meter to DCA to master valve to flow meter and the first fitting. A functional Mini-cilk freeze and rain sensor with a Hunter bypass switch must be installed in an approved location and by an approved method. For part numbers and pricing of any Rain Master equipment, contact Mark Stricker of John Deere Landscapes at 972-881-205. For technical questions, call John DuBose of RainMaster at 214-632-2289.

Communication is the key. **If you are unsure, CALL** Ron Lee, the Operations ager of the Addison Parks Department at (972) 450-2863.

12. Valves:

A. Master Valves: Every point of connection to the water supply system shall have a Weathermatic 11000 FCR series valve as the Master Valve, housed in a standard (large) Ametek rectangular plastic valve box with 4 to 6 inches of small (1/2") pea gravel placed underneath the valve in such a manner as to prevent soil infiltration into the box. Use concrete pavers or bricks placed under edges of valve box for stability. Note: Valve box must not rest on pipe. Blue wire shall be used as the station wire for the Master Valve.

C. Ball Valves: Female threaded plastic Spears or Lasco ball valves with positive T-handle cut off must be installed on every 200 feet of mainline

NOTE. —The irrigation system has been designed to operate with minimum static water pressure of 50 p.s.i. and total demand 26 gpm. Notify Town's representative if dicrepancy arises.

—Contractor shall verify nozzle sizes in field for MPR and head to head coverage. Contractor to mimimize overspray. No overspray onto pavement shall be accepted.

engineering drawing is a compilation of the sealed engineering drawing for this project; modified by addends, change orders and information furnished by the contractor. The information shown on the record drawings that was provided by the contractor or others not associated with the design engineer cannot be verified for accuracy or completeness. This original sealed arrawings are on file at the offices of Birkhott, Hendricks & Carter, L.L.P.

BY J.W.B. DATE 05/04/2010

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DATE:

isolation purposes. A ball valve is also required to be installed before every station valve. Use 10" Ametek valve box with a minimum of 3" extending below bottomof valve and fill to bottom of valve with ½" pea gravel. Use bricks or concrete pavers under box for stability.

D. Quick Coupler Valves: Use only Buckner V075 single lug ¾" quick coupling valves with a metal top. They are to be connected to a threaded fitting. Teflon tape and appropriate length of gray schedule 80 nipples and schedule 40 fittings are to be used for the swing joint. Secure to 18 inch by ½ inch steel rebar with a stainless steel worm screw clamp. House QCV in a 10 inch toround plastic Ametek valve box. Install Spears ball valve prior to each QCV. Bricks or pavers need to be installed under edges of valve boxes for Stability. Backfill bottom of box with ½" pea gravel half way up body of valve.

E. Flowmeters: Purchase from a Rain Master supplier and install appropriately sized Data Industrial flowmeter. Follow all installation instructions as approved by Rain Master. The irrigation contractor must also purchase from Rain Master and install shielded Rain Master EV-Cab-Sen flow meter cable and install within continuous % or larger gray PVC conduit with 6 inch or larger "J-boxes paced every 200 feet or where 360 degrees of fittings are installed; only sweep fittings are pernitted. Only a continuous run of cable is allowed; in a splices will be allowed except at the point of comection to the flow meter. Connections at the flow meter must first be soldered and then water proofed within a 3-M DBY connector. Note: certain Rain Master requirements must also be met regarding installation order and distances of separation between DCA, flow meter, master valve and the first fitting. It is the responsibility of the irrigation contractor to adhere to these requirements. At final walk through, proper operation of the flow meter at the Kain Master controller must be demonstrated by the irrigation contractor.

13. Heads: All heads will be installed using polyethylene green nipples (¾"x6" for rotors and ½"x6" for pop-ups) screwed into threaded fittings unless noted otherwise. No swing joints on 4" pop-ups or rotors will be allowed.

A. Pop-ups – Only Rainbird 1800 series are permitted. Install % inch the finished grade. above

a. 4 Inch pop-ups: turf, tree bubblers within turf areas (use Hunter PCN 10 bubbler nozzles on spray heads).

b. 6 inch pop-ups with no side inlet: very low ground cover (less than 6 inches at mature height).

d. Use ½" Sch 80 risers with schrub adapter and Hunter PCN 10 bubbler rozzles for all tree wells with tree grates. Risers shall be a minimum of 2" below bottom of tree grates with nozzle 2" above mulch. c.12 Inch pop-ups with side inlet: Ground cover and low growing shrubs. The ground cover and shrubs should not be more than 1.2" at maturity. The Town inspector reserves the right to determine if and when side inlets installed using funny pipe verses the bottom inlet will be allowed. When authorized, use Hardie Blue Line Pipe with Toro barbers.

of any

TOWN OF ADDISON, TEXAS

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ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE I
ADDISON ROAD IRRIGATION NOTES & DETAILS

DRAWN BY: DESIGNED BY: BIRKHOFF, HENDRICKS & CONWAY, LLP.

CONSULTING ENGINEERS

Dalias, Texas PROJECT

GJERGJI CEKU

В.Н. & С. DATE: **APRIL** 2007 SHEET NO.
62
68 SHEE

MODIFICATIONS TO ELECTRICAL SHEETS

A. Sheet No. 63 (Drawing E-1): Roadway Lighting - Part "A" include such cost in the appropriate bid items. Contractor shall implement these modifications to the Electrical Sheets and Add an additional Utility Service/Detail 2/E-5 for a total of two (2)

Utility Services. One service will serve Roadway Lighting exclusively

- Currently a single feeder containing 2 #10, #10G, 3/4" C, is shown from circuit will serve Roadway Lighting exclusively while the other feeder feeder of 2 #10, #10G, 2" C, to also loop through each pull box. One will serve Pedestrian and Irrigation Systems. the single service to each pullbox/light fixture. Provide an additional and the other service will serve Pedestrian and Irrigation Systems.
- Change the conduit size to 2-inch.
- B. Sheet No. 64 (DrawingE-2): Roadway Lighting Part "B"
- Currently a single feeder containing 2 #10, #10G, 3/" C, is shown from will serve Pedestrian and Irrigation Systems. circuit will serve Roadway Lighting exclusively while the other feeder the single service to each pullbox/light fixture. Provide an additional feeder of 2 #10, #10G, 2" C, to also loop through each pull box. One
- Change the conduit size to 2-inch.
- C. Sheet No. 65 (Drawing E-3): Roadway Lighting Part "C"
- Currently a single feeder containing 2 #10, #10G, 3/" C, is shown from circuit will serve Roadway Lighting exclusively while the other feeder will serve Pedestrian and Irrigation Systems. feeder of 2 #10, #10G, 2" C, to also loop through each pull box. One the single service to each pullbox/light fixture. Provide an additional
- Change the conduit size to 2-inch.
- D. Sheet No. 66 (Drawing E-4): Roadway Lighting Part "D"
- .-Add an additional Utility Service/Detail 2/E-5 for a total of two (2) and the other service will serve Pedestrian and Irrigation Systems. Utility Services. One service will serve Roadway Lighting exclusively
- 'n circuit will serve Roadway Lighting exclusively while the other feeder Currently a single feeder containing 2 #10, #10G, 3/4" C, is shown from feeder of 2 #10, #10G, 2" C, to also loop through each pull box. One will serve Pedestrian and Irrigation Systems. the single service to each pullbox/light fixture. Provide an additional
- Change the conduit size to 2-inch.

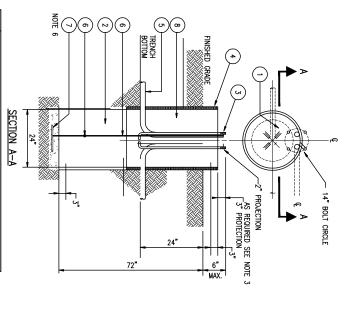
Ή Sheet No. 66 (Drawing E-5): Details.

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- Modify detail 3/E-5 height currently indicated for the pole as 38'-0" to 17'-0".
- fixture and obstruction light will both be served from the pedestrian lighting circuit and be controlled by the obstruction Detail (3/E-5) Fixture SAA add the following: "For combination roadway/pedestrian type SA fixture, the light ight photo cell."
- F. Sheet No. 67 (Drawing E-6):
- of the roadway fixture. The roadway light fixture SA will be served from the roadway lighting circuit and be controlled by the photo cell mounted on top
- 'n Add the description "photo cell" to the leader line on the detail for fixture SB to control the fixture.
- G. Sheet No. 68 (Drawing E-7): Schedules
- 1. Modify Light Fixture Schedule note 4. Note should read "Similar to "SA" except 17-feet maximum height pole".
- Add the following:

214-658-9000." selected by the Town of Addison. The manufacturer of the fixtures is Lumec-Schneder of Canada, local representative, "The poles shall be steel and the color of the top coats shall be

schematically. Place light standards to locations as shown on coordinate H. Locations of light standards shown on Electrical Sheets are shown layout plans, make field adjustment as required and approved by the



	8	7	6	5	4	3	2	1	IIEM
	CONCRETE (SEE NOTE 1)	GROUND, POLE BUTT WIRE COIL (SEE NOTE 6)	WIRE, #5 COPPER, SOLID (SEE NOTE 6)	CONDUIT, PVC OR PEC (NOT IN FOUNDATION AUN)	CONCRETE FOUNDATION TUBE, 24 IN. DIA.	ANCHOR BOLT, GALV. 1" W/25" & 30" POLES & 1/4" W40' POLES	#5 REBAR, STRAIGHT, 60 IN. LONG, 3" COVER	#3 REBAR, 18 IN. DIA. HOOP, 3 IN. OVERLAP	DESCIPTION

REET LIGHT BASE

1. CONCRETE TO BE N TOP OF FOUNDATION T SURFACE. AVOID EXCL OF 7 DAYS BEFORE P : MIN. 3000 PSI AT 28 DAYS. (5 SACK) MAX. 1 TO BE TROWELED TO A FLAT AND LEVEL CCESSIVE TROWELING. CONORETE TO SET A MIN. POLE INSTALLATION.

REBAR HOOPS ARE TIED BEGINNING 3' BELOW TOP OF CONCRETE FORM AND ARE REPEATED AT APPROXIMATE 1' INTERVALS TO BOTTOM OF FOUNDATION.

3. 1" ANCHOR BOLTS TO BE USED WITH 25' & 30' SQUARE AND ROUND POLES.

4. CONCRETE FORM OF SONOTUBE TO EXTEND TO BOTTOM OF TRENCH OR AS NEEDED.

5. PROVIDE MIN. 24° GROUND WIRE PIGTAIL OUT OF CONCRETE FOR CONNECTION TO POLE.

6. A MIN. OF 12' OF BARE #5 SD CU WIRE TO BE PLACED IN BOTTOM OF HOLE AND COVERED WITH 2' OF DIRT.

7. IF SOIL HAS BEEN OF DISTURBED SOIL. DISTURBED, EXTEND FOUNDATION BY DEPTH

8. NOT APPLICABLE FOR POLE HEIGHT GRATER THAN 40'.

engineering drawing for this project, modified by addenda, change orders and information furnished by the contractor. The information shown on the record drawings that was rounded by the contractor or others not associated with the design engineer cannot be verified for accuracy or applications original seaded drawings are on file at the original seaded drawings are on the at the offices of Birkhoff, Hendricks & Carter, L.L.P. DATE

DATE: HESE DOCUMENTS ARE FOR BIDDING, CONSTRUCTION, AND PERMIT PURPOSES.

TOWN OF ADDISON, TEXAS

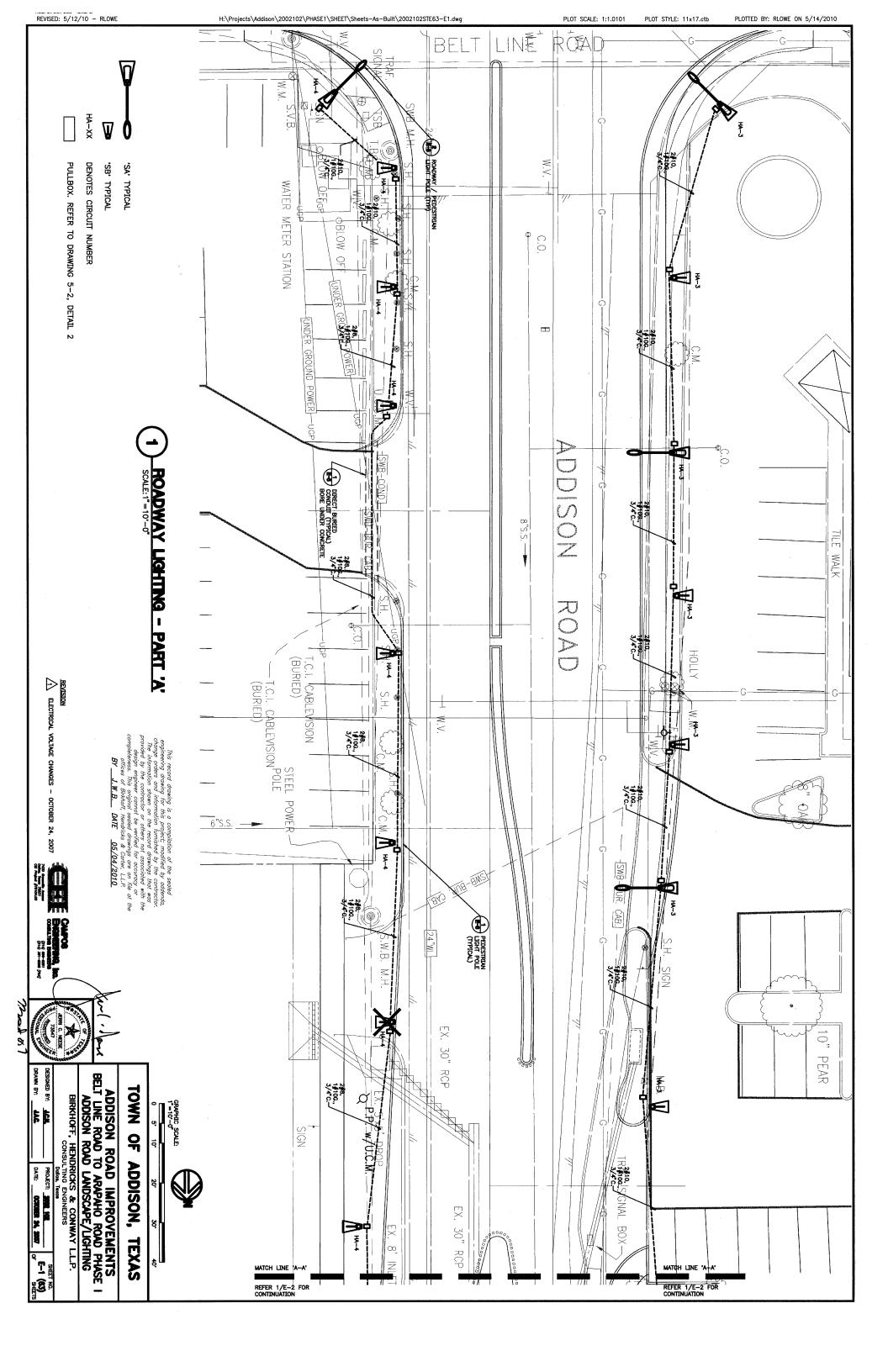
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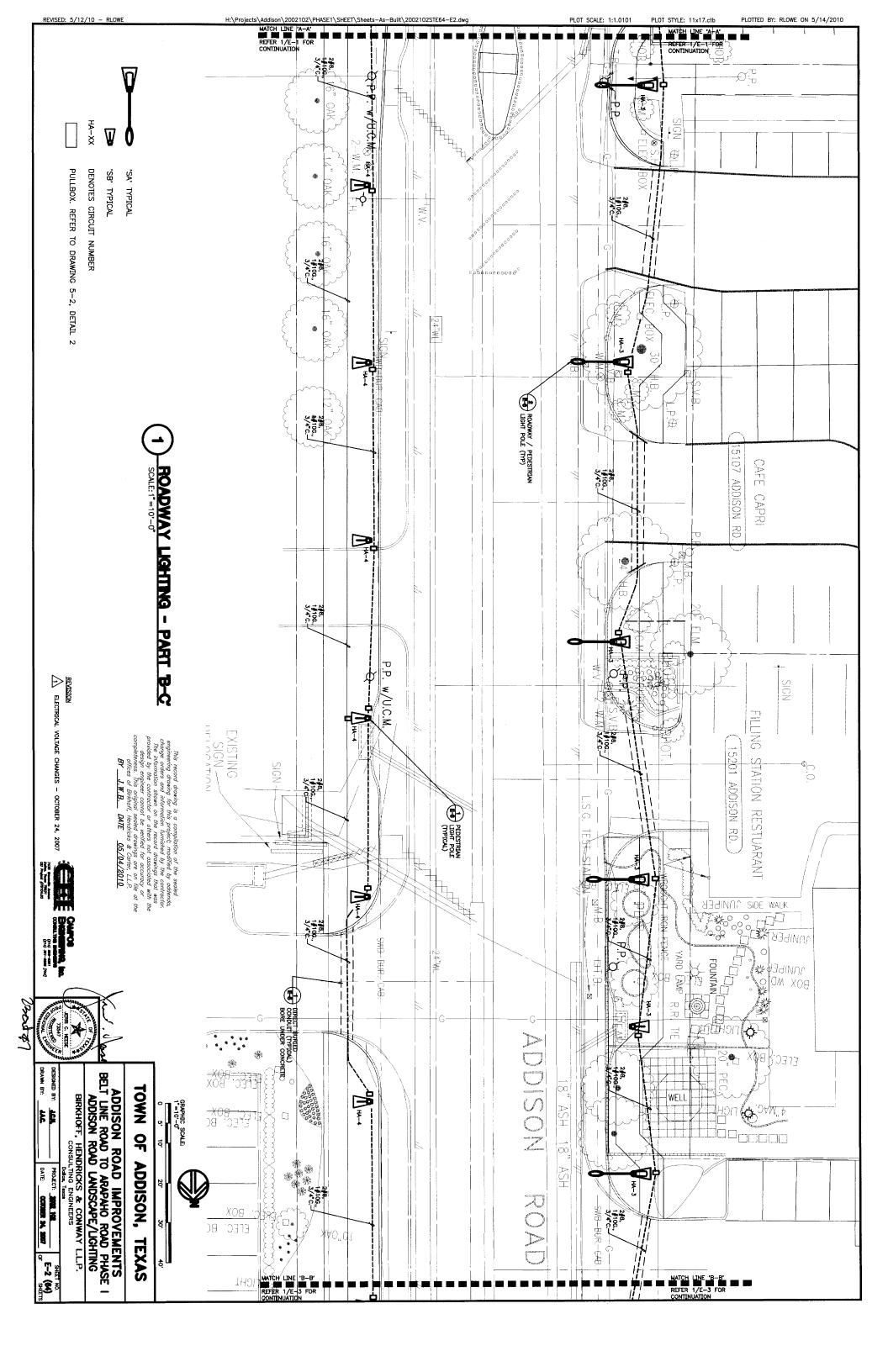
ADDISON ROAD IMPROVEMENTS
BELT LINE ROAD TO ARAPAHO ROAD PHASE
ELECTRICAL MODIFICATION NOTES

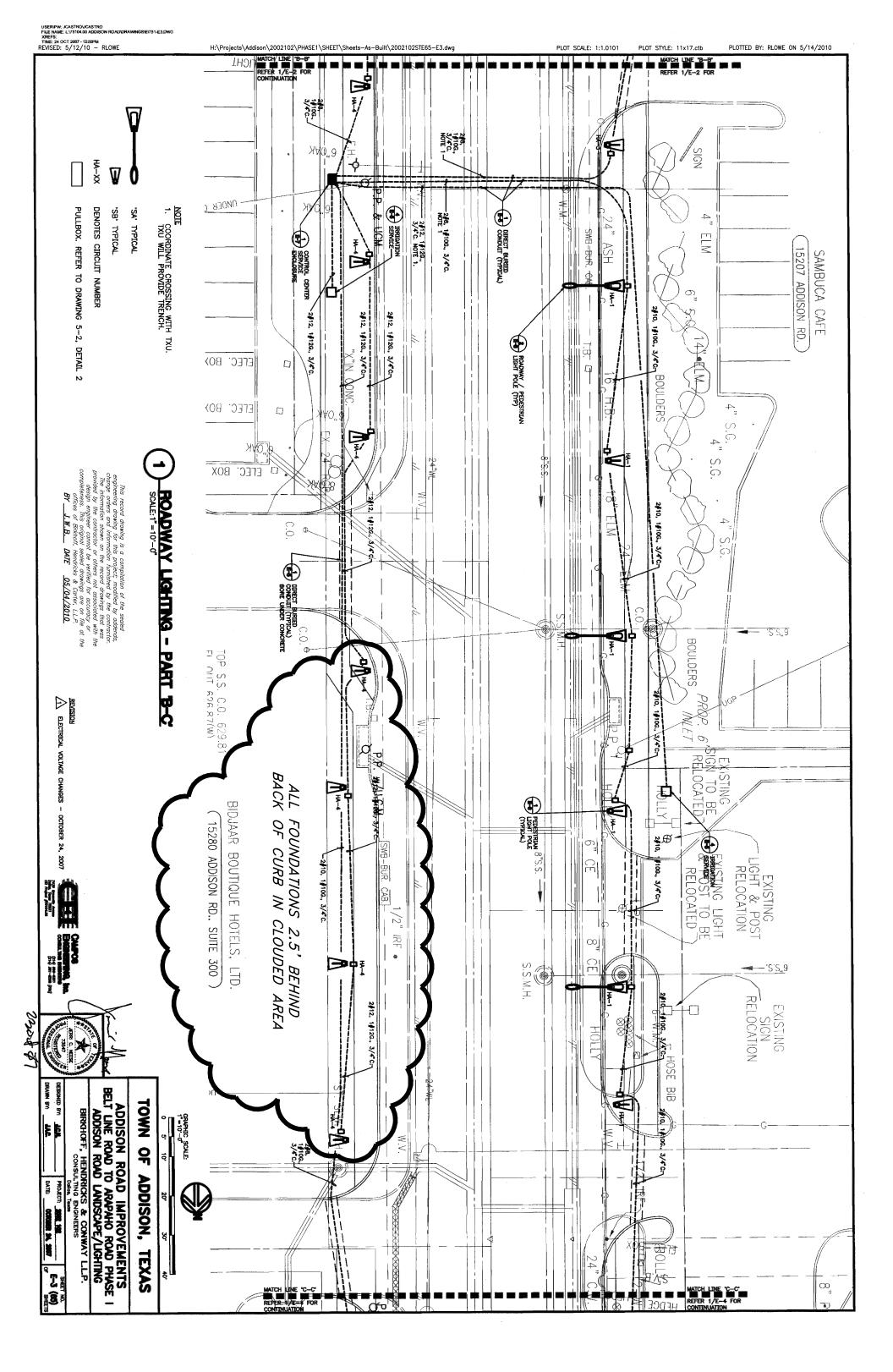
BIRKHOFF, HENDRICKS & CONWAY, L.L.P.
CONSULTING ENGINEERS
Dollas. Texas

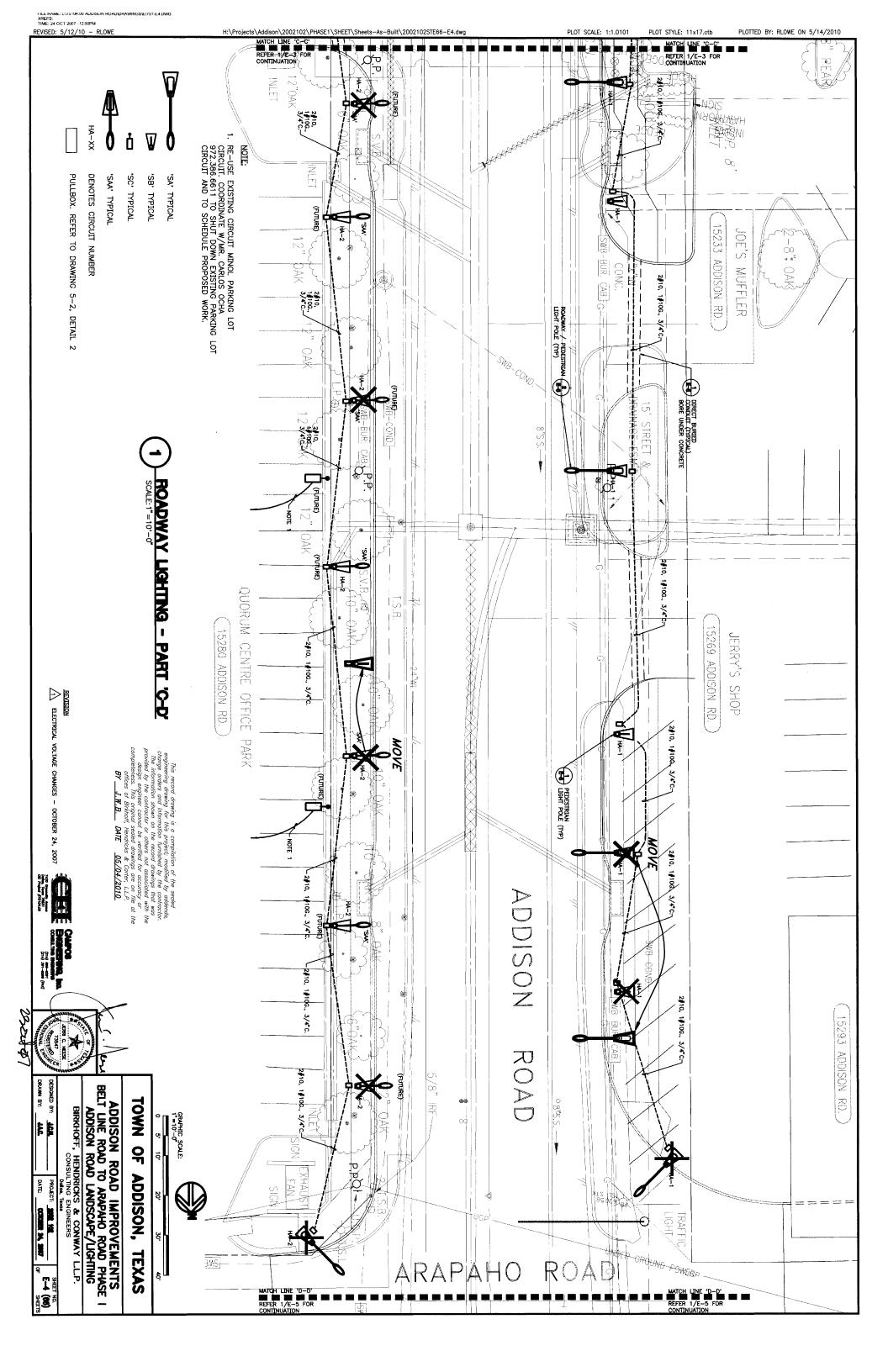
DATE: PROJECT: SEPTEMBER 2006

DRAWN BY: ESIGNED BY:









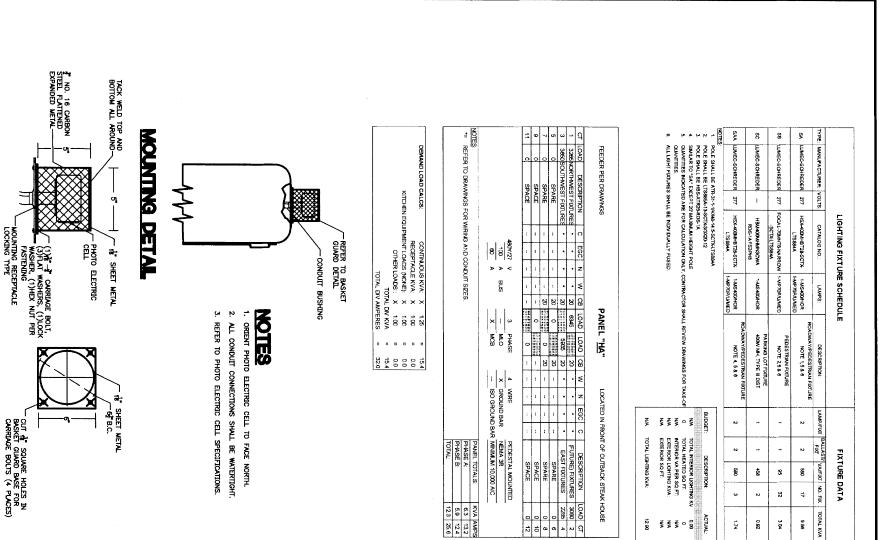
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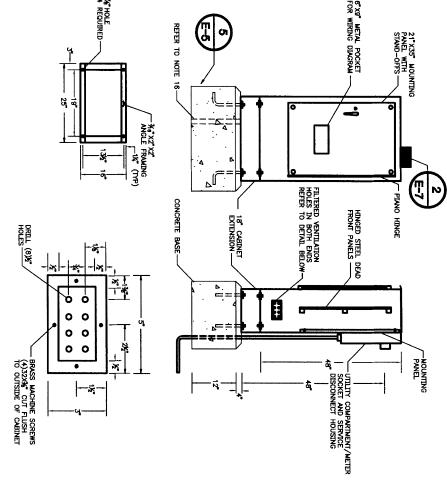
SHET NO. E-5 (88) SHETS

SHEET NO.

PHOTO ELECTRIC CELL MOUNTING DETAIL
SCALE:NONE

BASKET GUARD DETAIL





5. PROVIDE BOLT-ON CIRCUIT BREAKERS.

USE TAMPER-PROOF CABINETS SO AS TO PREVENT INSERTION OR WIRES OR OBJECTS INSIDE THE CABINET BY UNAUTHORIZED PERSONS.

MAKE FULL HEIGHT DOORS WHICH USE STAINLESS HINGES AND HAVE DOOR STOPS AND LOCKS.

USE HELIARC WELDING METHOD FOR ALL EXTERNAL AND INTERNAL CONTINUOUS WELDS, MAKE WELDS FREE OF CRACKS, BLOW HOLES AND OTHER IRREGULARITIES, MAKE INSIDE AND OUTSIDE EDGES OF CABINET FREE OF BURSS AND PROVIDE SMOOTH, UNIFORM AND NATURAL ALUMINUM FINISH.

INSTALL FREE STANDING AND SELF-SUPPORTING CONTROL CENTER CABINET, PER THE DRAWINGS, NEWA 3R CONSTRUCTION. USE ALUMINUM ALLOY, THICKNESS 0.125°, FOR CABINET HOUSING AND DOORS.

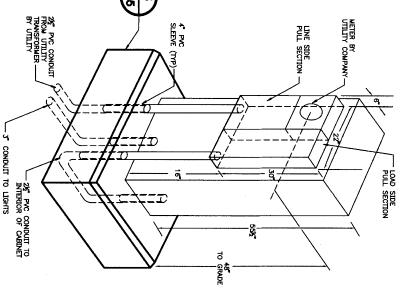
PROVIDE DOUBLE FLANGED DOOR OPENINGS ON ALL (4) FOUR SIDES TO INCREASE STRENGTH AROUND THE OPENINGS AND ALSO TO KEEP AWAY ANY LIQUID OR DIRT FROM ENTEXING THE ENCLOSURE WHEN THE DOOR IS OPENED.

PROVIDE A SLOPED SURFACE ON THE TOP OF THE CABINET SO AS TO PREVENT WATER ACCUMULATION.

Use door restraints to prevent door movement when opened in windy conditions.

THRNISH CASKETING FOR DOORS WHICH SATISFY PHYSICAL PROPERTIES AS FOUND IN UL 508 THAIL 21.1. MAKE WEATHER-TIGHT SEALS BETWEEN THE CABINET AND THE DOOR.

NOTES



19. COORDINATE THE CONTROL CENTER LOCATIONS AND EQUIPMENT SPACING WITH THE UTILLTY COMPANY. PROVIDE A SHOP DOWNING WITH PROPOSED DIMENSIONS FOR EACH UTILLTY SERVICE LOCATION. SHOW THE PROPOSED UTILLTY EQUIPMENT AND CONTROL CENTER LOCATION. SUBMIT TO THE ENGINEER FOR WEITIEN APPROVA PROOR TO INSTALLING EQUIPMENT.

18. UTILITY COMPANY SHALL PROVIDE AND INSTALL THE TRANSFORMER PAD, CONDUIT, AND WIRING TO METER. THE UTILITY WILL PROVIDE THE METER BASE TO BE INSTALLED BY CONTRACTOR. 17. USE AN APPROVED SILICON SEALER RTV TYPE GREY IN COLOR OR CLEAR, BETWEEN CABINET AND FOUNDATION. 16. IN UNPAKED AREAS A RAISED PCC PAD 36"X4"XWIDTH OF FOUNDATION, SHALL BE PLACED IN FRONT OF THE CABINET, PAD SHALL BE SET 2" BELOW THE FOUNDATION ELEVATION, SLOPE PAD AWAY FROM CABINET.

15. INSTALL 5/8" DIA. BY 10' COPPER CLAD GROUND ROD WITH CLAMP.

 $13.\,\mbox{REFER}$ to specifications for additional construction requirements.

14. THE DOOR SHALL BE RAINTIGHT AND DUSTPROOF.

12. FURNISH GASKETING FOR BOLT HOLES WHICH MEET OR EXCEED THE REQUIREMENT OF A NEMA $3\mathsf{R}$ RATING. 11. FURNISH HINNES MADE OF 075-14 CAUGES 300 SERIES STAINLESS STEEL WELD THE TOP AND BOTTOM OF THE HINGE PIN TO RENDER IT TAMPER PROOF. 10. USE 1/4-20 300 SERIES STAPILESS STEEL BOLTS AND 300 SERIES STANILESS STEEL NY-LOCK NUTS FOR BOLTING HINGES TO THE CABRET AND THE DOOR.

ord drawing is a compilation of the seoled drawing for this project; modified by addenda, is and information furnished by the contractor, is and information furnished by the contractor within shown on the record drawings that was the contractor or others not associated with the Project and the seoled drawings are on file at the soft Birkhoff, Hendricks & Carter, L.L.P.

05/04/2010

GRAPHIC SCALE:

CONTROL CENTER SERVICE ENGLOSURE SCALE: NONE

REVISION

REVISION

ELECTRICAL VOLTAGE CHANGES - OCTOBER 24, 2007

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DRAWN BY: DESIGNED BY:

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DATE:

0070BR 34, 3807

PROJECT: * C-7 (88)

TOWN OF ADDISON, TEXAS

BELT LINE ROAD TO ARAPAHO ROAD PHASE I ADDISON ROAD LANDSCAPE/LIGHTING ADDISON ROAD IMPROVEMENTS BIRKHOFF, HENDRICKS & CONWAY LLP.
CONSULTING ENGINEERS

GROUND ROOS

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- All ground rods installed at electrical pervices, including supplemental lightning protection ground rods specified by
 the plans in other locations such as pole paces, shall be capper coated steel and listed by a NRTL. Rods shall be a
 minimum diameter of 5/8 inch. The length shall be a minimum of 8 feet, turger diameter or length rods may be called
 for in some specific locations, see the individual plans sheets.
 Cround not clamps shall be listed to be in direct contact with the soil. Where concrete encasement is required, the clamp
 shall be listed for coacrete encasement.

B. CONSTRUCTION METHODS

- Ground rods installed in locations such as pole bases, to provide supplemental lightning protection need not be totally in contract with the soil. Where called for by the plans, rods may be encased in soil or concrete or any combination of soil and concrete. When concrete encased, the connection of the conductor to the rod shall be readily accessible for inspection or repairs. When driven into the soil the upper and shall be between 2 to 4 inches below linished grade. Ground rods shall not be placed in the same drilled hole as a timber pale. Ground rods shall be the placed in the same drilled hole as a timber pale. Ground rods shall be installed such that the mod imprinted with the rod's part number is installed as being the upper end. Mon-conductive coalings such as concrete solutions will be removed from the rod at the clamp location. Reuring of lightning protection ground rod wires shall be run as short and straight as passible. Where bends are required they shall have a numburar rodius of four inches.
- Maless specifically called for by the pions, conduits used for ground rad wires shall be mon-metallic. Where metal conduits are specified a grounding bushing and properly sized banding jumper shall be provided and properly installed
- an each end. Where rocky soil or a solid rock bottom is encountered when diving a ground-rod and the horizontal trench placement method is the any viable solution, written authorization from the Town must be aquired.

MI. CROUND BOX

A. MATERIALS

- Ground boxes 15x30x24 inches (WALXD) or smaller shall be polymer concrete of the type required by the descriptive code shown alsowhere. Larger ground baxes shall be as shown elsewhere in the plans.
 All ground boxes and covers shall be pernamently marked either by impress or by permanent ink, with manufacturer's model number and manufacturer's name or logo.
- Covers shall be boilted down, and boilt holes in the box shall be arranged to draft dirt.
 Ground box types A, B, C, D & E shall meet the following requirements:
- a. Ground boxes and covers will be manufactured from polymer concrete reinforced with continuous straints of woven or stitched borosilicate fiberglass cloth. The polymer concrete shall be made from catalyzed polyester resin, sand and aggregate, and shall have a minimum compressive strength of 11,000 psi. Palymer concrete containing chapped fiberglass or tiberglass reinforced plastic is not acceptable.
- Minimum Minimum inside dimensions shall be as follows (width x tength x depth). Type A shall be 11.5 inches x 21 inches x 10 inches, (122311). Type B shall be 11.5 inches x 21 inches x 20 inches, (122322). Type C shall be 15.25 inches x 28.25 inches x 10 inches, (162911). Type C shall be 15.25 inches x 28.25 inches x 20 inches, (162922).

- Type E shall be 11.5 inches x 21 inches x 36 inches, (122317)
 Bottom edge of box or extension shall be tooled with a minimum 1 1/4 inch floringe
 Bottom boxes shall withstand 600 lbs. per sq. ff. applied over the entire sidewall with less than 1/4 inch deflection
 per foot length of box. Ground boxes and covers shall withstand a test leading of 20,000 lbs. over a 10 inch by 10 inch
 area centered on the rower with less than 1/2 inch deflection. Ground boxes and covers shall meet Western Underground. Professional Engineer.
 Covers shall be 2 inch (naminal) Black polymer concrete. All hardware shall be stainless steel. Cover shall be secured area centered on the cover with less than 1/2 inch deflection. Ground boxes and covers shall meet Western Under Standards 3.6. Manufacturer shall supply certification by an independent laboratory or sealed by a Texas-Licensed
- with two 1/2 inch storiess steel bolts. Balts shall be self-retaining and shall withstand a minimum of 70 ft-lts. terque and shall have a minimum 750 lbs. straight pull out strength. Nots shall be floating and shall provide a minimum of 1/2 inch movement from the center of the nut. Covers shall be skid resistant, minimum 0.5 coefficient of frictism. Covers shall be interchangeable between manufacturers and shall conform to the dimensions shown herein. Unless otherwise approved by the fingineer, cover shall be legibly imprinted with the following words in minimum 1 inch

Ground Baxes containing wiring for trattle signats shall be labeled. Danger High Valtage Trattle Signats. Ground boxes containing wiring for Humination systems shall be labeled. Danger High Voltage Humination. Ground boxes containing wiring for traffic management systems shall be labeled. Danger High Voltage Traffic Management.

Ground boxes containing withing for sign Mumination systems shall be labeled, Denger High Voltage Sign Muraination

Costed boxes containing wiring for traffic signols that also contain illumination, powered by the signole electrical service, shall be liabeled, Danger High Valtage Traffic Signal.

CONSTRUCTION METHODS

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- 1. Ground baxes shall be set on a 9 inch (minimum) bed of course No. 1 aggregate as defined by Item 421, Gravet shall be in place prior to petting box and conduits shall be capped. Any grovel or dirt in conduit shall be removed.

 2. When required by Item descriptive code, construction of an apron excessing a ground box including concrete and reinforcing steel shall not be paid for directly but shall be substituted to the ground box. Reinforcing steel may be field bent. Concrete for aprons shall be considered miscellaneous concrete for testing purposes. Aproximately be cost in place.

 3. Conduit hales may be cut in the walls of type 8 & 0 boxes at least 18 inches beneath the cover.

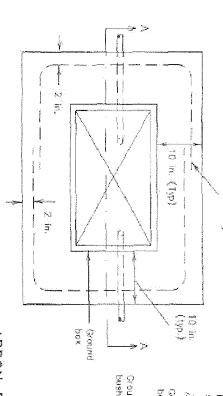
 4. If within the limits of this project, the Contractor must utilize an existing ground box equipped with a metal cover.
- work required as the grounding conductor. Connection of bonding jumper to metal ground cover-shall not be paid for directly but sha be subsidiary to virtius bid items. The box(es) must be dearly shown on the plans with plan notes fully describing the hin the limits of this project. The Contractor must utilize on existing ground box equipped with a metal cover, autructor shell band the sover 1s the grounding conductor with a 3 foot long flexible standed jumper the same size e-grounding conductor. Connection of bonding jumper to metal ground cover shall not be paid for directly but shall

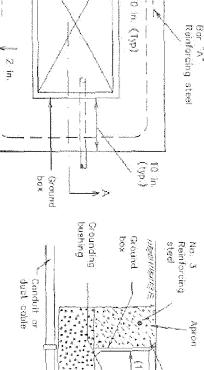
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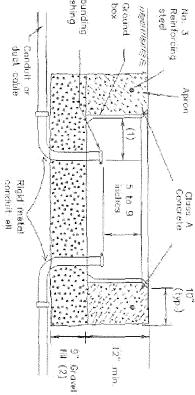
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BOX X

- If there are other ground boxes with metal Covers within the project limits but not involved in the controct, the Engineer may direct the Contractor to ground the covers, designating and identifying the specific boxes in writing Engineer may direct the Contractor This work will be paid for separately
- Termination to metal ground box covers shall be made using a tack ground type lug







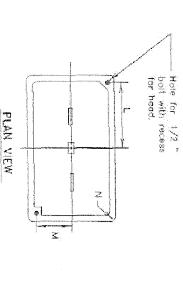
APRON FOR (Where required) GROUND BOXES

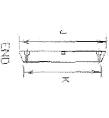
SECTION A - A

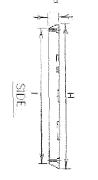
PLAN VIEW

- Final position of and of conduit shall not exceed one-half the distance to the side of box appests the conduit entry.
 Place grower "under" the box, not "in" the box. Gravel should not encroach on the interior volume of the box.
 Install bushing on the upper end of all ells.
 Where a ground not is present in the ground box, connect it to any and all equipment grounding conductors using a tisted connector.
 Valuation sufficient space between all conduits so as to allow for proper installation of bushings.
 All conduits shall be installed in a neat and workmanike manner.









GROUND BOX COVER

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1757/64 Traffic signal electrical - sheet GPY DESIGN: BL ARAPAHO ROAD PHASE III Grantham & Associates, Inc. GIMINAS DAS HOT TO SCALE JOB HO: HO.: 3200ETAILS-ELEC

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